

28 September 2020

Document Ref: AKL2020-0231AB Rev.0

Neil Construction Limited PO Box 8751 Auckland 1150

Attention: Trevor Canty

Dear Sir

RE: DUE DILIGENCE, GEOTECHNICAL INVESTIGATION REPORT
71 TRIG ROAD, WHENUAPAI

## 1 INTRODUCTION AND SCOPE

CMW Geosciences (CMW) was engaged by Neil Construction Limited as part of their due diligence process to carry out preliminary geotechnical investigation and eporting of the site at 71 Trig Road, Whenuapai, which is being considered for the construction of light industrial buildings.

The scope of work and associated terms and conditions of our engagement were detailed in our services proposal letter referenced AKL2020-0231AA Rev.0, dated 10 September 2020.

The purpose of this report is to describe he investigation completed, the ground conditions encountered and to provide recommendations with respect to geotechnical aspects of the proposed subdivision development as detailed in our proposal letter

#### 2 SITE DESCRIPTON

The site is located at 71 Trig Road, Whenuapai, legally described as Lots 2 and 5 and Lot 2 DP 101583, Lot 2 DP 117365 as illustrated in *Figure 1* below. The property has an approximate area of 6.9 hectares and is roughly rectangular in shape, stretching in a west to east direction.

The gradient across the site is generally consistent, falling gradually from the south at approximately RL42.0m down to RL25.5.0m in the north eastern corner with an approximate gradient of 1(V):15(H) across most of the site.

Auckland Council GIS maps a possible creek exiting the north-eastern corner of the site with numerous contributary overland flow paths mapped across the site.

The site itself currently comprises mostly pasture and overgrown, historic horticultural patches. Tall trees run along most of the paddock fence lines as well as being scattered in the north and north eastern areas of the property. An existing structure is located along the northern boundary.

The site is bound to the north by residential dwellings in the form of larger lifestyle blocks and to the west and south by farmland. The eastern boundary backs onto the wider Trig and Brigham Creek Road development site which has recently been cleared in preparation for earthworks to be undertaken (CMW has also been engaged by the client as the Geotechnical Engineer on this adjacent site).

The subject site is accessed by a gravel driveway which runs along the southern boundary and extends from the south-western corner off Trig Road.



Figure 1: Site Location (Google Maps)

#### 3 PROPOSED DEVELOPMENT

At the time of undertaking our investigation and reporting the project was in the early stages of planning and it was anticipated the geotechnical investigation would provide details of preliminary feasibility options for the site.

No architectural or engineering design drawings have been supplied to date and we have prepared this report on the basis that a future dev lopment will broadly comprise minor cuts and fills to form a near level site supporting industrial buildings with shallow strip and pad foundations and widespread floor loads of up to 20kPa.

#### 4 INVESTIGATION SCOPE

#### 4.1 Desktop Study

A desktop study was carried out before commencing fieldwork. This included online research through Auckland Council Geo Maps, Dial Before You Dig, aerial photographs and review of existing information.

A review of historic and recent aerial photographs between 1959 and 2017 indicated the following activity:

Between 1959 and 1996 the subject site was developed from agricultural land for horticultural purposes.
 The existing structure on site appears to have been constructed around the same time as the development of the land.

- From 2003 to approximately 2012 the sites horticultural development appears to have been let go and the area returned to farmland.
- From approximately 2015 onwards the site appears to have been returned to horticultural activities, however, to date the site is currently unattended.

## 4.2 Field Investigation

Following a Dial Before You Dig review, the field investigation was carried out on 16 Sep ember 2020 All fieldwork was carried out under the direction of CMW Geosciences in general accordance with the NZGS guidelines<sup>1</sup>. The scope of the fieldwork was as follows:

- A walkover survey of the site to assess the general landform, site conditions and adjacent structures /
  infrastructure. The site walkover generally confirmed the existing contours of the area and there was no
  evidence of any recent change in the site conditions.
- Ten hand auger boreholes, denoted HA01-20 to HA10-20, were d illed using a 50mm diameter auger to
  target depths of up to 5.0m below existing ground levels to observe the near surface soil profile and to
  facilitate vane shear strength testing. All ten hand auger boreholes reached the target depth of 5.0m.
  Engineering logs of the hand augers, together with peak and remoulded vane shear strengths can be
  found appended to this report.

The approximate locations of the respective investigation sites referr d to above are shown on the Site Investigation Plan appended to this report. Test locations were measured using a handheld GPS. Elevations were inferred from the existing Auckland Council GIS contours.

#### 5 GROUND MODEL

### 5.1 Published Geology

Published geological maps<sup>2</sup> for the area depict the regional geology as comprising Late Pliocene to Mid Pleistocene alluvial deposits of t e Puketoka Formation as illustrated in *Figure 2* below.

These alluvial clay and silt deposits include pumiceous mud, sand and gravel with muddy peat and lignite, rhyolitic pumice (including non-welded gnimbrite, tephra and alluvial pumice deposits) and massive micaceous sand beds. Below these upper soil layers, the deeper geological formation is reported to comprise interbedded muddy sandstones and siltstones of the East Cast Bays Formation within the Waitemata Group.

Based on t e known history of the site and surrounding land levels, we expect that uncompacted fill may be present across some areas of the site due to soft landscaping and historic development.

<sup>&</sup>lt;sup>1</sup> NZ Geotechnical Society (2005), Field Description of Soil and Rock, Guideline for the field classification and description of soil and rock for engineering purposes.

<sup>&</sup>lt;sup>2</sup> Edbrooke, S. W. (compiler) 2001: Geology of the Auckland area. Institute of Geological & Nuclear Sciences 1:250 000 geological map 3. 1 sheet +74 p. Lower Hutt, New Zealand. Institute of Geological & Nuclear Sciences.

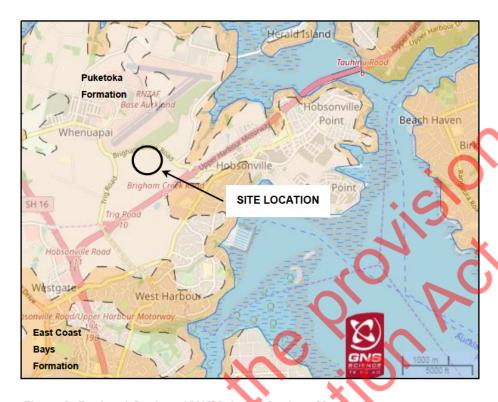


Figure 2: Regional Geology (GNS Science Geology Map)

### 5.2 Stratigraphic Units

#### 5.2.1 Topsoil

Topsoil was encountered in all hand auger bor holes from depths of 0.1m to 0.4m. Topsoil was generally moist to wet and dark brown.

#### 5.2.2 Puketoka Formation Alluvium

Alluvial deposits of the Puketoka Formation were encountered in all hand auger boreholes underlying the topsoil and to the target depth of 5.0m. The extent of the alluvium was not observed due to the limitation on investigation depth. The alluvium generally comprised orange, brown, dark brown, grey mottled orange and black, clays silty clays, clayey silts, silts and sandy silts. Minor organic staining and wood fragments were encountered throughout the alluvium.

These soils were gen rally firm to very stiff with vane shear strengths ranging from 39kPa to >193kPa and averaging around 100kPa to 130kPa. Outlier vane shear strengths of 40kPa in HA02-20, 52kPa in HA06-20 and 39kPa in HA08-20 were generally encountered where the groundwater was sitting or within organic stained material.

#### 5 2 3 Waitemata Group Soils

Wait mata Group soil were not clearly identified within any of the hand auger boreholes, however based on investigations undertaken on the adjacent site we expect these soils to underlie the site at depth.

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#### 5.3 Groundwater

During the investigation, which was completed in early spring conditions (September 2020), groundwater was encountered within the boreholes at the depths provided in Table 1 below.

Table 1: Groundwater Data		
Borehole	September 2020	
	Depth (mbgl)	Elevation (m RL)
HA01-20	4.0	29.0
HA02-20	2.2	32.6
HA03-20	4.4	35.6
HA04-20	3.2	29.5
HA05-20	4.5	31.0
HA06-20	2.5	29.3
HA07-20	10	28.8
HA08-20	27	26.7
HA09-20	4.8	31.1
HA010-20	2.2	29.7
Note: mbgl = metres below ground level. NE = Not Encountered.		

Groundwater was encountered in a l of the hand auger boreholes during out investigation, between RL35.6m and RL26.7m. However, given the presence of a variable and clayey soil profile, it is possible that perched groundwater may occur at different levels during and following periods of rainfall. It should also be noted that groundwater will fluctuate seasonally.

## 6 PRINCIPAL GEOHAZARDS AND RECOMMENDATIONS

#### 6.1 General

On the basis of our investigation and review of findings we are satisfied that the site is generally geotechnically suitable for a subdivision containing light industrial structures, subject to the comments and recommendations below.

### 6.2 Liquefaction

Soil liquefaction is a process where typically saturated, granular soils develop excess pore water pressures during cyclic (earthquake) loading that exceed that effective stress of the oil. In loose soils, some dilation can occur during this process, which can lead to individual soil grains moving into suspension. Following the onset of liquefaction, the shear strength and stiffness of the liquefied soil is effectively lost causing excessive differential settlement of the ground surface, bearing capacity failure and collapse of structures and low-angle

CMW Geosciences Ref: AKL2020-0231AB Rev.0 lateral spreading of slopes in liquefiable soils. In accordance with the NZGS guidance<sup>3</sup> the liquefaction susceptibility of the soils on this site has been considered with respect to geological age and soil fabric.

Across this site, soils below the water table comprise alluvial deposits of the Puketoka Formation. These soils are significantly older than what case history data would suggest as being susceptible to liquefaction. In addition, they typically comprise silty clays and clays that are considered to be too fine grained to be at risk of liquefaction.

### 6.3 Load Induced Settlement

Based on the materials observed in our boreholes, settlement is considered to be a low risk for light w ig t commercial or industrial buildings. However, it is recommended that any soft materials in the creek/guly uncovered as part of earthworks operations, are undercut, and replaced with compacted engineered fill.

Soft subsoils may be subject to consolidation settlements due to potential loadings from industrial buildings and floor slabs. Pre-loading of soft soils, general ground improvement during earthworks and possibly piling, or reinforced fill rafts and basal reinforcements may be necessary to mitigate any significant settlement hazards across the industrial zones.

Depending on the proposed earthworks plans, settlement analyses may need to be undertaken as part of any future detailed investigation and design. This will allow for the development of appropriate ground remediation options if necessary.

## 6.4 Seismic Site Subsoil Category

Based on the conditions observed during the investigation, combined with experience working in the surrounding area, the seismic site subsol category is provisionally assessed as being Class C (shallow soil site) in accordance with NZS 1170.5

#### 6.5 Earthworks

All earthwork activities must be carried out in general accordance with the requirements of NZS 4431 and the requirements of the Auckland Council Infrastructure Development Code under the guidance of a Chartered Professional (Geotechnical) Engineer.

## 7 FURTHER WORK

This investigation and reporting has been undertaken to support a pre-purchase assessment of the site. If purchase proceeds, furthe detailed investigation will be required once design/development plans have been con irmed.

#### 8 LIMITATION AND CLOSURE

This report has been prepared for our client Neil Construction Limited. Liability for its use is limited to these parties and to the scope of work for which it was prepared, as it may not contain sufficient information for other parties or for other purposes.

It should be noted that factual data for this report has been obtained from discrete locations using normal geotechnical investigation techniques. As such investigation methods by their nature only provide information about a relatively small volume of subsoils, there may be special conditions pertaining to this site which have not been disclosed by the investigation and which have not been considered in the report.

<sup>&</sup>lt;sup>3</sup> Earthquake Geotechnical Engineering Practice, Module 3: Identification, assessment and mitigation of liquefaction hazards", (May 2016)

If site conditions encountered vary from those outlined above and/or if any unforeseen conditions develop, CMW must be advised immediately such that we can review the recommendations and advise any changes that may be required.

#### For and on behalf of CMW Geosciences

Prepared by:

Reviewed and authorised by:

Jasmine Walden

**Project Engineering Geologist** 

Andrew Linton

Principal Geotechnical Engineer

Distribution: 1 electronic copy to Neil Construction Limited via email

Original held at CMW Geosciences

Attachments: Site Investigation Plan

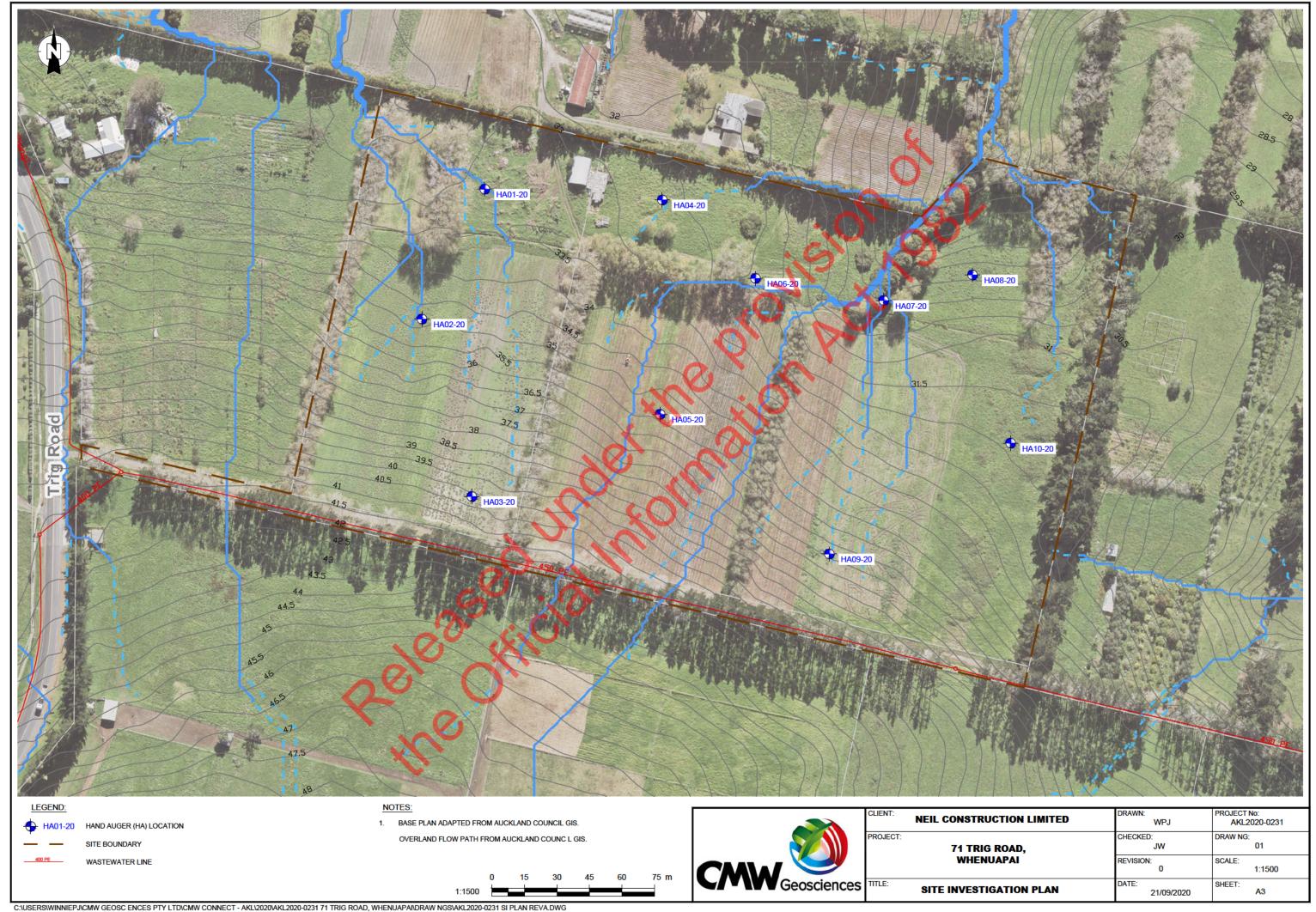
Hand Auger Borehole Logs











## HAND AUGER BOREHOLE LOG - HA01-20

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231

Date: 16/09/2020 Checked by: JW Scale: Borehole Location: See site plan Logged by: FS Sheet 1 of 1 Position: 1744840.0mE; 5926203.0mN Projection: NZTM Elevation: 33.00m Datum: AUCKHT1946 Survey Source: AC Geomaps Dynamic Cone Penetrometer Samples & Insitu Tests **3raphic** Log Groundwater Moisture Condition Material Description
Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)
Rock: Colour; fabric; rock name; additional comments. (origin/geological unit)  $\widehat{\Xi}$ (Blows/100mm) Depth 귐 Cons 10 Type & Results Depth 33.0 OL: TOPSO L: dark brown. 32.7 ML: Clayey SILT: brown mottled dark brown and orange. Very stiff, low plasticity, moderately sertitive. Peak = 155kPa Residual = 40kPa 0.4 (Puketoka Formation) ... at 0.60m, becoming greyish brown mottled orange Peak = 134kPa Residual = 37kPa 0.8 ... at 1.00m, becoming light grey mottled orange. Peak = 142kPa Residual = 40kPa 1.2 1.6 Peak = 174kPa Residual = 67kPa 2.0 Peak = 136kPa Residual = 67kPa VSt 30.7 CH: CLAY wit minor silt: light grey. Stiff to very s ff high plasticity, insensitive to moderately sensitive. (Puketoka 2.4 Peak = 110kPa orm t n) Residual = 64kPa 2.8 Peak = 107kPa Residual = 64kPa ... at 3.00m, becoming light grey mottled orange 3.2 Peak = 110kPa Residual = 70 P at 3.20m, boming dark grey with minor black mottling. ... at 3.80m, silt becoming absent. Peak = 104kPa Residual = 67kPa ... at 4.20m, becoming grey. Peak = 78kPa s St ... at 4.60m, becoming dark grey. 4.8 Peak = 88kPa Residual = 67kPa

Termination Reason: Target depth reached
Shear Vane No: 1620 DCP No:
Remarks: Groundwater encountered at 4.0m.

This report is based on the attached field descrip ion for soil and rock, CMW Geosciences - Field Logging Guide, Revision 3 - April 2018.

# **HAND AUGER BOREHOLE LOG - HA02-20**

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231

Date: 16/09/2020

Borehole Location: See site plan Logged by: FS Checked by: JW Scale: Sheet 1 of 1 Position: 1744811.0mE; 5926143.0mN Projection: NZTM Elevation: 34.75m Datum: AUCKHT1946 Survey Source: AC Geomaps Dynamic Cone Penetrometer Samples & Insitu Tests **3raphic** Log Groundwater Moisture Condition Material Description
Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)
Rock: Colour; fabric; rock name; additional comments. (origin/geological unit)  $\widehat{\Xi}$ (Blows/100mm) Depth 귐 Cons 10 Type & Results Depth 34.8 OL: TOPSO L: dark brown. 34.4 ML: Clayey SILT: brown and grey. Stiff to very stiff, low plasticity, moderately sensitive Peak = 83kPa Residual = 21kPa 0.4 (Puketoka Formation) Peak = 123kPa Residual = 59kPa 0.8 ... at 0.80m, becoming grey mottled orange Peak = 112kPa Residual = 48kPa 1.2 ... at 1.20m, becoming light grey. St to VSt 1.6 Peak = 128kPa Residual = 53kPa Peak = 140kPa Residual = 59kPa 2.0 at 2.00m, becoming I ht gi 2.4 Peak = 155kPa 32.4 CL: CLAY w h trace silt: dark grey. Very stiff o hard, low plasticity. (Puke ka Formation) Residual = 64kPa 2.8 Peak = >187 from 3.00m to 3.10m, contains trace fine grained sand VSt to 3.2 Peak = 120kPa ML: SILT with trace clay: dark grey. Firm, low plasticity, insensitive. Residual = 60kPa (Puketoka Formation) VSt 40kPa F 4.8 Peak = 160kPa sidual = 107kPa VSt Borehole terminated at 5.0 m

Termination Reason: Target depth reached
Shear Vane No: 1620 DCP No:
Remarks: Groundwater encountered at 2.2m.

# **HAND AUGER BOREHOLE LOG - HA03-20**

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231



Borehole Location: See site plan Logged by: FS Checked by: JW Sheet 1 of 1 Scale: Position: 1744834.0mE; 5926061.0mN Projection: NZTM Elevation: 40.00m Datum: AUCKHT1946 Survey Source: AC Geomaps Dynamic Cone Penetrometer Samples & Insitu Tests Graphic Log Groundwater Moisture Condition Material Description
Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)
Rock: Colour; fabric; rock name; additional comments. (origin/geological unit)  $\widehat{\Xi}$ (Blows/100mm) Depth 귐 Cons 10 Type & Results Depth 40 O OL: TOPSO L: dark brown. Peak = 91kPa Residual = 13kPa ML: Clayey SILT: greyish brown mottled orange. Stiff to very stiff, low plasticity, insensitive to sensi (Puketoka Formation) 0.4 39.6 Peak = 185kPa Residual = 63kPa 0.8 ... at 0.80m, becoming light brownish grey mottled orange. Peak = UTP 1.2 ... at 1.20m, becoming light grey mottled orange. 1.6 Peak = 160kPa Residual = 88kPa Peak = 150kPa Residual = 96kPa 2.0 2.4 Peak = 174kPa St to Residual = 110kPa 2.8 Peak = 147kPa Residual = 90kPa n bec ming orange mottled grey. 3.2 Peak = 120kPa Residual = 64 P eak 131kPa esid al = 70kPa Peak = 118kPa Residual = 67kPa W to ... at 4.20m, becoming light brownish grey. Peak = 96kPa 35.5 CL: CLAY with minor silt: dark grey. Very stiff, low plasticity, insensitive. (Puketoka Formation) VSt 4.8 Peak = 160kPa Borehole terminated at 5.0 m

Termination Reason: Target depth reached
Shear Vane No: 1620 DCP No:
Remarks: Groundwater encountered at 4.4m.

# **HAND AUGER BOREHOLE LOG - HA04-20**

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231

Date: 16/09/2020



Position: 1744922.0mE; 5926198.0mN Projection: NZTM Datum: AUCKHT1946 Elevation: 32.75m Survey Source: AC Geomaps Samples & Insitu Tests Graphic Log Groundwate Material Description

Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)

Rock: Colour; fabric; rock name; additional comments. (origin/geological unit) Moisture Condition  $\widehat{\mathbf{E}}$ (Blows/100mm) Depth Cons 싵 Depth Type & Results 32.8 OL: TOPSO L: dark brown. 324 ML: Clayey SILT: orangeish brown. Stiff to very stiff, low plasticity, insensitive to moderately sens 0.4 (Puketoka Formation) Peak = 134kPa Residual = 59kPa 0.8 ... at 0.80m, becoming grey mottled orange Peak = 136kPa Residual = 53kPa 1.2 .. at 1.50m, becoming light grey. Peak = 127kPa 1.6 Residual = 67kPa at 1 90m, becoming grey, and dark grey 2.0 Peak = 147kPa 2 Residual = 78kPa at 2.20m, b coming lig grev mottled orang St to 2.4 Peak = 131kPa Residual = 67kPa at 2 0m, becoming with m nor c ay and ace fine grained sand, whitish grey mottled orange. 2.8 Peak = 126kPa Residual = 62kPa 3.2 Peak = 107kPa Residual = 59 P at 3.50m, becoming with some fine grained sand, clay becoming absent. eak = 88kPa Resid al = 43kPa Peak = 91kPa Residual = 43kPa 28.6 ML: Sandy SILT: light brownish grey mottled orange. Very stiff, low plasticity, insensitive to moderately sensitive. Sand is fine grained, poorly graded. (Puketoka Formation) Peak = 110kPa VSt 4.8 Peak = 115kPa Borehole terminated at 5.0 m

Termination Reason: Target depth reached Shear Vane No. DCP No: Remarks: Groundwater encountered at 3.2m.

## **HAND AUGER BOREHOLE LOG - HA05-20**

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231

Date: 16/09/2020 Checked by: JW Scale: Borehole Location: See site plan Logged by: FS Sheet 1 of 1 Position: 1744921.0mE; 5926099.0mN Projection: NZTM Elevation: 35.50m Datum: AUCKHT1946 Survey Source: AC Geomaps Dynamic Cone Penetrometer Samples & Insitu Tests **3raphic** Log Groundwater Moisture Condition Material Description
Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)
Rock: Colour; fabric; rock name; additional comments. (origin/geological unit)  $\widehat{\Xi}$ (Blows/100mm) Depth Cons 귐 Type & Results Depth 35.5 OL: TOPSO L: dark brown. 35.2 ML: Clayey SILT: dark brownish grey mottled orange. Very stiff to hard, low plasticity, insensitive Peak = 107kPa Residual = 27kPa 0.4 (Puketoka Formation) Peak = 120kPa Residual = 37kPa 0.8 Peak = 144kPa Residual = 56kPa 1.2 ... at 1.20m, becoming grey mottled orange at 1.50m, becoming light grey mottled orange 1.6 Peak = >187 VSt to Peak = >187 2.0 2.4 Peak = >187 2.8 Peak = 174kPa Residual = 107kPa 3.2 Peak = 120kPa Residual = 59 P CH: CLAY with ace silt: light grey mottled orange and red. Stiff, low plasticity, insensitive. (Pu etok Fo mation) at 3.50m, becoming dark brownish grey mottled black. ... at 3.80m, becoming grey with minor black mottling. Peak = 88kPa Residual = 64kPa St Peak = 67kPa ... from 4.50m to 4.70m, contains 200mm lens of dark brown to black clay. s 4.8 Peak = 78kPa Residual = 64kPa

Termination Reason: Target depth reached
Shear Vane No: 1620 DCP No:
Remarks: Groundwater encountered at 4.5m.

This report is based on the attached field descrip ion for soil and rock, CMW Geosciences - Field Logging Guide, Revision 3 - April 2018.

## **HAND AUGER BOREHOLE LOG - HA06-20**

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231

Date: 16/09/2020 Checked by: JW Scale: Borehole Location: See site plan Logged by: CK Sheet 1 of 1 Position: 1744965.0mE; 5926161.8mN Projection: NZTM Elevation: 31.80m Datum: AUCKHT1946 Survey Source: AC Geomaps Dynamic Cone Penetrometer Samples & Insitu Tests Graphic Log Groundwater Moisture Condition Material Description
Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)
Rock: Colour; fabric; rock name; additional comments. (origin/geological unit)  $\widehat{\Xi}$ (Blows/100mm) Depth 귐 Cons 10 Type & Results Depth 31.8 TOPSOIL: (Puketoka Formation) 31.5 CH: CLAY with minor silt: light grey streaked trace orange and light brown. High plasticity, mode ately Peak = 165kPa Residual = 47kPa 0.4 sensitive (Puketoka Formation) ... from 0.70m to 1.20m, ... becoming CLAY Peak = 141kPa Residual = 44kPa 0.8 1.2 Peak = 132kPa Residual = 66kPa ... from 1.20m to 2.60m, ... becoming whitish grey streaked range nd light brown 1.6 Peak = 135kPa Residual = 52kPa Peak = 177kPa Residual = 83kPa VSt 2.0 Peak = 171kPa 2.4 Residual = 85kPa 29.2 CH: S ty CLAY: light greyish whee mo I orange and black. High plasticity, moderately sensitive. (Puketoka Formation) 2.8 Peak = 116kPa Residual = 41kPa 3.2 Peak = 135kPa Residual = 50 P 27.9 CH: CLAY: black. High plasticity Peak = 52kPa Residual = 39kPa (Puketoka Formation) 27.6 CH: Silty CLAY with minor sand: light brownish grey mottled black. High plasticity, sand is fine grained. (Puketoka Formation) Peak = 91kPa St 4.8 Peak = 138kPa VSt

Termination Reason: Target depth reached
Shear Vane No: 2904 DCP No:
Remarks: Groundwater encountered at 2.5m.

This report is based on the attached field descrip ion for soil and rock, CMW Geosciences - Field Logging Guide, Revision 3 - April 2018.

## **HAND AUGER BOREHOLE LOG - HA07-20**

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231

Date: 16/09/2020 Borehole Location: See site plan Logged by: CK Checked by: JW Sheet 1 of 1 Scale: Position: 1745024.3mE; 5926151.6mN Projection: NZTM Datum: AUCKHT1946 Elevation: 29.75m Survey Source: AC Geomaps Dynamic Cone Penetrometer Samples & Insitu Tests **3raphic** Log Groundwater Moisture Condition  $\widehat{\Xi}$ Material Description (Blows/100mm) Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)
Rock: Colour; fabric; rock name; additional comments. (origin/geological unit) Depth 귐 Cons 10 Type & Results Depth 29.8 TOPSOIL: 29.6 MH: Clayey SILT: dark brown streaked orange. Low plasticity (Puketoka Formation) Peak = 152kPa Residual = 63kPa 0.4 29.2 CH: CLAY: light greyish brown mottled black. High plasticity, insensitive to sensitive. (Puketoka Formation) Peak = 97kPa Residual = 22kPa 0.8 ... from 1.10m to 3.10m, ... becoming streaked orange and dark brow Peak = 74kPa Residual = 17kPa 1.2 1.6 Peak = 77kPa St Residual = 14kPa Peak = 69kPa 2.0 Residual = 28kPa 2.4 Peak = 102kPa Residual = 52kPa VSt 2.8 Peak = 72kPa Residual = 33kPa w .10m to 3 70m, ... becoming light brown mottled black ... from 3.2 Peak = 97kPa Residual = 44 P eak = 97kPa sid\_al = 41kPa ... from 3.70m to 4.10m, ... becoming dark greyish brown mottled trace dark orange Peak = 85kPa Residual = 25kPa ... from 4.10m to 5.00m, ... becoming grey with trace of hard clay inclusions which are coarse gravel sized Peak = 94kPa 4.8 Peak = 97kPa

Termination Reason: Target depth reached
Shear Vane No: 2904 DCP No:
Remarks: Groundwater encountered at 1.0m.

This report is based on the attached field descrip ion for soil and rock, CMW Geosciences - Field Logging Guide, Revision 3 - April 2018.

# **HAND AUGER BOREHOLE LOG - HA08-20**

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231



Borehole Location: See site plan Logged by: CK Checked by: JW Scale: Sheet 1 of 1 Position: 1745065.3mE; 5926163.3mN Projection: NZTM Elevation: 29.40m Datum: AUCKHT1946 Survey Source: AC Geomaps Dynamic Cone Penetrometer Samples & Insitu Tests **3raphic** Log Groundwater Moisture Condition Material Description
Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)
Rock: Colour; fabric; rock name; additional comments. (origin/geological unit)  $\widehat{\Xi}$ (Blows/100mm) Depth 귐 Cons 10 Type & Results Depth 29.4 TOPSOIL: 29.1 CH: CLAY: light grey streaked orange and brown. High plasticity, insensitive to moderately sensi Peak = 168kPa Residual = 66kPa 0.4 (Puketoka Formation) Peak = 165kPa Residual = 69kPa 0.8 Peak = 149kPa Residual = 72kPa 1.2 VSt ... from 1.20m to 1.80m, ... becoming whitish grey streaked ace dak orange 1.6 Peak = 171kPa Residual = 99kPa ... from 1.80m to 2.50m, ... becoming w tish grey Peak = 141kPa Residual = 85kPa 2.0 2.4 Peak = 69kPa Residual = 41kPa from 2 50m to 2.70m, ... becomin black St be oming ght greyish brown streaked black 2.8 Peak = 39kPa Residual = 17kPa F ... from .10m to 4 20m, ... becoming streaked orange 3.2 Peak = 69kPa Residual = 39 P St Peak = 113kPa Residual = 55kPa VSt ... from 4.20m to 5.00m, ... becoming light greyish brown with minor 1-5cm black wood fragment Peak = 83kPa St 4.8 Peak = 88kPa Residual = 41kPa Borehole terminated at 5.0 m

Termination Reason: Target depth reached
Shear Vane No: 2904 DCP No:
Remarks: Groundwater encountered 2.7m.

## **HAND AUGER BOREHOLE LOG - HA09-20**

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231

Date: 16/09/2020 Checked by: JW Scale: 1:25 Borehole Location: See site plan Logged by: CK Sheet 1 of 1 Position: 1744999.2mE; 5926034.5mN Projection: NZTM Elevation: 35.90m Datum: AUCKHT1946 Survey Source: AC Geomaps Dynamic Cone Penetrometer Samples & Insitu Tests **3raphic** Log Groundwater Moisture Condition Material Description
Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)
Rock: Colour; fabric; rock name; additional comments. (origin/geological unit)  $\widehat{\Xi}$ (Blows/100mm) Depth Cons 귐 Type & Results Depth 35.9 TOPSOIL: 35.7 CH: CLAY with minor silt: Orange brown mottled grey and dark brown. High plasticity. (Puketoka Formation) 0.4 Peak = UTP Peak = 163kPa Residual = 74kPa 0.8 1.2 Peak = UTP ... from 1.30m to 2.30m, ... becoming whitish grey streaked ora 1.6 Peak = >193 Peak = UTP D 2.0 VSt ... from 2.30m to 2.70m 2.4 Peak = UTP be oming dark brown streaked black with trace fine grained sand 2.8 Peak = UTP . from 2.90m to 3.30m ... becoming light grey streaked dark brown 3.2 Peak = 149kPa Residual = 11 kP from 3.3 m to 5.00m, ... becoming whitish grey streaked orange and yellow Peak = 108kPa Residual = 80kPa М St 4.8 Peak = 83kPa

Termination Reason: Target depth reached
Shear Vane No: 2904 DCP No:
Remarks: Groundwater encountered at 4.8m.

This report is based on the attached field descrip ion for soil and rock, CMW Geosciences - Field Logging Guide, Revision 3 - April 2018.

## **HAND AUGER BOREHOLE LOG - HA10-20**

Client: Neil Construction Limited

Project: 71 Trig Road Site Location: Whenuapai Project No.: AKL2020-0231

Date: 16/09/2020 Borehole Location: See site plan Logged by: CK Checked by: JW Sheet 1 of 1 Scale: Position: 1745082.9mE; 5926085.7mN Projection: NZTM Elevation: 31.90m Datum: AUCKHT1946 Survey Source: AC Geomaps Dynamic Cone Penetrometer Samples & Insitu Tests **Braphic** Log Groundwater Moisture Condition  $\widehat{\Xi}$ Material Description (Blows/100mm) Soil: Soil symbol; soil type; colour; structure; bedding; plasticity; sensitivity; additional comments. (origin/geological unit)
Rock: Colour; fabric; rock name; additional comments. (origin/geological unit) Depth 귐 Cons 10 Type & Results Depth 31.9 TOPSOIL: 31.7 CH: Silty CLAY: light greyish brown streaked orange. High plasticity. (Puketoka Formation) 0.4 Peak = UTP 31.2 CH: CLAY: whitish grey streaked orange. High plasticity D Peak = 168kPa Residual = 72kPa 0.8 (Puketoka Formation) Peak = 179kPa Residual = 88kPa 1.2 VSt 30.6 ML: Clayey SILT with minor sand: whitish grey streaked orang (Puketoka Formation) Low plasticity, sand is fine gr ined 1.6 Peak = UTP 30.2 CH: Silty CLAY: whitish grey streak d ora ge. High plasticity (Puketoka Formation) М Peak = UTP 2.0  $\blacksquare$ ... from 2.20m to 2 40m. becoming light grey sh whit streaked trace orange 2.4 Peak = 72kPa from 2.40m to 4.00m Residual = 50kPa 2.8 Peak = 69kPa Residual = 41kPa 3.2 Peak = 66kPa Residual = 41 P St eak = 77kPa Resid al = 44kPa Peak = 80kPa Residual = 50kPa ... from 4.00m to 4.40m, ... becoming light greyish white Peak = 102kPa ... from 4.40m to 5.00m, ... becoming with some fine grained sand VSt 4.8 Peak = 61kPa St

Termination Reason: Target depth reached
Shear Vane No: 2904 DCP No:
Remarks: Groundwater encountered at 2.2m.

This report is based on the attached field descrip ion for soil and rock, CMW Geosciences - Field Logging Guide, Revision 3 - April 2018.