

**Appendix B: Investigation Data** 



COMMENT:

## **EXCAVATION LOG**

EXCAVATION NUMBER:

TP 1

Logged By: MDP

Sheet: 1 of 1

Checked Date:

	ROJECT: Northbro		e		INCLINATION: VERTION	CAL	JOB NI	UMBER: 200490
E	ASTING:		mE	EQUIPMENT:	8 T Excavator	OPI	RATOR:	Ethan
	RTHING:		mN	INFOMAP NO.			MPANY:	Diverse Works
ELE	VATION:		m	DIMENSIONS:		HOLE S	TARTED:	7-Jun-17
N	ИЕТНОD:			EXCAV. DATUM:		HOLE FI	NISHED:	7-Jun-17
DEРТН (m)	SOIL / ROCK TYPE	GRAPHIC LOG		DESCRIPTIC	io	USCS GROUP	GROUNDWATER / SEEPAGE	ŞCALA PENETROMETER
0.2	TOPSOIL	ς Σ	Dark brown, orga	anic SILT. Silt is non-p	lastic. Moist.			
0.7	LOESS	XX XX XX XX	Moist.		dium. Silt is non-plastic. Firm.	Y		
0.9	COLLUVIUM	$\rho \times \rho$	Brown, silty GRA Loose. Moist.	VEL. Gravel is fine to	coarse. Silt is non-plastic.			
1.4	OUTWASH GRAVEL	0.000	Grey, sandy GRA coarse. Medium	dense. Moist.	coarse. Sand is fine to			
2.1	OUTWASH GRAVEL		Grey, bouldery s dense, Moist.	andy GRAVEL. Boulde	rs up to 500mm. Medium			
2.9	GLACIAL TILL	× × × ×	Grey, silty SAND non-plastic. Med	with some gravel. San lium dense. Moist.	d is fine to medium. Silt is		Minor seepage	
	OUTWASH GRAVEL				L. Sand is fine to coarse.		or see	
32		0	Gravel is fine to	coarse. Medium dens	e. Moist.		Mino	
J.Z	OUTWASH SAND		Grey, SAND and	silty SAND. Sand is fin	e to coarse. Silt is non-			
3.3			plastic. Medium	dense. Wet.				
	111		Total Depth = 3.3 r	m				



EXCAVATION NUMBER:

TP 2

Checked Date:

Sheet: 1 of 1

	CATION: See Site		е			INCLINATION: VEI	RTICAL	JOB N	UMBER: 200490
F	ASTING:			mE	FOLIIPMENT:	8 T Excavator	ΛP	ERATOR:	Ethan
	ORTHING:			mN	INFOMAP NO.	O I EXCAVATOR		MPANY:	Diverse Works
	VATION:			m	DIMENSIONS:			TARTED:	7-Jun-17
	METHOD:			1	EXCAV. DATUM:			NISHED:	7-Jun-17
	<u>'</u> I								
DEРТН (m)	SOIL / ROCK TYPE	GRAPHIC LOG			DESCRIPTIO	40	USCS GROUP	GROUNDWATER / SEEPAGE	SCALA PENETROMETER
0.4	TOPSOIL	3×3 3×3		own, orga stic. Mois		and sandy SILT. Silt is	P	),	
2.7	GLACIAL TILL		Grey/bi Silt is n	rown, silty on-plastic	ty SAND with some grace. Medium dense. Mo	vel is fine to coarse. Medi	m.	NO SEEPAGE	
			rotal Dep	oth = 3.5 r	n				
	COMMENT:							l o	ogged By: MDP



EXCAVATION NUMBER:

	ROJECT: Northbro		е			INCLINATION	ON: VERTIC	٠٧١	JOB N	IUMBER:	200490
		гіан		_	FOLUDATA				DATOR		-11
	EASTING: PRTHING:			mE mN	INFOMAP N	T: 8 T Excavator	r		RATOR: MPANY:		than se Works
	VATION:			m	DIMENSION			HOLE S			Jun-17
	METHOD:		ı	111	EXCAV. DATUM			HOLE FI			Jun-17
	<u>'</u>					1				V	
DEРТН (m)	SOIL / ROCK TYPE	GRAPHIC LOG			DESCRIP <sup>-</sup>			USCS GROUP	GROUNDWATER / SEEPAGE		CALA PROMETER
0.4	FILL		to coars		andy GRAVEL. Sand . Moist.	d is fine to coarse	. Graver is line	P			
1.5	OUTWASH GRAVEL		predomi	inantly m	dy GRAVEL with mi ledium to coarse. G um dense. Moist.						
3.5	GLACIAL TILL		medium	ty gravel	ilty SAND with som on-plastic, Medium y SAND. Sand is fir n-plastic. Medium o	dense. Moist.			NO SEEPAGE		
	<b>!</b>		Total Dept	h = 4 m				<u> </u>			

COMMENT:	Logged By: MDP
	Checked Date:
	Sheet: 1 of 1



COMMENT:

## **EXCAVATION LOG**

**EXCAVATION NUMBER:** 

TP 4

Logged By: MDP Checked Date:

Sheet: 1 of 1

EASTING:  MR  MORTHING:  MN  MN  MNFOMAP NO.  METHOD:  ME		CATION: See Site		e 			INCLINATION: \	VERTICAI	L	JOB N	UMBER: 200490
ELEVATION:  METHOD:  DESCRIPTION  DESCRIPTIO	E	ASTING:		ml	E	EQUIPMENT:	8 T Excavator		OPE	RATOR:	Ethan
METHOD:  EXCAV. DATUM:  DESCRIPTION  DESCRIPTION  TOPSOIL  O.3  COLLUVIUM  Brown, gravelly SILT with some sand. Gravel is fine to coarse. Silt is non-plastic. Firm. Moist.  GLACIAL TILL  GREY, SIND SAND with some gravel. Sand is fine to medium. Silt is				ml	N						
SOIL / ROCK TYPE  TOPSOIL  O.3  COLLUVIUM  GLACIAL TILL  Grey, silty SAND with some gravel. Sand is fine to medium. Silt is  DESCRIPTION  DESCRIPTION  DESCRIPTION  DESCRIPTION  SCALA PENETROMETER  SCALA PENETROMETER  SCALA PENETROMETER  SCALA PENETROMETER  SCALA PENETROMETER  SCALA PENETROMETER  GLACIAL TILL  Grey, silty SAND with some gravel. Sand is fine to medium. Silt is				m							
Dark brown, organic SILT with roots. Silt is non-plastic. Moist.  COLLUVIUM  Brown, gravelly SILT with some sand. Gravel is fine to coarse. Silt is non-plastic. Firm. Moist.  GLACIAL TILL  Grey, silty SAND with some gravel. Sand is fine to medium. Silt is	IV	TETHOD:				EXCAV. DATUIVI:		Į H	IOLE FII	MISHED:	7-Jun-17
0.3  COLLUVIUM  Brown, gravelly SILT with some sand. Gravel is fine to coarse. Silt is non-plastic. Firm. Moist.  0.7  GLACIAL TILL  Grey, silty SAND with some gravel. Sand is fine to medium. Silt is							٠,	j	USCS GROUP	GROUNDWATER / SEEPAGE	
COLLUVIUM  Brown, gravelly SILT with some sand. Gravel is fine to coarse. Silt is non-plastic. Firm. Moist.  O.7  GLACIAL TILL  Grey, silty SAND with some gravel. Sand is fine to medium. Silt is		TOPSOIL		Dark brown	n, orga	nic SILT with roots. S	ilt is non-plastic. Mois	st			
0.7 GLACIAL TILL Grey, silty SAND with some gravel. Sand is fine to medium. Silt is		COLLUVIUM	$ abla^{\circ}  abla$	Brown, gra	vellv S	ILT with some sand.	Gravel is fine to coars	e. Silt is	-		
	0.7			non-plastic	c. Firm	. Moist.	$\mathcal{O}$				
3.8				non-plastic	c. Medi	ium dense. Moist.				NO SEEPAGE	
Total Depth = 3.8 m			'	Total Depth =	= 3.8 m	n					



EXCAVATION NUMBER:

	CATION: See Site		<u>e</u>		INCLINATION: VERT	ΓICAL	JOB N	UMBER: 200490
E	ASTING:		mE	EQUIPMENT:	8 T Excavator	OP.	ERATOR:	Ethan
	RTHING:		mN	INFOMAP NO.			MPANY:	Diverse Works
ELE	VATION:		m	DIMENSIONS:		HOLE S	TARTED:	7-Jun-17
Λ	ИЕТНОD:			EXCAV. DATUM:		HOLE F	INISHED:	7-Jun-17
DEРТН (m)	SOIL / ROCK TYPE	GRAPHIC LOG	Dock brown orga	DESCRIPTIO	ON Oilt is non-plastic, Moist	USCS GROUP	GROUNDWATER / SEEPAGE	SCALA PENETROMETER
0.2	TOPSOIL	$^3\times$	Dark brown, orga	THE SILT WITH TOOLS. S	ont is non-plastic. Woist			
0.3	COLLUVIUM	XşX			Gravel is fine to coarse. Silt	is		
0.7		×××	non-plastic. Firm	i. Moist.				
0.7	GLACIAL TILL	<del>*</del> 🕹 🐴	Grev silty SAND	with some gravel Sa	nd is fine to medium. Silt is			
3.0	GLACIAL TILL	$\times$ $\times$ $\times$ $\times$ $\times$ $\times$ $\times$ $\times$ $\times$	Grey, gravelly silt medium. Silt is n	on-plastic. Medium d	e to coarse. Sand is fine to ense. Moist.		NO SEEPAGE	
	XX		Total Depth = 3.4 n	n				
	COMMENT:	_						gged By: MDP
							Check	ked Date:
								Sheet: 1 of 1



**EXCAVATION NUMBER:** 

	CATION: See Site		е		INCLINATION: VERTI	CAL	JOB N	UMBER: 200490
F	ASTING:		mE	FOUIPMENT:	8 T Excavator	OPI	ERATOR:	Ethan
	RTHING:		mN	INFOMAP NO.	e i Executate.		MPANY:	Diverse Works
	EVATION:		m	DIMENSIONS:			TARTED:	7-Jun-17
	METHOD:			EXCAV. DATUM:		HOLE FI		7-Jun-17
	<u>'</u> I							
DEРТН (m)	SOIL / ROCK TYPE	GRAPHIC LOG	. 011 7 6	DESCRIPTIO	io	USCS GROUP	GROUNDWATER / SEEPAGE	SCALA PENETROMETER
0.2	TOPSOIL	3×		t is non-plastic. Moist.				
0.7	COLLUVIUM	XX XX XX	Brown, sandy SI non-plastic. Firn	LT with some gravel. S n. Moist.	Sand is fine to coarse. Silt is	V		
3.3	GLACIAL TILL			dium dense. Moist	nd is fine to medjum. Silt is		NO SEEPAGE	
<b>T</b>	*//,							
	COMMENT:							ogged By: MDP



EXCAVATION NUMBER:

	CATION: See Site		<u> </u>			INCLINATION: VEF	RTICAL	JOB N	UMBER: 200490
Е	ASTING:			mE	EQUIPMENT:	8 T Excavator	OP	ERATOR:	Ethan
	RTHING:			mN	INFOMAP NO.			MPANY:	Diverse Works
	EVATION:			m	DIMENSIONS:			TARTED:	7-Jun-17
Λ	ИЕТНОD:				EXCAV. DATUM:		HOLE F	INISHED:	7-Jun-17
DEРТН (m)	SOIL / ROCK TYPE	GRAPHIC LOG			DESCRIPTIO	40	USCS GROUP	GROUNDWATER / SEEPAGE	SCALA PENETROMETER
0.2	TOPSOIL	$\times^{3}_{\Sigma}$	Dark bro	own, orga	nic SILT. Silt is non-p	astic. Moist.			
0.3	COLLUVIUM	¥ × ×× ×× × ×	Brown, to coars	sandy SIL se. Schist	T with some gravel a to 700mm. Silt is not	nd schist clasts. Sand is f n-plastic. Firm. Moist.	ine		
3.0	GLACIAL TILL	××××××××××××××××××××××××××××××××××××××	non-pla:	stic. Med	with some gravel. Sai ium dense. Moist.	nd is fine to medium. Silt i	S	NO SEEPAGE	
<b>Y</b>			Total Dep	th = 3 m					
	COMMENT:							Lo	ogged By: MDP
	X								ked Date:
									Sheet: 1 of 1



EXCAVATION NUMBER:

TP8

PROJECT: Northbrook	Village				Job Number: 200490
LOCATION: See Site Pla	n		Inclination:	VERTICAL	Direction:
EASTING:	mE	EQUIPMENT:	20 tonne digger	OPERAT	OR: Sandy
NORTHING:	mN	Infomap no.		COMPA	ANY: Civil Construction
ELEVATION:	m	DIMENSIONS:		HOLE START	TED: 19-Jan-17
METHOD:		EXCAV. DATUM:		HOLE FINISH	IED: 19-Jan-17

						GEOLOGICAL
SCALA PENETRATION	GROUNDWATER / SEEPAGE	DEРТН (m)	GRAPHIC LOG	SOIL / ROCK CLASSIFICATION, PLASTICITY OR PARTICLE SIZE CHARACTERISTICS, COLOUR, WEATHERING, SECONDARY AND MINOR COMPONENTS	WATER CONTENT	SOIL / ROCK TYPE, ORIGIN, MINERAL COMPOSITION, DEFECTS, STRUCTURE, FORMATION
		0.6	3×3	Dark brown, organic SILT with roots.		TOPSOIL
		1.4	$\stackrel{XXX}{\sim}$	Brown to grey, SILT with rootlet voids. Non-plastic. Stiff. Massive.	Moist	LOESS
	9 m	3.0		Brownish grey, sandy GRAVEL with minor cobbles & rare boulders. Gravel is fine to coarse. Sand is medium to coarse. Boulders maximum size 300mm. Medium dense. Bedded.	Moist	OUTWASH GRAVEL
	Seepage @ 3.9	3.9	0.000	Brownish grey, sandy GRAVEL with minor cobbles & a trace of silt. Sand is fine to coarse, Gravel is fine to coarse, Medium dense. Bedded.	Moist	OUTWASH GRAVEL
		5.0	××× ×××	Brown to grey, sandy SILT with minor gravel. Sand is fine. Gravel is fine to medium. Non-plastic. Stiff, Massive to bedded.	Moist	GLACIAL TILL
	V		X	Total Depth = 5 m		

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COMMENT: Minor collapse in upper outwash gravel unit. Ground water - minor perched seepage at 3.9m along top of till (northern end of pit).

Logged By: GSH/JH
Checked Date:

Sheet: 1 of 1



EXCAVATION NUMBER:

TP 9

PROJECT: Northbrook	Village				Job Number: 200490	
LOCATION: See Site Plan	n		Inclination:	VERTICAL	Direction: 🥢	
EASTING:	mE	EQUIPMENT:	20 tonne digger	OPERAT	OR: Sandy	
NORTHING:	mN	INFOMAP NO.		COMPA	NY: Civil Construction	
ELEVATION:	m	DIMENSIONS:		HOLE START	ED: 19-Jan-1 <mark>7</mark>	
METHOD:		EXCAV. DATUM:		HOLE FINISH	IED: 19-Jan-17	

						GEOLOGICAL
SCALA PENETRATION	GROUNDWATER / SEEPAGE	DEРТН (m)	GRAPHIC LOG	SOIL / ROCK CLASSIFICATION, PLASTICITY OR PARTICLE SIZE CHARACTERISTICS, COLOUR, WEATHERING, SECONDARY AND MINOR COMPONENTS	WATER CONTENT	SOIL / ROCK TYPE, ORIGIN, MINERAL COMPOSITION, DEFECTS, STRUCTURE, FORMATION
	m	0.3	ζ.	Black, organic SILT.		TOPSOIL
	@ 1.3	0.9	× × >	Brown, silty SAND. Medium dense. Massive.	Wet	LOESS
	Seepage	1.2		Brownish grey, SAND with some silt. Sand is fine to medium. Medium dense.  Massive	aturated	LOESS
	5	1.9	X X X	Grey, SILT. Non-plastic. Very stiff. Weakly laminated.	Moist	POND SEDIMENT
		3.4		Brown to grey, sandy GRAVEL. Gravel is fine to medium. Sand is fine to coarse. Medium dense. Bedded.	Moist	OUTWASH GRAVEL
		3.6	4	Brown to grey, SAND. Sand is fine to medium. Medium dense. Bedded.	Moist	OUTWASH SAND
		4.2		Brown to grey, interbedded SAND & sandy GRAVEL. Sand is fine to medium. Gravel is fine to medium. Medium dense. Bedded.	Moist	OUTWASH SAND/GRAVEL
		4.8	000	Brown to grey, sandy GRAVEL with minor cobbles. Sand is medium to coarse. Gravel is fine to coarse. Medium dense. Bedded.	Moist	OUTWASH GRAVEL

Total Depth = 4.8 m

	COMMENT: Tes	t pit wa	ls collapsing. Minor seepage at 1.3m.	Logged By: GSH/JH
				Checked Date:
•				Sheet: 1 of 1



EXCAVATION NUMBER:

TP 10

PROJECT: Northbrook Villa	age				Job Number: 200	490
LOCATION: See Site Plan			Inclination:	VERTICAL	Direction:	
EASTING:	mE	EQUIPMENT:	20 tonne digger	OPERAT	OR: Sandy	
NORTHING:	mN	INFOMAP NO.		COMPA	NY: Civil Constructio	n
ELEVATION:	m	DIMENSIONS:		HOLE START	ED: 1-Feb-17	
METHOD:		EXCAV. DATUM:		HOLE FINISH	ED: 1-Feb-17	

						GEOLOGICAL
SCALA PENETRATION	GROUNDWATER / SEEPAGE	DEРТН (m)	GRAPHIC LOG	SOIL / ROCK CLASSIFICATION, PLASTICITY OR PARTICLE SIZE CHARACTERISTICS, COLOUR, WEATHERING, SECONDARY AND MINOR COMPONENTS	WATER CONTENT	SOIL / ROCK TYPE, ORIGIN, MINERAL COMPOSITION, DEFECTS, STRUCTURE, FORMATION
		0.3	<b>,</b>	Dark brown, organic SILT with roots.	Moist	TOPSOIL
		0.9	, X, Y, X,	Brown, silty GRAVEL with some boulders. Gravel is fine to coarse. Boulders max. size 700mm. Medium dense. Massive.	Moist	GLACIAL TILL
		1.4		Grey, SAND. Sand is fine. Medium dense. Bedded.	Moist	OUTWASH SAND
		2.0		Grey, SAND with minor boulders. Sand is medium to coarse. Boulders max. size 300mm. Medium dense. Bedded.	Moist	OUTWASH SAND
		3.8		Brown, sandy GRAVEL with minor silt. Sand is medium to coarse. Gravel is fine to coarse. Medium dense. Bedded.	Moist	OUTWASH GRAVEL
		4.1	0.7	Grey, sandy GRAVEL. Sand is fine to coarse. Gravel is fine to coarse. Medium dense.	Moist	OUTWASH GRAVEL
	C	5.0		Grey, sandy GRAVEL. Sand is fine to coarse. Gravel is fine to medium. Medium dense. Bedded.	Moist	OUTWASH GRAVEL
	X	5.2	à q	Grey, gravelly SAND. Sand is fine to medium. Gravel is fine. Medium dense. Bedded.	Moist	OUTWASH SAND
S	NO SEEPAGE	6.2		Grey, sandy GRAVEL. Sand is fine to medium. Gravel is fine to medium. Medium dense. Bedded.	Moist	OUTWASH GRAVEL

Total Depth = 6.2 m

COMMENT:	Logged By: GSH/JH
	Checked Date:
	Sheet: 1 of 1



EXCAVATION NUMBER:

TP 11

PROJECT: Northbrook Vill	age		•	•	Job Number: 200490	1
LOCATION: See Site Plan			Inclination:	VERTICAL	Direction:	
EASTING:	mE	EQUIPMENT:	20 tonne digger	OPERAT	OR: Sandy	
NORTHING:	mN	INFOMAP NO.		COMPA	NY: Civil Construction	
ELEVATION:	m	DIMENSIONS:		HOLE START	ED: 1-Feb-17	
METHOD:		EXCAV. DATUM:		HOLE FINISH	IED: 1-Feb-17	

				EXCAV. DATOWI:		
						GEOLOGICAL
SCALA PENETRATION	GROUNDWATER / SEEPAGE	DEРТН (m)	GRAPHIC LOG	SOIL / ROCK CLASSIFICATION, PLASTICITY OR PARTICLE SIZE CHARACTERISTICS, COLOUR, WEATHERING, SECONDARY AND MINOR COMPONENTS	WATER CONTENT	SOIL / ROCK TYPE, ORIGIN, MINERAL COMPOSITION, DEFECTS, STRUCTURE, FORMATION
		0.3	۲,	Brown, organic SILT with rootlets.	Moist	TOPSOIL
		1.1		Brown, GRAVEL. Gravel is fine to coarse. Medium dense. Bedded.	Moist	OUTWASH GRAVEL
		3.1		Grey/brown, sandy GRAVEL. Sand is fine to coarse. Gravel is fine to coarse. Medium dense. Bedded.	Moist	OUTWASH GRAVEL
		3.5	9.4	Grey, gravelly SAND. Sand is fine. Gravel is fine to medium. Medium dense. Bedded.	Moist	OUTWASH SAND
		4.3		Grey, sandy GRAVEL. Sand is medium to coarse. Gravel is fine to coarse. Medium dense. Bedded:	Moist	OUTWASH GRAVEL
2	NO SEEPAGE	6,4	$\times \times $	Brown, gravelly silty SAND. Gravel is fine to medium. Sand is fine to coarse. Medium dense. Massive.	Moist	GLACIAL TILL

Total Depth = 6.4 m

COMMENT:	Logged By: GSH/JH
	Checked Date:
	Sheet: 1 of 1



EXCAVATION NUMBER:

TP 12

PROJECT: Northbrook Vill	age		•	•	Job Number: 200490	1
LOCATION: See Site Plan			Inclination:	VERTICAL	Direction:	
EASTING:	mE	EQUIPMENT:	20 tonne digger	OPERAT	OR: Sandy	
NORTHING:	mN	INFOMAP NO.		COMPA	NY: Civil Construction	
ELEVATION:	m	DIMENSIONS:		HOLE START	ED: 1-Feb-17	
METHOD:		EXCAV. DATUM:		HOLE FINISH	IED: 1-Feb-17	

<u> </u>		VILTITOD.		LACAV. DATOWI.		
						GEOLOGICAL
SCALA PENETRATION	GROUNDWATER / SEEPAGE	DEРТН (m)	GRAPHIC LOG	SOIL / ROCK CLASSIFICATION, PLASTICITY OR PARTICLE SIZE CHARACTERISTICS, COLOUR, WEATHERING, SECONDARY AND MINOR COMPONENTS	WATER CONTENT	SOIL / ROCK TYPE, ORIGIN, MINERAL COMPOSITION, DEFECTS, STRUCTURE, FORMATION
		0.3	<b>;</b> >	Dark brown, organic SILT with rootlets.	Moist	TOPSOIL
		0.6	X	Brown, SILT with rootlet voids. Non-plastic. Stiff. Massive.	Moist	LOESS
		1.4		Brown, sandy GRAVEL with minor cobbles. Sand is fine to coarse. Gravel is fine to coarse. Medium dense. Bedded.	Moist	OUTWASH GRAVEL
		1.0		Grey, SAND, Sand is fine, Medium dance, Redded, Grey, sandy GRAVEL with minor boulders, Sand is fine to coarse. Gravel is fine to	1	OUTWASH SAND OUTWASH GRAVEL
		4.2		coarse. Boulders max. size 300mm. Medium dense. Bedded.	Moist	
		4.4		Grey, SAND. Sand is fine to medium. Medium dense. Bedded.	Moist	OUTWASH SAND
		4.9	0 V	Grey, sandy GRAVEL. Sand is fine to coarse. Gravel is fine to coarse. Medium dense. Bedded.	Moist	OUTWASH GRAVEL
	V	5.5	X X X X	Brown to grey, SILT with thin silty sand & sand beds. Sand is fine. Non-plastic. Firm. Laminated.	Moist	OUTWASH SILT
	ш	6.3		Grey, sandy GRAVEL. Sand is medium to coarse. Gravel is fine to coarse. Medium dense. Bedded.	Moist	OUTWASH GRAVEL
X	NO SEEPAGE	7.0		Grey, SAND. Sand is fine to medium. Medium dense. Bedded.	Moist	OUTWASH SAND
				Total Depth = 7 m		

Total Depth = 7 m

COMMENT:	Logged By: GSH/JH
	Checked Date:
	Sheet: 1 of 1



**EXCAVATION NUMBER:** 

TP 13

	PROJECT: Northbro	JOB N	UMBER: 200490					
F	ASTING:		mE	EOUIPMENT:	35 T Excavator	OPI	RATOR:	Matiu
	RTHING:		mN	INFOMAP NO.			MPANY:	Civil Construction
	EVATION:		m	DIMENSIONS:		HOLE S		8-Apr-19
Ν	METHOD:			EXCAV. DATUM:		HOLE FI	NISHED:	8-Apr-19
DEРТН (m)	SOIL / ROCK TYPE	GRAPHIC LOG	Database	DESCRIPTIO	io	USCS GROUP	GROUNDWATER / SEEPAGE	SCALA PENETROMETER
0.3	TOPSOIL	$\mathbf{x}_{\mathbf{x}}^{\mathbf{z}}$	moist.		s. Silt is non-plastic. Dry to			
0.7	COLLUVIUM	$\stackrel{X}{\overset{X}}{\overset{X}{\overset{X}}{\overset{X}{\overset{X}{\overset{X}}{\overset{X}{\overset{X}}{\overset{X}{\overset{X}}{\overset{X}{\overset{X}{\overset{X}}{\overset{X}{\overset{X}}{\overset{X}}{\overset{X}{\overset{X}}{\overset{X}}{\overset{X}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}{\overset{X}}}{\overset{X}}}{\overset{X}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}{\overset{X}}{\overset{X}}}}{\overset{X}}}{\overset{X}}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}}{\overset{X}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset{X}}{\overset{X}}}{\overset{X}}}{\overset{X}}}{\overset$		n. Silt is non-plastic.	avel and trace rootlets. Sand Loose to medium dense.			
3.0	GLACIAL TILL	$\times$ $\times$ $\times$ $\times$ $\times$ $\times$ $\times$ $\times$	and boulders. Sa is non-plastic. M	nd is fine to medium ledium dense. Massi	s fine to medium. Gravel is o-angular. Silt is non-plastic.			
4.0	0.						NO SEEPAGE	
<b>▼</b>	11/10		Total Depth = 4 m				<u>- 1</u>	

COMMENT:

Logged By: MDP
Checked Date:
Sheet: 1 of 1



COMMENT:

## **EXCAVATION LOG**

**EXCAVATION NUMBER:** 

TP 14

Logged By: MDP

Sheet: 1 of 1

Checked Date:

PROJECT: Northbrook Village  LOCATION: INCLINATION:									200490
	EASTING:		mE	EQUIPMENT:	35 T Excavator	OPE	RATOR:	١	/latiu
	ORTHING:		mN	INFOMAP NO.			MPANY:	_	nstruction
	EVATION:		m DIMENSIONS: HOLE S						Apr-19
	METHOD:			EXCAV. DATUM:		HOLE FI	NISHED:	8-7	Apr-19
DEРТН (m)	SOIL / ROCK TYPE	GRAPHIC LOG	DESCRIPTION						CALA PROMETER
0.0	TOPSOIL	$_3^{3} \times$	Dark brown, orga	anic SILT with rootlet	s. Silt is non-plastic. Moist.	5			
0.3	COLLUVIUM		fine to coarse ar	nd sub-rounded to su	d is fine to coarse. Gravel is b-angular. Loose to medium				
0.7	GLACIAL TILL		dense. Massive.	_	d trace cobbles and boulders.				
4.4			Sand is fine to m Massive. Dry to	nedium. Silt is non-pla moist.			NO SEEPAGE		
			Total Depth = 4.4 i	n					

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	-	-2	Silty/ sandy fin greyish brown subangular, so thin lenses of OUTWASH)	. Gravel, fir chist and q	ne to co uartz, s	oarse, su sand, fine	ibrounde to coai	ed to rse. Inc		\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	); 							5					
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14mm GEOPROBE 8140LC	3.04	1 dees	× · · · · \ da / 3.	90m - 3.04m SiLT with minor sand ark brown mottling. "Soft"; moist; lot .04m - 6.00m Sandy SiLT with som et; sand fine to medium; gravel, finubangular schist and quartz	w plasticity; san e gravel; darkig	d, fine. preyish brown. "Soft to fin	n";	10,	T 3.04 m 11, 10, 9, 11; N = 40	SPT - Rec: 39 some gravel.	0mm; Sandy SILT with
SONIC114	4.56	Slow	· · · × · · · · · · · · · · · · · · · ·	56m - 4.80m Sand lense (medium					「4.56 m , 5, 5, 7, 6; 23	SPT - Rec: 38 SAND.	Omm; Medium to coarse
2	7.60	2	Si   Si   Si   Si   Si   Si   Si   Si	00m - 6.08m SILT with minor clay, irim"; moist to wet; low plasticity; dil ubangular, schist and quartz; sand, 08m - 6.10m CORE LOSS. 10m - 7.00m Sandy, SILT with som range brown weathering. "Firm"; no ravet, fine to medium (rare coarse), uartz. 00m - 9.34m Silty sandy fine to me ight orange brown weathering. Gra ubangular, schist and quartz; sand, 60m - 9.12m Fine to medium (rare uartz. Very dense.	atant; gravel, fir fine.  e gravel; dark g n-plastic; dilatar subrounded to dium GRAVEL; vel, fine to medi fine to coarse.	ne to medium, subrounder greyish brown with slight nt; sand, fine to medium; subangular, schist and dark greyish brown with ium, subrounded to	ed to /	SP** 5,1 7; N	F 6.08 m 0, 8, 7, 7, 1 = 29	SPT - Rec: 34 some gravel.	0mm; Sandy SILT with
E 1	xpla		0 X 8 0×5 0x5 ns:	Soil Types:		Backfill:	Remarks 1. PQ sized core	15, Nc	14; = 60	ushed with potable	water.
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	Wa Ris Sm Lar	ter Ris e Time all Dis	ike (1st, 2nd) se (1st, 2nd) s (minutes) turbed Sample turbed Sample	and SILT (SDS) SAND	TOPSOIL PEAT FILL	Bentonite  Grout  Drill arisings or collapsed hole  Filter material	Cohesive soil marks.     Sample moist for non-cohesive 5. Core loss attrextraction (suction. SPT tests: continued to the suction of the second s	strength ter ture contents soils; base ibuted to on on). mmenced a	ms based on fie s: soil moisture l d on field descri e or combination pprox. 70mm be	eld description of d levels affected by of iption of drill arising n of compression of alow end-of-run de	inless stated.  Ill arisings and indicated in thill arisings and indicated in thill method (500L added); as for cohesive soils. In soil or outflow of materia oths; hammer energy ratio Rec"= recovered sample is

Project:   Northisko Development   Wanda, Olgo   Sarr Date:   Wanda, Olgo   Sarr Date:   Ol-12-15   Ground Level (m.LNZ):   Co-Ordinates (NZTM2000):   E 375,808.3   N 808,484.6   E   E   Sarr Date:   Ol-12-15   Ground Level (m.LNZ):   Co-Ordinates (NZTM2000):   E 375,808.3   N 808,484.6   E   Sarr Date:   Ol-12-15   Ground Level (m.LNZ):   Co-Ordinates (NZTM2000):   E 375,808.3   N 808,484.6   E   Sarr Date:   Ol-12-15   Ground Level (m.LNZ):   Co-Ordinates (NZTM2000):   E 375,808.3   N 808,484.6   E   Ol-12-15   Ground Level (m.LNZ):   Ol-12-15   Ground Level (m.LNZ)	C			E SULTANTS	395 Chris	ey Consultants Lim Madras Street, Christchurch stchurch 8013 03 3794402 : 03 3794403	ited								ı	<b>30</b>	RE	НО	LE LC	G	
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For a second point of the	Type		Fluid & Water		Rock L fabric a Soil de Streng G Soil de beddin	Description (where applicate and orientation; strength (Goscaphicate) (Goscaphica	le): Weat EOLOGIO ): Fraction tional (GE n <u>(reco</u> ): Fractior icity; addi	n (MAJO EOLOG OVEREC n (MAJO itional	OR/m GICAL U d as): OR/mi	inor); col JNIT) : inor); col	our; bedd our;	xture; ling.	Elevation (m LINZ)	Depth (m)	Tes	ts	5	Sample	Description	0	Backfill /
EOH @ 10.64 m  FOH @ 10.64 m  The second of	SONIC114mm GEOPROBE 8140LC				9.12m - 9.34m - brown.	12m bgl. (GLACIAL TILL)  9.34m CORE LOSS; very  10.64m Fine to medium (r Gravel predominately subr	dense.	e) GRA	VEL w	ith minor	· silt/sand;			_	8, 12, 13, 20, 10 (430mm);	19,		) XC/		)	
Explanations:    Water Strike (1st, 2nd)   CLAY   TOPSOIL   Bentonite   Bentonite   Sill T   PEAT   Grout   Fill   PEAT   Standard Penetration Test (SPT)   GRAVEL   NO RECOVERY*   Filter material   No Recovery*   Rig/Plant Used:   Driller:   Logaed by: Check			S		EOH @	2 10.64 m							2	-13	7, 10, 10, 10, 13 (410mm);	10,	(rare co	oarse) G			
Explanations:  Soil Types:  Backfill:  PEAT  Fill Bentonite  Small Disturbed Sample (SDS)  Large Disturbed Sample (LDS)  Standard Penetration Test (SPT)  Standard Penetration Test (SPT)  Remarks  1. PQ sized core; 95mm diameter casing; flushed with potable water. 2. Heave (up-flow of material into casing) minimal (<100mm) unless stated. 3. Cohesive soil strength terms based on field description of drill arisings and indicated in quarks. 4. Sample moisture contents: soil moisture levels affected by drill method (500L added); not for non-cohesive soils; based on field description of drill arisings for cohesive soils. 5. Core loss attributed to one or combination of compression or soil or outflow of material due to some or combination of compression or soil or outflow of material due to none or combination of compression or soil or outflow of material due to none or combination of compression or soil or outflow of material due to none or combination of compression or soil or outflow of material due to none or combination of compression or soil or outflow of material due description of drill arisings and indicated in quarks. 4. Sample moisture contents: soil moisture levels affected by drill method (500L added); not for non-cohesive soils; based on field description of drill arisings and indicated in quarks. 5. Core loss attributed to one or combination of compression or soil or outflow of material due description of drill arisings and indicated in quarks. 5. Sample moisture contents: soil moisture levels affected by drill method (500L added); not for non-cohesive soils; based on field description of drill arisings and indicated in quarks. 5. Score loss attributed to one or combination of compression or soil or outflow of material due to none or combination of compression or soils. 5. Core loss attributed to one or combination of compression or soil or outflow of naterial due to none or combination of compression or soil or outflow of naterial due to none or combination of compression or soil or outflow of naterial		Y												-16 -						-	
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**Appendix C: B F Whitham - Stage 15 Geotechnical Completion Report** 

1 Hawkdun Place ALEXANDRA 9320 Tel 64 3 448 8503 Mobile \$ 9(2)(a)

Email: s 9(2)(a)

### GEOTECHNICAL COMPLETION REPORT

### NORTHLAKE SUBDIVISION STAGE 15

AUBREY / OUTLET ROAD, WANAKA

for

NORTHLAKE INVESTMENTS LTD

Date: 7 February 2020 Reference: B 66212-11

#### 1. Introduction

B F Whitham Ltd has been engaged by Civil Construction Ltd to oversee the construction on stages 1-15 of the Northlake Subdivision at Aubrey and Outlet Roads in Wanaka.

This report relates to stage 15 of the subdivision (excluding the carriageways).

This report includes:

- a) A review of all previous geotechnical reports prepared for this site and their recommendations,
- b) An overview of the earthworks completed on site including
  - i. excavations and fill operations
  - ii. fill certification in accordance with NZS4431 for the reinstatement of the temporary storm water retention pond affecting lots 5001 5009.
- c) Recommendations regarding suitability for foundation design within stage 15 as required by Section 2.6.1 of the Queenstown Lakes District Council Land Development and Subdivision Code of Practice (LDSCOP)

#### 2. Site description

Stage 15 – is described as 175 lots being numbered 5001 – 5175

Stage 15 is formed in excavation and fill. Some of the Lots had cut and fill operations.

- The excavated lots were Lots 5038 5047, 5053 5098, 5100, 5101, 5110 5114, 5166 5172) with cuts up to 9.5m deep. The excavated material was utilised in the fill areas in Stage 15 and elsewhere within the subdivision.
- The filled lots were Lots 5001 5037, 5049 5052, 5127 5143, 5161 5164.
- Lots 5001 5009 were utilised as a temporary stormwater retention pond area for the subdivision. This area was subsequently undercut and backfilled as per NZS 4431. The maximum backfilled depth of 5.0m was within this retention pond area.
- Lots 5001 5020 along the Outlet Road boundary has a 1.0m high timber post and board retaining wall designed and checked by others.
- Lots 5021 5032 have been tested and certified by Ground Consulting Ltd to be have an ultimate bearing capacity of 300kPa from finished ground level.

#### Refer to Appendix B for

- Stage 15 As built cut-fill plan RM 171190 (Paterson Pitts Group ref W5431-014-100a 4 dated 17 January 2020). With NDM test locations
- Stage 15 As built cut-fill plan SDP B & C RM 171190 (Paterson Pitts Group ref W5431-014-100b 4 dated 17 January 2020). With NDM test location.
- Stage 15 NDM Testing results sheet (Paterson Pitts Group ref W5431-014-101 rev 1 dated 17 January 2020)
- Stage 15 As built contours RM 171190 (Paterson Pitts Group ref W5431-014-102a and 102b rev 2 dated 20 January 2020)
- Stage 15 Scala test locations RM 171190 (Paterson Pitts Group ref W5431-014-102a and 102b rev 2 dated 20 January 2020)
- Stage 15 Scala test results

From the as built contour plan it can be observed stage 15

- Generally has a gentle topography of approximately 1 in 50 from east to west.
- Lots 5001 5020 along outlet Road have a batter slope to the boundary of 3H:1V with an overall height differential of some 2.0 metres. These lots are subject to a separate contract subsequent to the earthworks contract that included a 990mm high retaining wall and backfill slope of 3H:1V.

There is no sign of slope instability.

There is no trace of water table.

#### 3. Scope of works

This report included a desk top study of previous geotechnical reports, site inspections during construction, and review of the testing regime and test results.

The issue of a Schedule 2A certificate with conditions as required, is attached as Appendix A

#### 4. Previous reports

Previous reports relevant to this area of stage 15 are listed below and are on the Queenstown Lakes District files and attached to previous Geotechnical Completion Reports for this subdivision.

- 1. Riley Consultants geotechnical assessment report 15832-B dated 17 February 2016
- 2. Geosolve Ltd Geotechnical Report Northlake Subdivision subzone area B2, B3 and C1, Outlet Road, Wanaka dated August 2017.

Riley Consultants Ltd (Riley) report 15832-B dated 17 February 2016 was a geotechnical assessment prepared in support of the ODP application approved by the Queenstown Lakes District Council in RM160152.

As part of their assessment Riley carried out shallow subsurface geotechnical investigations including 32 hand auger boreholes to 3.0 metres deep and 42 shallow test pit investigations including in-situ strength testing with shear vane and scala penetrometer testing where appropriate. Also deep ground investigations including four rotary-sonic machine boreholes (two at 10m depth and two at 15m depth) with in-situ soil strength tests by standard penetration test (SPT).

The shallow subsurface geotechnical investigations relevant to stage 15 are listed below with the geotechnical ultimate bearing capacity listed. This table below is from page 8 of the report and the inspection logs in Appendix A of that report.

	Investigation ID	200kPa (m below topsoil)	300kPa (m bgl)
Lot 5017 approx	HA2	0.0	0.6
Lot 5144 approx	HA3	0.0	0.2
Lot 5010 approx	TP1	These test pits TP1 and TP2	
Lot 5014 approx	TP2	metre deep into sandy silt g	ravel (glacial till)

B F Whitham Ltd Ref: 66212 – 11

Riley stated in their executive summary "the site was found to be generally suitable for lightweight structures with shallow foundation types, such as residential housing. An ultimate bearing capacity of 200kPa consistently available across the site from below the topsoil, an ultimate bearing capacity of 300kPa was available below 1.0m across most of the site" This statement was for the whole site but the local test results for Stage 15 indicate an ultimate bearing capacity of 300kPa at 0.2 metres to 0.6 metres from the finished level.

The Geosolve Ltd report dated August 2017 reports on some of the area in stage 15. Test pits relevant to stage 15 are TP3, TP22, TP23 and TP25.

The results from these test pits are consistent with the Riley report and what was encountered on site. Namely the area had some 0.2 metres of topsoil overlaying a 0.3-0.6 metre layer of colluvium on top of glacial till.

From these reports it can be generally accepted the subgrade over stage 15 will achieve the ultimate bearing capacity of 300kPa.

#### 5. Site works

#### 5.1 Site works

The extent of the construction works area in the stage 15 area was the excavation down to the required design levels, with the depth varying from original ground level to 9.5 metres in depth and the backfilling to grade design levels with a maximum fill depth of 5.5m. Refer to appendix A - Stage 15 As built cut-fill plan.

The underlying fill subgrade was a silty/sand/gravel colluvium which is consistent with the findings contained in the Riley report.

The area required for filling was prepared as per Civil Construction Ltd method of construction. The subgrade was cleared, compacted and tested by scala penetrometer, as per standard practice, to confirm an allowable bearing capacity of 100kPa prior to any fill being placed. The temporary pond area (Lots 5001 – 5009) was stripped out of the pond sealing layer to a subgrade of alluvial gravels.

No indication of ground water was observed during the pre-fill subgrade inspections or during the earthworks filling operations. Accordingly, it was assessed that there was no requirement for under-fill drainage

The backfill was placed and constructed in terms of NZS4431:1989. The fill was placed in 200 – 300mm layers and compacted with a 16 ton vibrating drum compactor. The fill was also further compacted with construction traffic of fully laden rubber tyred 50 ton dump trucks.

The fill material used was clean sandy silt and silty gravelly material sourced from within the stage 15. This material as reported by:

- 1. Riley "is unlikely to be subject to seasonal wetting/shrink-swell effects and can be categorised as non-expansive Class A soils in terms of AS2870".
- 2. Geosolve in section 5.3 Site preparation / earthworks this material "colluvium can be used as engineered fill on site".

The lots adjacent to Outlet Road (5001 - 5032) have a fill batter sloping down at 3H:1V to the boundary. For lots 5001 - 5020, a contract subsequent to the earthworks contract, constructed a retaining wall some 0.9 metres high. It is recommended any foundation within 10 metres of this Outlet Road boundary of these Lots 5001 - 5032 be assessed by a CPEng professional.

#### 5.2 Testing regime

As stated above the fill subgrade area was tested prior to filling and thereafter testing was carried out for compaction compliance with relative compaction testing. This was undertaken at or in excess of the frequency recommended in NZS4431:1989 "Code of practice for earth fill for residential development" as recommended by the Riley report (section 9.1).

Refer to Appendix B for

- 1. Stage 15 As built cut-fill plan RM 171190 (Paterson Pitts Group ref W5431-014-100a and 100b 4 dated 17 January 2020). With NDM test locations
- 2. Stage 15 NDM Testing results sheet (Paterson Pitts Group ref W5431-014-101 rev 1 dated 17 January 2020)
- 3. Stage 15 Scala test locations ref W5431-014-102a and 102b rev 2 dated 20 January 2020". The scala penetrometer testing showed a subgrade with a working bearing capacity of 100kPa or an ultimate bearing capacity of 300kPa.
- 4. Scala test results

The NDM Relative Compaction tests were carried out by Central Testing Ltd. Of the 152 test results 6 were at 94% and 3 at 93%. These 9 tests results below 95% are considered to be within the range allowed by NZS4431. The 143 other tests results exceed the 95% minimum allowable in terms of NZS4431:1989.

As stated above, the completed surface of stage 15 was tested by scala penetrometer. Each Lot had two locations tested and the results proved an allowable bearing capacity of 100kPa which equates to an ultimate bearing capacity of 300kPa.

#### 6. Conclusion and recommendation

NZS 3604:2011 clause 3.3.8 prescribes that for "good ground" each lot shall have a minimum four penetrometer tests to confirm the bearing capacity. This testing programme is for an individual lot in isolation and was not carried out.

However, from the NDM testing regime carried out, and two scala penetrometer tests per lot, it can be concluded that all the sites within Stage 15 satisfy the definition of "good ground" within NZS3604:2011.

As stated above Lots 5001 – 5032 adjacent to Outlet Road have a sloping fill batter and any foundation in this area shall be assessed by a CPEng professional.

On the basis of:

- 1. The construction method undertaken, machinery and plant used,
- 2. The material used in the construction
- 3. All fill compaction testing being undertaken at or in excess of the frequency recommended in NZS4431:1989 and satisfying the 95% requirement.

4. The scala penetrometer testing regime upon completion proving all tests exceeded 100kPa allowable bearing capacity

I believe on reasonable grounds that Stage 15 can be classified as "good ground" as having an allowable bearing capacity of 100kPa or greater.

As per clause 21(1) of the Resource Consent RM160509, a Schedule 2A certificate as per NZS4404:2010 "Land Development and Subdivision Infrastructure" is attached as Appendix A.

Please also note the standard clause 4 of the Schedule 2A certificate.

Yours faithfully

Bernard Whitham

MIPENZ (Civil), CPEng IntPE, Regn no. 58201, MBA (Tech Mgt.)

#### 7. Appendices

Appendix A – Schedule 2A certificate

#### Appendix B -

- Stage 15 As built cut-fill plan RM 171190 (Paterson Pitts Group ref W5431-014-100a 4 dated 17 January 2020). With NDM test locations
- Stage 15 NDM Testing results sheet (Paterson Pitts Group ref W5431-014-101 rev 1 dated 17 January 2020)
- Stage 15 As built cut-fill plan SDP B & C RM 171190 (Paterson Pitts Group ref W5431-014-100b 4 dated 17 January 2020). With NDM test location.
- Stage 5 As built contours RM 171190 (Paterson Pitts Group ref W5431-014-102a and 102b rev 2 dated 20 January 2020)
- Stage 15 Scala test locations RM 171190 (Paterson Pitts Group ref W5431-014-102a and 102b rev 2 dated 20 January 2020)
- Stage 15 Scala test results

#### **SCHEDULE 2A**

## STATEMENT OF PROFESSIONAL OPINION ON SUITABILITY OF LAND FOR BUILDING CONSTRUCTION

Dev	/elopi	ment	Northla	ke Subdivis	ion, Wanaka -	- Stage 15				
Dev	/elope	er	North	lake Invest	ments Ltd	***************************************	**********			
Loc	ation		Aubrey	Road / Ou	ıtlet Road, Wa	anaka				
١	Bern	ard Fran	cis Whitham		of	BFW	hitham Ltd,	22 Clty Road	I, Dunedin	
		(Fi	ull name)				(Name a	nd address o	of firm)	′ . (
Her	eby c	confirm th	nat:						5	
1.			ofessional a onal on the		n clause 1.2.2 relopment.	of NZS 4404	4:2010 and	was retained	by the deve	loper as the
2.	date beer the r	d re-eval results of	uated in the	, and the preparation pre-evolution pre-ev	stigations are conclusions on of this repo valuations car 66212/11 and	and recoment. The exteried out are	mendation ent of my in as descri	ns of that/tho aspections du bed in my ge	ose docume uring constr	ruction, and
3.	In m	The ear	th fills show	n on the at	oe construed W54 ttached Plan I 431:1989	31-014-100a No	a and 100b have	been placed	in complian	nce with the
	(b)	the app	ended four	dation red	o account lan commendation d final site co	ns and eart	hworks res	-		*
	(c)	erection	of building ereport 662	s designed 12 / .1.1 all	s Schedule, to d according to lots, satisfy, re	NZS 3604 equirements.	provided t fac."gaad g	hat:	Ü	
	(d)	designe	d according	to NZS 3	ois Schedule, 604 provided lots satisfy re uiring foundati	that:	for "good a	round" as per	NZS3604 S	Section 1
7	(e)	(ii) The orig	inal ground	not affect	ed by filling ar	nd the filled (	ground are	not subject to	o erosion, s	ubsidence,
C X	X	or slippo provide (i) (ii)	d that:	not applicat	h the provisio				Manageme	nt Act 1991

NOTE - These subclauses may be deleted or added to as appropriate, to include such considerations as expansive soils

where excluded from NZS 3604, and site seismic characteristics as covered in clause 3.1.3 of NZS 1170.5.

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4,	This professional opinion is furnished to the TA and the developer for their purposes alone on the express
	condition that it will not be relied upon by any other person and does not remove the necessity for the
	normal inspection of foundation conditions at the time of erection of any building.

This certificate shall be read in conjunction with my geotechnical report referred to in clause 2 above and shall not be copied or reproduced except in conjunction with the full geotechnical completion report.

Bernard Whitham

CPEng IntPE, MBA (Tech Mgt)

(Name, title, and professional qualifications)

Date .....7 February 2020

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#### **APPENDIX B**

### Paterson Pitts Group drawings

- Stage 15 As built cut-fill plan RM 171190 (Paterson Pitts Group ref W5431-014-100a 4 dated 17 January 2020). With NDM test locations
- Stage 15 As built cut-fill plan SDP B & C RM 171190 (Paterson Pitts Group ref W5431-014-100b 4 dated 17 January 2020). With NDM test location.
- Stage 15 NDM Testing results sheet (Paterson Pitts Group ref W5431-014-101 rev 1 dated 17 January 2020)
- Stage 15 As built contours RM 171190 (Paterson Pitts Group ref W5431-014-102a and 102b rev 2 dated 20 January 2020)
- Stage 15 Scala test locations RM 171190 (Paterson Pitts Group ref W5431-014-102a and 102b rev 2 dated 20 January 2020)
- Stage 15 Scala test results

20,0





	Stage	15 NDM T	esting	
Test ID	Northing (m)		RL (m)	Relative Compaction
18101217	806665.1	375569.0	336.5	96.4
18101218	806667.0	375587.4	336.3	96.8
18101219	806671.1	375612.2	336.1	94.7
18101220	806665.4	375638.2	335.7	97.3
18101221	806674.5	375661.4	335.6	95.8
18101222	806683.2	375674.5	334.9	95
18101223	806689.9	375637.2	335.9	95.7
18101224	806692.2	375611.9	336.4	96
18101225	806687.3	375587.9	336.5	95.5
18101226	806687.2	375567.2	336.8	98.5
18112801	806841.6	375837.8	329.3	96
18112802	806850.8	375825.2	329.4	95,3
18112803	806862.9	375808.0	329.5	98
18112804	806873.6	375793.3	329.7	97
18112805	806884.4	375780.9	329.7	96.3
18112806	806893.6	375767.5	329.8	98.9
18112807	806907.2	375751.6	329.9	98.9
18112808	806917.5	375739.7	329.9	100.7
18112809	806928.8	375726.5	330.0	95.9
18112810	806943.0	375709.1	330.1	104.3
18112811	806960.8	375717.6	329.5	101.4
18112812	806941.6	375744.1	329.3	101
18112813	806932.3	375755.3	329.3	98.5
18112814	806925.8	375764.0	329.2	99.7
18112815	806913.9	375777.9	329.2	99.8
18112816	806903.9	375790.7	329.1	100.5
18112817	806889.7	375808.2	329.0	96.8
18112818	806881.3	375820.3	328.9	100
18112819	806868.0	375838.7	328.8	100.6
18112820	806849.5	375862.4	328.4	97.8
18113001	806995.2	375543.4	329.7	100
18113002	806989.6	375559.6	329.7	103
18113003	806980.7	375576.5	329.8	100
18113004 18121401	806973.7	375596.1	329.8	102
18121401	806817.4	375818.4	329.7	98
18121402	806847.0	375778.7	330.1	98
18121403	806865.3	375754.2	330.2	97
	806880.4	375733.7	330.4	97
18121405	806887.5	375724.3	330.4	98
18121406	806904.7	375701.9	330.5	97
19011602 19011603	806992.6	375550.0	330.3	98
19011603	806990.0 806975.6	375570.4	330.2	102
19011604	806973.6	375584.8 375604.5	330.1 330.2	104
19011606	806949.3	375615.8	330.2	98
19011607	806926.7	375621.0	331.8	96 97
19011608	806940.8	375637.0	330.9	94
19011609	806932.6	375658.0	330.9	93
19011610	806950.4	375666.4	330.4	96
19011610	806912.8	375647.6	331.9	
19070406	806637.7	375572.0	334.8	97 104
19070407	806647.6	375618.5	334.4	104
181114254	806725.9	375690.7	334.9	98
181114255	806712.6	375690.3	334.9	95
181130272	806785.9	375687.7	333.5	95
181130273	806779.4	375704.9	333.2	98
181130274	806791.3	375713.4	331.8	100
181130275	806802.5	375698.1	332.0	95
181211286	806812.8	375828.7	329.6	96
181211287	806838.8	375787.4	330.1	96
181211288	806857.7	375763.1	330.2	97
181211289	806869.8	375747.2	330.3	98
181211290	806893.2	375718.5	330.4	98
190122297	806871.2	375697.4	331.0	97
190122298	806857.9	375719.5	330.7	96
190122299	806838.4	375736.4	330.6	95
190122300	806833.4	375766.1	330.5	94
190129301	806827.0	375765.3	331.2	96
190129302	806836.0	375744.6	331.5	94
190129303	806852.8	375734.9	331.6	94
190129304	806868.0	375712.2	331.9	94
190129305	806878.7	375695.1	332.1	97
190129306	806890.5	375666.0	332.4	98
	806899.0	375602.1	332.6	97
190129308	000055.0			
190129308 190129309	806937.8	375644.9	331.1	95
		375644.9 375665.4	331.1 330.8	95 100

Stage 15 NDM Testing								
Test ID	Northing (m)	Easting (m)	RL (m)	Relative Compaction				
190129312	806947.0	375573.1	332.4	98				
190129313	806968.5	375561.1	332.1	94				
190129314 190129315	806963.8 806960.7	375599.3 375631.9	331.2 330.5	96				
190129315	806960.7	375697.8	330.5	102 93				
190129317	806971.7	375671.3	330.2	98				
190711477	806652.8	375658.6	335.6	103				
190711478	806646.9	375616.1	335.6	104				
190711479	806640.5	375583.2	336.0	98				
CT1	806918.2	375653.7	329.3	95				
CT2 CT3	806945.0 806972.6	375660.3 375674.1	329.1 328.9	98.9 97.1				
CT4	806982.0	375645.4	328.8	96.2				
CT5	806963.7	375629.2	328.9	95.1				
СТ6	806939.9	375619.9	329.3	98.3				
CT7	807003.4	375608.6	328.1	97.7				
CT8	806978.1	375594.6	328.2	98.8				
CT9 CT10	806991.8 807023.8	375575.9	328.1	97.8				
CT12	807023.8	375575.8 375632.9	328.0 328.7	96.0 98.9				
CT13	806967.3	375697.7	328.7	96.7				
CT14	806962.8	375658.7	328.9	95.6				
CT15	806980.6	375624.3	328.8	95.8				
CT17	806978.1	375580.6	329.1	99.1				
CT18	806954.4	375642.7	329.1	97.7				
CT19 CT20	806939.1 806915.3	375688.5 375634.3	329.4	97.4				
CT21	806936.0	375641.2	330.0 330.0	98.7 96.8				
CT22	806949.7	375591.6	330.0	96				
CT23	806966.4	375563.4	330.0	99.6				
CT25	806929.7	375667.0	330.4	101.9				
CT26	806932.1	375625.9	330.7	97.6				
CT27	806949.5	375594.8	330.6	100.9				
CT28 CT179	806976.3 806670.9	375548.8 375663.2	330.9 335.4	93.3 97.8				
CT180	806670.0	375648.4	335.4	96.4				
CT181	806671.9	375632.4	335.6	100.0				
CT182	806667.0	375604.0	336.0	98.6				
CT183	806681.3	375609.5	336.1	100.8				
CT184	806665.1	375579.3	336.1	95.6				
CT254 CT255	806725.9 806712.6	375690.7 375690.3	334.9 334.9	99.1 96.2				
CT272	806785.9	375687.7	333.5	98.1				
CT273	806779.4	375704.9	333.2	102.8				
CT274	806791.3	375713.4	331.8	103.2				
CT275	806802.5	375698.1	332.0	100.3				
CT286	806812.8	375828.7	329.6	99.7				
CT287 CT288	806838.8	375787.4	330.1	97.3				
CT289	806857.7 806869.8	375763.1 375747.2	330.2 330.3	101.3 100.7				
CT290	806893.2	375718.5	330.4	100.7				
CT297	806871.2	375697.4	331.0	98.0				
CT298	806857.9	375719.5	330.7	98.8				
CT299	806838.4	375736.4	330.6	96.2				
CT300	806833.4	375766.1	330.5	97.2				
CT301 CT302	806827.0 806836.0	375765.3 375744.6	331.2 331.5	97.2 94.8				
CT302	806852.8	375744.6	331.5	94.8 96.8				
CT304	806868.0	375712.2	331.9	96.4				
CT305	806878.7	375695.1	332.1	99.3				
CT306	806890.5	375666.0	332.4	100.8				
CT307	806896.5	375651.9	332.5	97.0				
CT308 CT309	806899.0 806937.8	375602.1 375644.9	332.6	99.7				
CT310	806937.8 806926.4	375644.9 375665.4	331.1 330.8	97.7 100.4				
CT311	806937.4	375599.8	331.7	100.4				
CT312	806947.0	375573.1	332.4	99.8				
CT313	806968.5	375561.1	332.1	96.8				
CT314	806963.8	375599.3	331.2	100.5				
CT315	806960.7	375631.9	330.5	99.7				
CT316 CT317	806960.5 806971.7	375697.8 375671.3	330.1	96.7				
CT477	806971.7	375658.6	330.2 335.6	96.3 96				
CT478	806646.9	375616.1	335.6	96				
CT479	806640.5	375583.2	336.0	93				
CT478	806646.9	375616.1	335.6	96				

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NORTHLAKE wanaka Purpose & Drawing Title:

Stage 15 NDM Testing Results Sheet

Surveyed by:	PPG	Original Size:	Scale:					
Designed by:	77.		4	1:1500 @ A3				
Drewn by:	AGM	A3	1:1500 @ A3					
Checked by:	DA							
Approved by:	AGT		DO N	OT SCALE				
Job No: W5431	Onewing No: 014	Sheet No: 101	Revision No:	Date Created: 17/01/2020				







