



Geotechnical Investigation Report

43A Vipond Road & 20 Melia Place, Stanmore Bay
For Melia Development Limited

Reference: 2282
Date: 21 May 2021
Revision: B

Table of Contents

1	Introduction	1
2	Site Description	1
3	Proposed Development	4
4	Geological Map	4
5	Previous Geotechnical Report	5
6	Site investigation	5
7	Subsoil Conditions	5
8	Groundwater	6
9	Ground Stability	6
10	Earthworks	6
11	Recommendations	6
11.1	In-ground Palisade Wall	6
11.2	Earthworks	7
11.3	Foundations	8
11.3.1	Suspended floors on Pile Foundations	8
11.3.2	Concrete Slab Foundations	8
11.4	Retaining Walls	9
11.5	Construction Inspections	10
12	Limitations	11

Appendices

Appendix A: Draft Site Plans & Elevation

Appendix B: Investigation Logs

Appendix C: Slope Stability Analysis

Appendix D: SK-2282-01, SK-2282-02 & SK-2282-03

Released under the provision of
the Official Information Act 1982

1 Introduction

GeoStudio Ltd has been engaged to undertake a subsoil investigation and provide geotechnical recommendations for the proposed new 2 to 3 story multi-unit development at 43A Vipond Road & 20 Melia Place, Stanmore Bay.

This geotechnical investigation has been conducted to provide recommendations with respect to slope stability and foundation design for the proposed new units. It aims to determine whether the land on which the new units have been proposed is likely to be subject to erosion, subsidence, or slippage; or whether the proposed development work itself is likely to accelerate, worsen, or result in instability of the land or any other property; and to recommend adequate provisions to protect the land or the proposed development or other property from instability in accordance with the provisions of the Building Act 2004.

2 Site Description

The site location is indicated on Figure 1. Its legal description is Lot 1 & Lot 2, DP 169527.

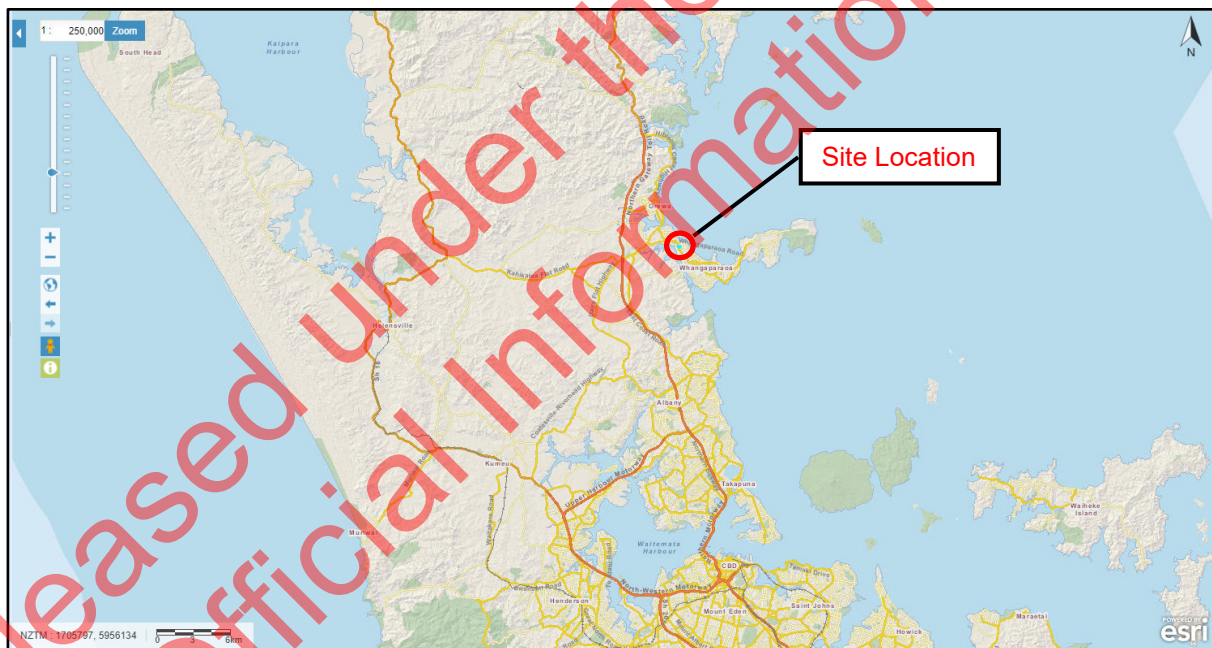


Figure 1: Site Location Plan (Courtesy of Auckland Council Geomaps)

The existing site layout is shown in Figure 2 and Photos 1 to 4.



Figure 2: Site Layout and Investigation Plan (courtesy of Auckland Council Geomaps)





Photo 2: This photo was taken from the north eastern corner of 20 Melia Pl facing south west. It shows the drive way/ parking area at 34A Vipond Rd on the left and the grass area of 20 Melia Pl on the right.



Photo 3: This photo was taken from the south west corner of the exsiting bowling green facing north east. It shows the existing bowling green and existing RSA building in the bacground.



Both properties can be accessed via a long driveway from Vipond Rd and Melia Pl. At the time of our investigation the properties consisted of two existing large buildings and one existing bowling green. The area surrounding the existing buildings and bowling green consisted mostly of grass and small to large trees. The ground in the area of the proposed new units at 20 Melia Pl slopes down towards east between 8-12°. The ground in the area of the proposed new units at 43A Vipond is terraced down towards the south with small slopes of 5-7° in between the terraces. Immediately south of the proposed units at the southern area of the bowling green, the ground becomes very steep with slopes of up to 22° to the south and east.

3 Proposed Development

According to the draft site plans provided by paterson + cullen + archaus, it is proposed that the existing bowling green will be removed. It is then proposed there will be earthwork excavation of up to **2.4m** and fill of up to **3.0m**, from the existing ground level for the construction of the new units and associated civil works.

The draft site plans are attached in Appendix A.

4 Geological Map

According to Geological Map of the Auckland Area, 1:250 000 geological Map 3 by Institute of Geological and Nuclear Sciences, the site is likely to be underlain by East Coast Bays Formation (ECBF) of Waitemata Group. These soils have alternating sandstone and mudstone with variable volcanic content and interbedded volcanoclastic grit beds.

The clay rich and cohesive silt soils may also be prone to shrinking and swelling following changes in natural moisture content.

5 Previous Geotechnical Report

We have also viewed the Geotechnical Completion Report prepared by Soil & Rock Consultants, Rev: 18334, dated 24 October 2018. The specific stability and foundation design recommendations in this report are summarised below.

- Based on the laboratory tests carried out, the soils present are considered to lie in two different site classes Site Class H1 (Highly Reactive) and Site Class H2 (Highly Reactive) in terms of BRANZ Addendum Study Report 102A (based on AS 2870:2011).
- A Dependable Bearing Capacity of 150kPa (Ultimate Bearing Capacity = 300kPa, $\phi_{bc} = 0.5$) is available for shallow foundations embedded 450mm into stiff natural ground or engineered fill or 600mm below final ground surface (whichever gives the deeper embedment).

6 Site investigation

We carried out a site walkover on 18th December 2020. Eight 50mm diameter hand augered exploratory holes were carried out in the area of the proposed fifty-eight new units. The locations of the exploratory holes are shown in Figure 2. In-situ Pilcon Shear Vane tests were carried out in the augered holes generally at 0.5m depth intervals. An additional Scala Penetrometer test was carried out at the base of HA08, and an additional 12.5m deep machine borehole was carried out at the edge of the steep slope along the southern edge of the existing bowling green on 18th December 2020. SPTs were carried out at 1.5m intervals. Hand auger investigation results from the previous geotechnical report are also attached herein and shown in Figure 2.

The graphical hand auger and machine borehole logs are presented in Appendix B.

7 Subsoil Conditions

The findings from our hand auger investigation generally agree with the Geological Map 3 of the Auckland area. During the day of our site investigation, we encountered non engineered fill soils which extended from the existing ground level down to 0.4-0.8m bgl in HA01, HA03, HA04 & HA05, and down to 4.7m bgl in HA08. Underlain these non-engineered soils and from the existing ground level in HA02 & HA06, we encountered original East Coast Bays Formation (ECBF) soils which extended down to our hand auger termination depths of 3.0m bgl in HA01 – HA07 and 5.0m bgl in HA08. An additional Scala Penetrometer test was carried out at the base of HA08 which was terminated at 7.0m bgl. These original ECBF soils encountered during drilling varied from clay to silt. The shear vane tests suggest that these clayey to silty soils are generally stiff to very stiff in consistency. These stiff to very stiff original ECBF soils are considered suitable bearing strata for the proposed excavation and construction. We deem the soils encountered on-site during the day of drilling to be highly expansive (**H1**).

The machine bore hole carried out onsite encountered non-engineered fill soils which extended from the existing ground level down to 7.2m bgl. Underlain these non-engineered fill soils the bore hole encountered original alluvial soils which extended from 7.2m bgl down to 7.7m bgl. Underlain these original alluvial soils the bore hole encountered original ECBF, weathered sand and silt stone which extended from 7.7m bgl down to the bore hole termination depth of 12.5m bgl after encountering three consecutive SPT 50+ for 450mm.

A (0.2-0.3m) deep layer of topsoil and a (0.4-4.7m) deep layer of non-engineered fill was encountered in all hand auger holes during the day of exploratory drilling. This topsoil and non-engineered fill is not considered consistent bearing strata for the proposed development.

8 Groundwater

No ground water table was encountered during the day of exploratory drilling.

9 Ground Stability

On the day of our site visit, no obvious signs of global instability were observed.

We have carried out a slope stability analysis of the site with finished cut and fill levels for the proposed building platform (cross-section A-A' shown on our site plan in Appendix B). The calculated factors of safety for the proposed building platform for normal groundwater, elevated groundwater and seismic conditions are 2.13, 1.57 and 1.52 respectively. They are considered acceptable for the proposed development. The analysis results are presented in Appendix D.

10 Earthworks

The proposed depth of excavation (up to 2.4m) and fill (up to 3.0m) within the boundaries of the subject site is considered reasonable.

The proposed excavations are setback at least 2x excavation depth from any neighbouring structures. We consider that the proposed excavation posts low risk to neighbouring properties, provided that good retaining wall construction practice, i.e. safe temporary batter and good construction time management are implemented.

The proposed fills are mainly in areas underlain by original ECBF soils. These stiff to very stiff, silty to clayey soils are not overly sensitive to settlement. Some fill locations, namely in proximity of the existing 525mm diameter concrete pipe, would likely cover by existing fill. Although the fill encountered was tested to be stiff to very stiff and appeared to have been reasonably compacted a number of years ago. Some minor long term settlement is still expected for the proposed maximum 3m deep additional fill. This settlement is expected to be minor and relatively uniform. It would unlikely adversely affect the performance of driveways and underground services. However, building foundations shall be supported on piles founded below the fill layer as per Section 11.

It is our opinion that provided our recommendations in Section 11 are fully implemented the earthworks would be unlikely to cause any significant ground settlement.

11 Recommendations

Our recommendations are summarised as follows:

11.1 In-ground Palisade Wall

Our sketch (SK-2282-01, shown in Appendix D) shows the location of our recommend in-ground palisade wall along the crest of the existing non-engineered fill slope. This palisade wall shall be designed as follows:

- The palisade wall shall be constructed immediately to the south of the proposed units and extend a minimum of **2.0m** beyond the width of the proposed building footprint. The final location of the palisade wall shall be confirmed by GeoStudio Ltd.
- The wall shall be designed by a chartered structural engineer and shall be a minimum of **0.6m** diameter steel-reinforced concrete piles at a maximum of **3xD** centre spacing. A concrete capping beam shall be included close to ground level.
- Minimum embedment depth = **10.0m** below existing ground level or socketed at least **2.5m** in moderately weathered ECBF rock whichever is greater.
- Lateral soil load depth = **5.0m** for structural design of piles. Lateral soil loads may be calculated using $\gamma=18\text{kN/m}^3$ and $K_0=0.5$ over a width of pile centre spacing.

11.2 Earthworks

- Detailed earthwork and retaining wall plans are not yet available at the time of preparing this report. Geostudio Ltd should be asked to review detailed earthwork and retaining wall plans to confirm our recommendations herein.
- Based on our subsoil investigation, the upper **1.5m to 2.3m** depths soils were mainly moist. These moist soils would likely be able to be reused directly as engineered fill materials. Underlying these moist soils are wet to saturated soils which are unlikely to be suitable for direct use as engineered fill.
- For the benefits of utilising in situ cohesive soils for fill materials, we recommend that any major earth works are carried out during the summer months from November to March. This would reduce the risk of additional stabilisation of the fill material being required. However, the condition of the excavated material shall be inspected by a geotechnical engineer for its suitability as engineered fill. Additional air drying and/or lime stabilisation may still be required.
- We recommend that all cut slopes into natural ground and fill slopes using engineered fill should not be steeper than 2.5H:1V (21.8°). All retaining walls shall be designed by a chartered professional engineer.
- All topsoils and unsuitable soils shall be removed and benched into the slope prior to filling. The cleared ground shall be inspected by a chartered geotechnical engineer.
- Due to the complexity and volume of proposed earthwork, a Geotechnical Completion Report is recommended to ensure all earthworks are appropriately supervised during construction and are certified by a chartered professional geotechnical engineer.
- Any **cohesive fill** shall be constructed using a suitable sized pad-foot roller in layers no more than **200mm** thick. We provide the following specifications for cohesive fill compaction.
 - Undrained shear strength minimum single value = **110kPa**.
 - Undrained shear strength average value = **140kPa**.
 - Maximum single value air void = **12%**.
 - Average value air void < **10%**.

- All engineered fill shall be inspected by a geotechnical engineer every **0.5m** layer, or otherwise instructed by the engineer on site.
- Granular fill (hardfill) may be used for earthworks outside summer months. It shall be constructed using suitably sized vibrating drum roller or vibrating plat compactor in suitable thicknesses depending on compactor size. We provide the following specifications for granular fill compaction:
 - Granular fill shall be certified aggregates (GAP40 or GAP65). Other aggregate type i.e. crushed concrete/Soft Pit Run (SPR) may be accepted for fill in non-critical areas subject to specific engineer approval.
 - Clegg Hammer Impact Value (CIV) **>25**.
 - Achieve **95%** maximum dry density and **±3%** of optimum moisture content.
 - All engineered fill shall be inspected by a geotechnical engineer every **0.5m** layer, or otherwise instructed by the engineer on site.

11.3 Foundations

11.3.1 Suspended floors on Pile Foundations

Due to the presence of non-engineered fill encountered, all load bearing footings foundations as shown as shaded areas in **SK-2282-01** shall be supported on piled foundations extending below the existing non-engineered fill as follows.

- Geotechnical ultimate bearing capacity (unfactored) = **720kPa** for piles through fill with a minimum of **0.6m** into stiff original soils. The expected pile depths range from **1.2m** to **8.0m** below the existing ground level.
- Geotechnical ultimate bearing capacity (unfactored) = **4000kPa** for piles through fill with a minimum of **0.6m** into moderately weathered ECBF rock (SPT=50+). The expected pile depths range from **8.0m** to **11.0m** below the existing ground level.
- Geotechnical ultimate skin friction (unfactored) of **30kPa**, the friction contribution of the upper **0.6m** below ground level, or non-engineered fill depth should be ignored, whichever is greater.
- Due to risk of long term settlement of the underlying non-engineered fill, concrete slabs or subfloor shall be designed as **fully suspended**, i.e. not relying on ground for support.
- Based on the investigation results and our experience with similar sites, the site may be classified as subsoil **Class C** – shallow soil in accordance with ASNZS1170.5.

11.3.2 Concrete Slab Foundations

For areas outside the shaded areas in **SK-2282-01**, shallow foundations are considered suitable. We provide the following foundation design recommendations:

- The natural soils, i.e. excluding topsoil and any non-engineered fill, are generally considered suitable for NZS3604:2011 type foundations, except that due to the potential for shrinkage and swelling, the subsoil lies outside of the definition of 'good ground' as defined by NZS3604:2011. The perimeter footings shall have a minimum embedment depth of **750mm** below the cleared

ground level, or alternatively the foundations shall be specifically designed by a chartered professional engineer for expansive soil **Class H1** in terms of AS2780:2011.

- For design of shallow foundations on competent natural ground, a geotechnical ultimate bearing capacity (unfactored) of **300kPa** may be assumed.
- Based on the investigation results and our experience with similar sites, the site may be classified as subsoil **Class C** – shallow soil in accordance with ASNZS1170.5.
- All unsuitable soils (topsoil, non-engineered fill, soft original soil) shall be removed and replaced with compacted engineered fill (GAP40) under all concrete slabs. The expected undercut depth is approximately **0.1m to 0.5m** below the existing ground level.
- For the use of other pile foundations (if required), our recommendations are as follows;
 - Minimum pile depth = **0.6m** into stiff original soils.
 - Geotechnical ultimate bearing capacity (unfactored) = **300kPa or 540kPa for piles >2.5m deep**
 - Geotechnical ultimate skin friction (unfactored) of **30kPa**. The friction contribution of the upper **0.6m** below ground level.

11.4 Retaining Walls

- All new retaining walls shall be specifically designed by a chartered professional engineer for the actual retaining height and any surcharge (i.e. additional load from slopes, upper retaining walls or site boundary).
- Furthermore, we recommend that any vertical cut faces which exceed heights of **1.5m** should be benched at **1V:1H** or to be supported by additional in-ground piles prior to bulk excavation. Along with good practice during construction (covering cut faces, constructing of retaining walls within seven days of excavation), this method is to further ensure safety during works close to, or below cut faces.
- We provide the following general soil parameters for retaining wall design:

○ Bulk unit weight for retaining material	18.0kN/m ³
○ Soil internal friction (ϕ')	30°
○ Undrained shear strength (s_u)	50kPa
○ Geotechnical ultimate bearing capacity (unfactored)	300kPa
- All retaining wall shall be constructed with appropriate subsoil drainage system discharged as per council's requirements.
- It is the builder's/owner's responsibility to ensure that temporary stability of any soil cut face is maintained during construction of retaining walls. Where possible, all retaining walls should be constructed in dry weather conditions over a short period of time (typically within 10 days) to reduce the risk of temporary instability.

11.5 Construction Inspections

- A pre-construction meeting is highly recommended prior to commence of earthworks.
- All soil subgrade (after stripping of topsoil) shall be inspected by a chartered professional engineer (geotechnical) prior to commencement of engineered filling (if required).
- The compaction of all engineered fill shall be inspected by a chartered professional engineer (geotechnical).
- All foundation pile drill holes (if applicable) shall be inspected by a chartered professional engineer (geotechnical) prior to installation of timber poles and pouring of concrete.

12 Limitations

This report is the property of our client and GeoStudio Ltd.

Our professional services are performed using a degree of care and skill normally exercised, under similar circumstances, by reputable consultants practicing in this field at this time. No other warranty, expressed or implied, is made as to the professional advice presented in this report, in regard to its accuracy or completeness.

The recommendations and opinions contained in this report are based on our visual reconnaissance of the site, information from geological maps and field investigation(s) at discrete locations. Inferences are made about the nature and continuity of ground conditions away from the investigation(s) which cannot be guaranteed. The descriptions detailed on the exploratory hole logs are based on the field descriptions of the soils encountered at the time of investigation(s).

This report has been prepared for the particular project described to us and no responsibility is accepted for the use of any part of this report in any other context or for any other purposes. Except as required by law, no third party (excluding the local authority) may use or rely upon this report unless authorised by GeoStudio Ltd in writing. To the extent permitted by law, GeoStudio Ltd expressly disclaims and excludes liability for any loss, damage, cost or expense suffered by any third party relating to or resulting from the use of, or reliance upon any information contained in this report. It is the responsibility of third parties to independently make enquiries or seek advice in relation to their particular requirements.

All appendices should be read in conjunction with the main body of the report and this report should not be considered complete without them.

REPORT PREPARED BY:



Joel Scheepens
BSc (Geology)



Engineering Geologist

REPORT REVIEWED BY:



Geoffrey Kang
BE(civil), ME(civil), CPEng, MIPENZ



Director / Geotechnical Engineer

Appendix A

Draft Site Plans and Sections

Released under the provision of
the Official Information Act 1982

SITE CALCULATIONS - 20 MELIA PLACE
LOT 2, DP 169527

SITE AREA:	18250.6m²
BUILDING COVERAGE:	3120.7m² (17.1%)
IMPERVIOUS AREA:	4640.9m² (25.4 %)
LANDSCAPED AREA:	10489m² (57.5%)

DRAFT

REV	DATE	INITIAL	AMENDMENT
REVISIONS			

PROJECT STATUS

CLIENT
MELIA DEVELOPMENT LIMITED
PROJECT
20 MELIA PLACE
20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE

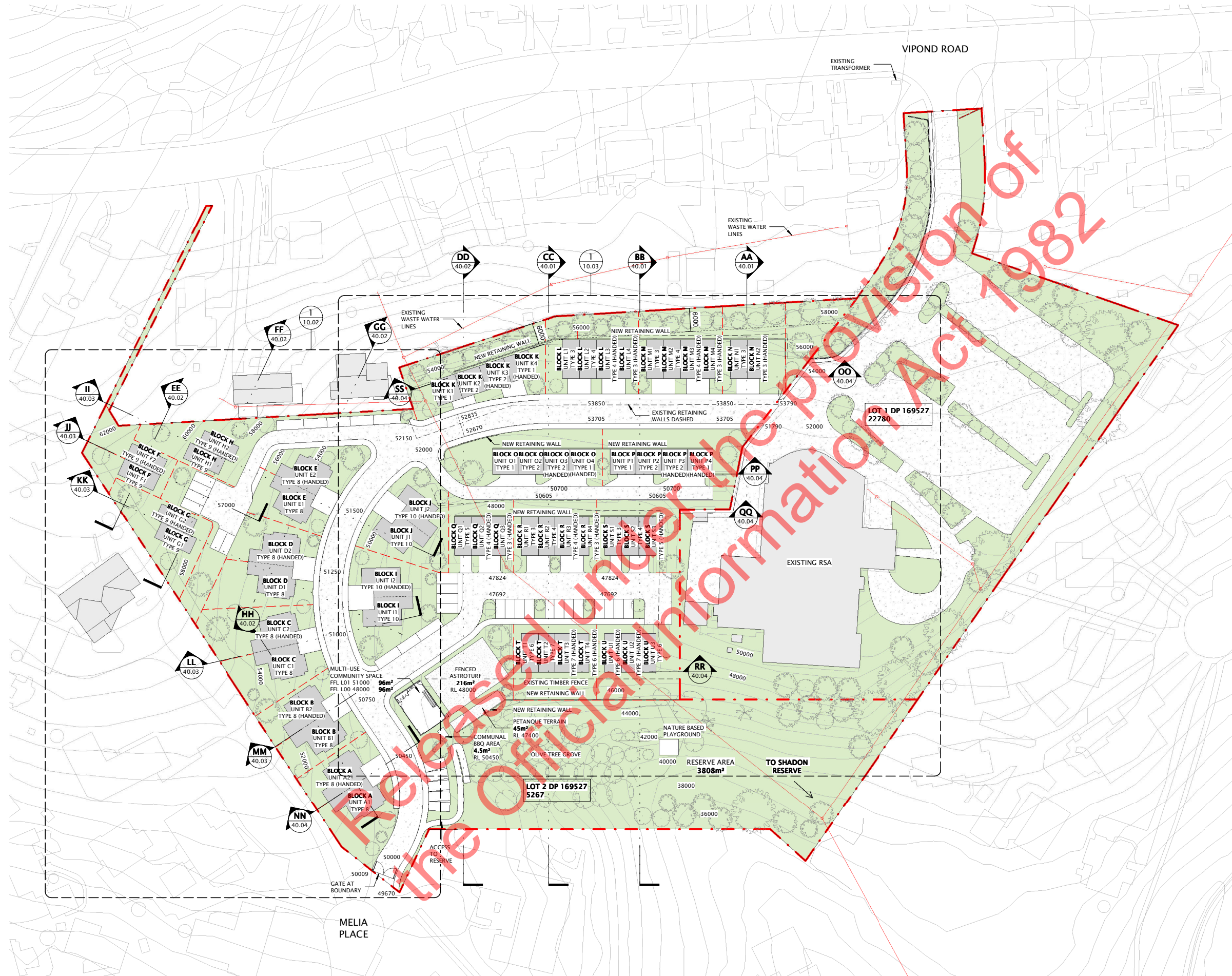
**PROPOSED OVERALL
DEVELOPMENT**

TRUE NORTH	PROJECT NORTH	DESIGN	DRAWN
N	N	CM	LM
		SCALE @ A1 (HALF SCALE IF PRINTED @ A3)	1 : 500

FIRST ISSUE DATE	PROJECT No.	20053
03/25/21		REVISION
SHEET No.		10.01



paterson +
cullen + archaus



Released under the provision of
the Official Information Act 1982

DRAFT

REV	DATE	INITIAL	AMENDMENT
REVISIONS			

PROJECT STATUS

CLIENT
MELIA DEVELOPMENT LIMITED

PROJECT
20 MELIA PLACE
20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE
PROPOSED DEVELOPMENT
ZONE 1

TRUE NORTH	DESIGN	DRAWN
N	CM	LM
	SCALE @ A1 (HALF SCALE IF PRINTED @ A3) 1 : 250	

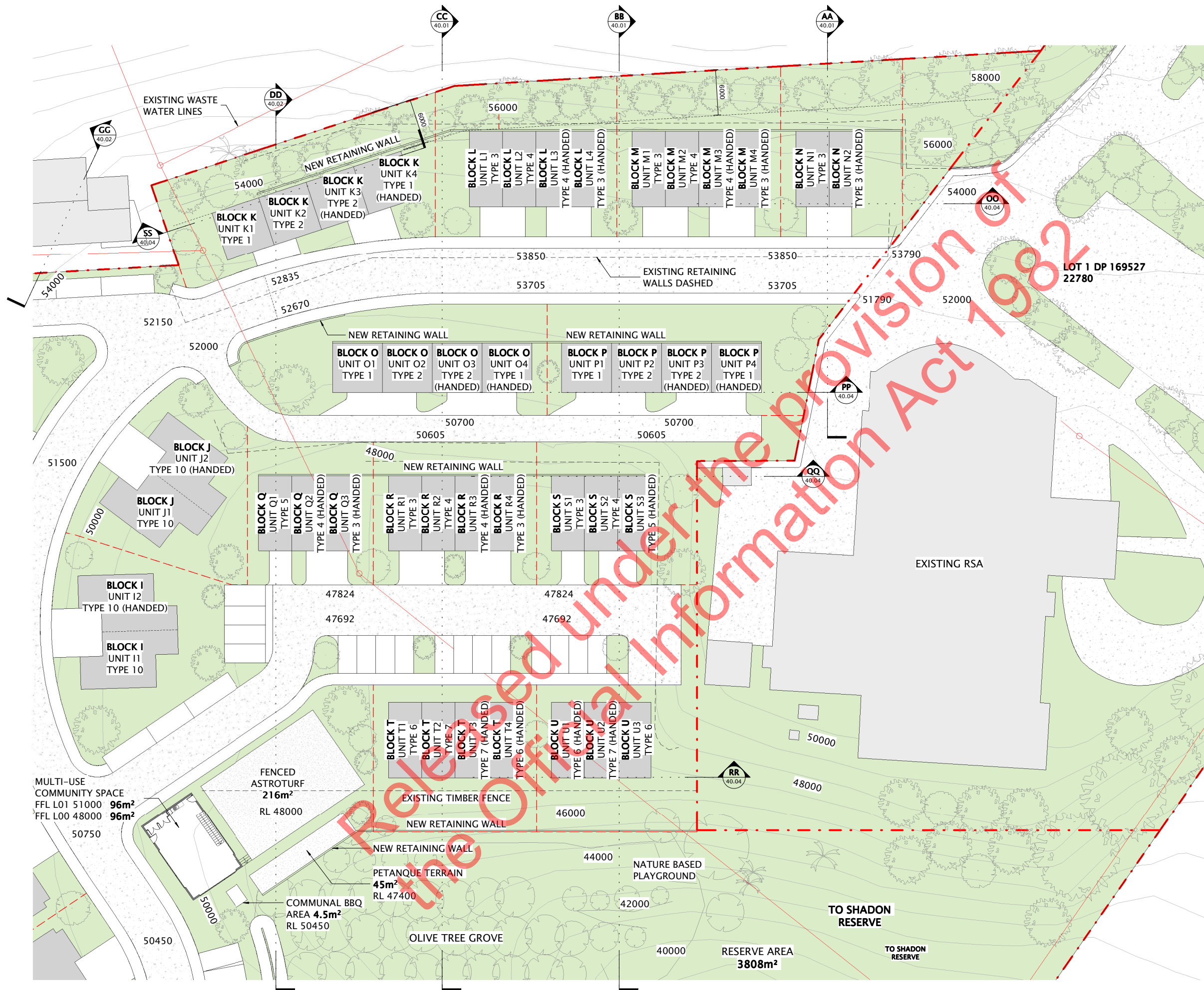
FIRST ISSUE DATE	PROJECT No.	20053
01/27/21		
SHEET No.	REVISION	

10.02



paterson +
cullen + archaus

(09) 309 8931
www.patersoncullenarchaus.co.nz



DRAFT

REV	DATE	INITIAL	AMENDMENT

PROJECT STATUS

CLIENT
MELIA DEVELOPMENT LIMITED
PROJECT
20 MELIA PLACE
20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE

PROPOSED DEVELOPMENT ZONE 2

TRUE NORTH	DESIGN	DRAWN
N	CM	LM
	SCALE @ A1	
	(HALF SCALE IF PRINTED @ A3)	
	1 : 250	

FIRST ISSUE DATE	PROJECT No.	20053
01/27/21		REVISION
SHEET No.		

10.03



paterson +
cullen + archaus

SITE PLAN LEGEND

- 2 LEVEL UNITS
- 3 LEVELS UNITS
- COMMUNITY FACILITIES

2 LEVEL UNITS	27 (45.8%)
3 LEVELS UNITS	32 (54.2%)
TOTAL NO. OF UNITS	59

DRAFT

REV	DATE	INITIAL	AMENDMENT
REVISIONS			

PROJECT STATUS

CLIENT
MELIA DEVELOPMENT LIMITED
PROJECT
20 MELIA PLACE
20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE

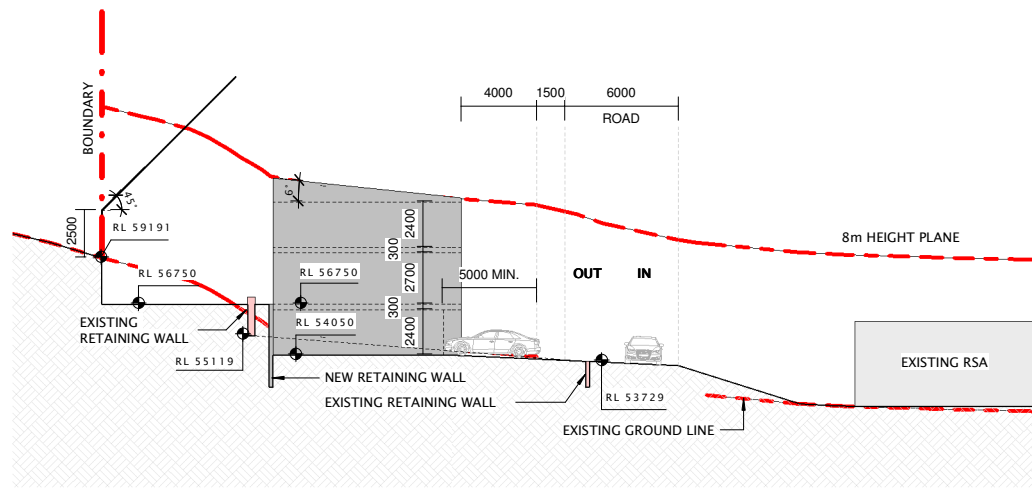
PROPOSED OVERALL DEVELOPMENT - TYPOLOGIES

TRUE NORTH	PROJECT NORTH	DESIGN	DRAWN
N	N	CM	LM
		SCALE @ A1 (HALF SCALE IF PRINTED @ A3)	1 : 500

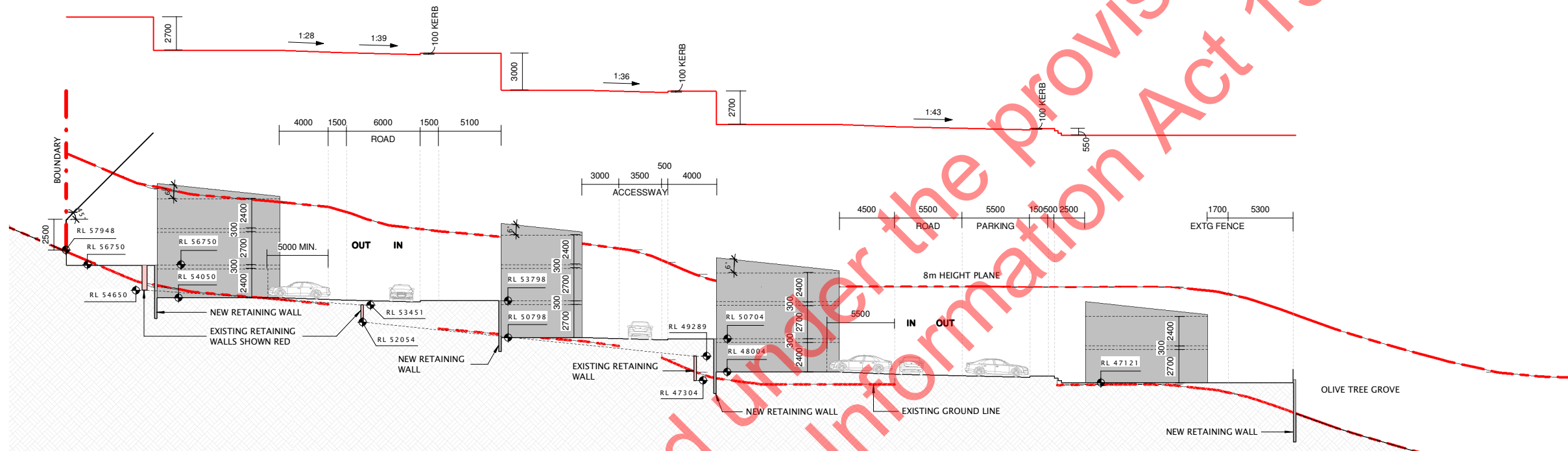
FIRST ISSUE DATE	PROJECT No.	20053
03/25/21		REVISION
SHEET No.		10.04



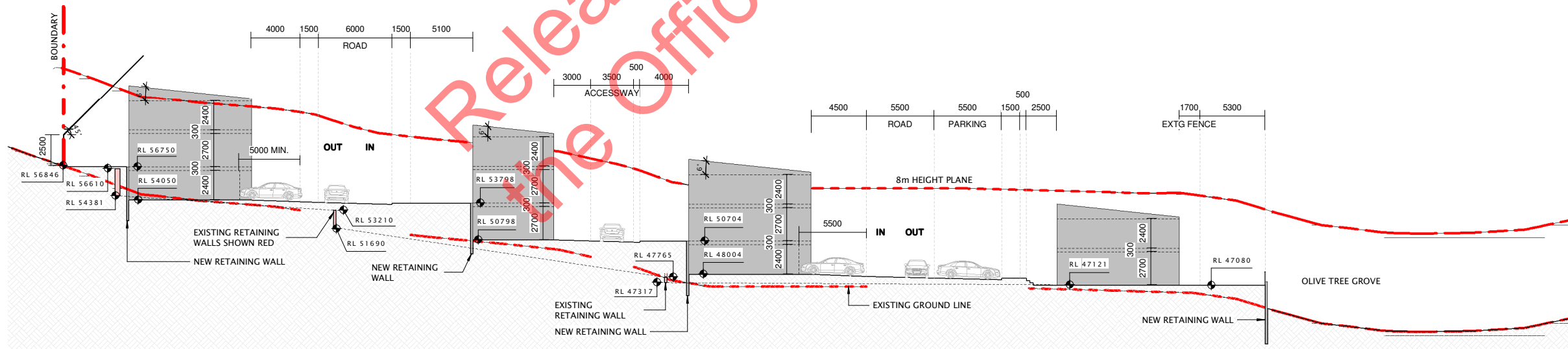
paterson +
cullen + archaus



AA SITE SECTION A-A
SCALE: 1 : 200



BB SITE SECTION B-B
SCALE: 1 : 200



CC SITE SECTION C-C
SCALE: 1 : 200

DRAFT

REV	DATE	INITIAL	AMENDMENT
REVISIONS			

PROJECT STATUS

CLIENT
MELIA DEVELOPMENT LIMITED
PROJECT
20 MELIA PLACE
20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE
SITE SECTIONS

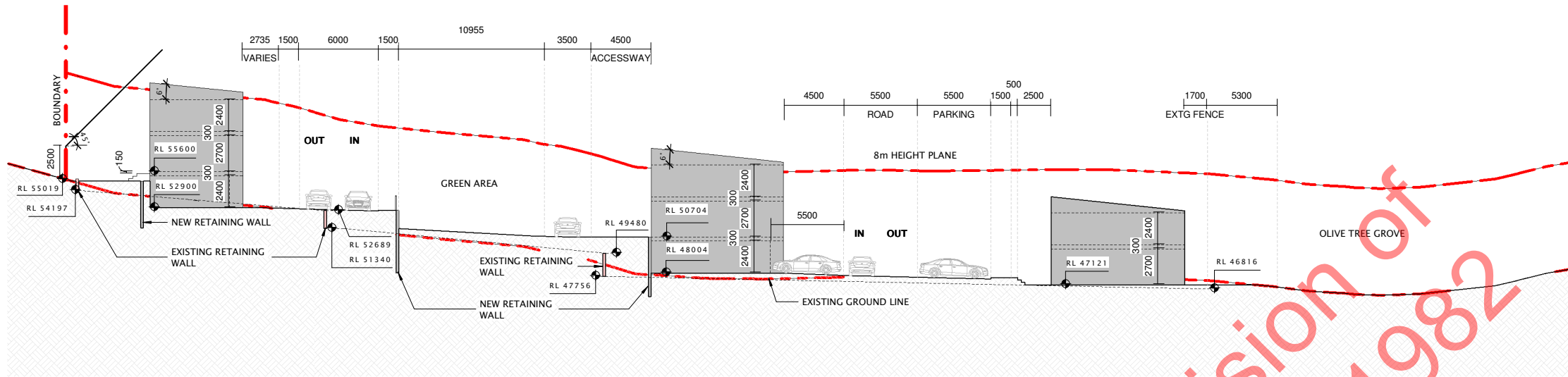
DESIGN	DRAWN
CM	LM
SCALE @ A1 (HALF SCALE IF PRINTED @ A3)	
1 : 200	

FIRST ISSUE DATE	PROJECT No.
01/25/21	20053
SHEET No.	REVISION

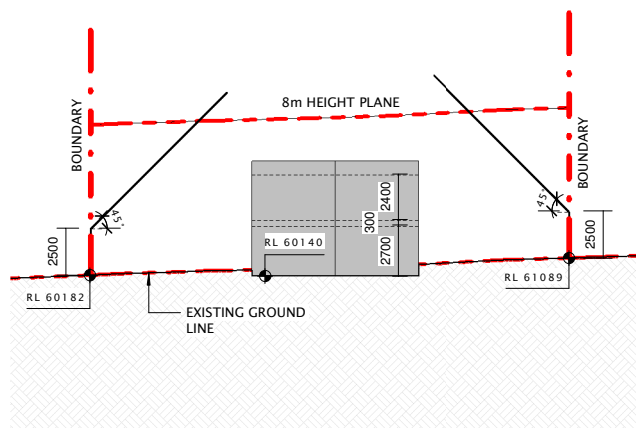
40.01



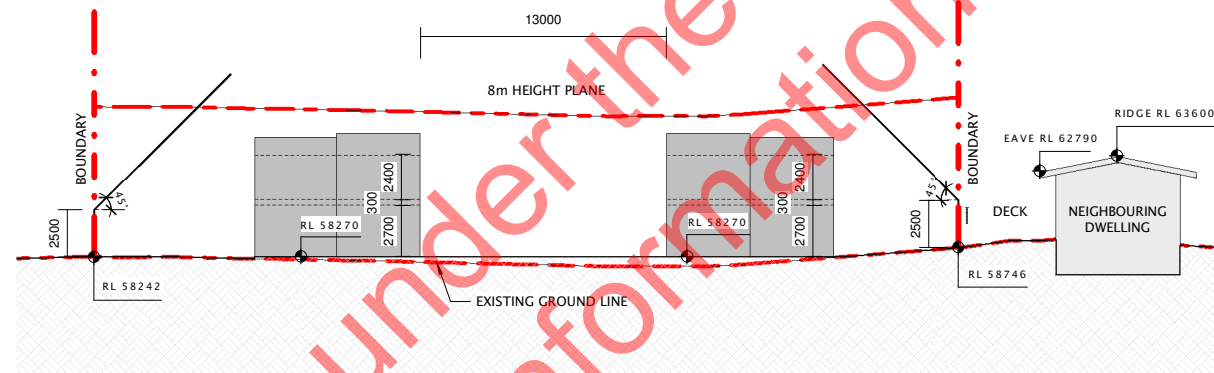
paterson +
cullen + archaus



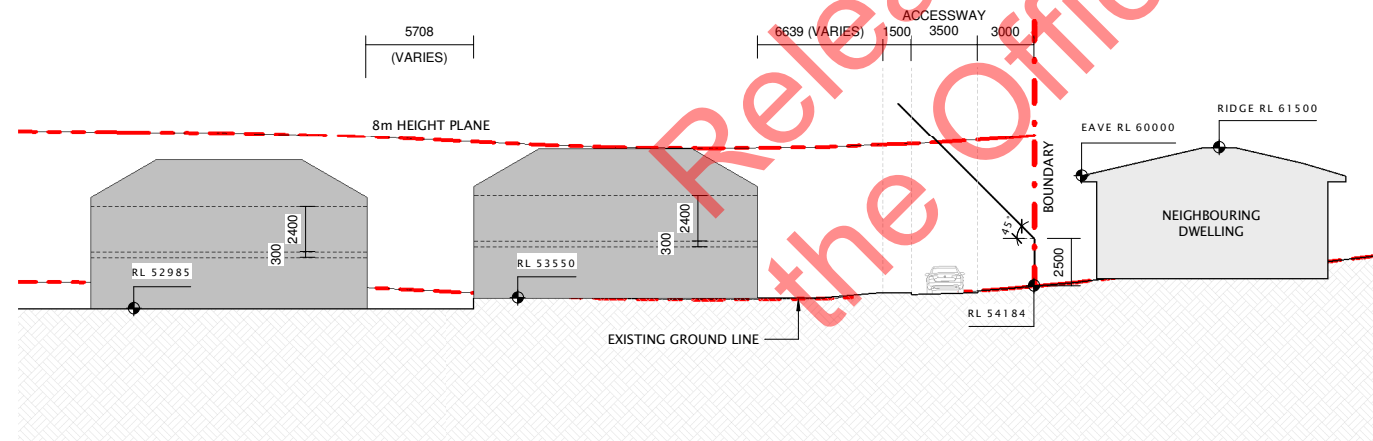
DD SITE SECTION D-D
 10.01 SCALE: 1 : 200



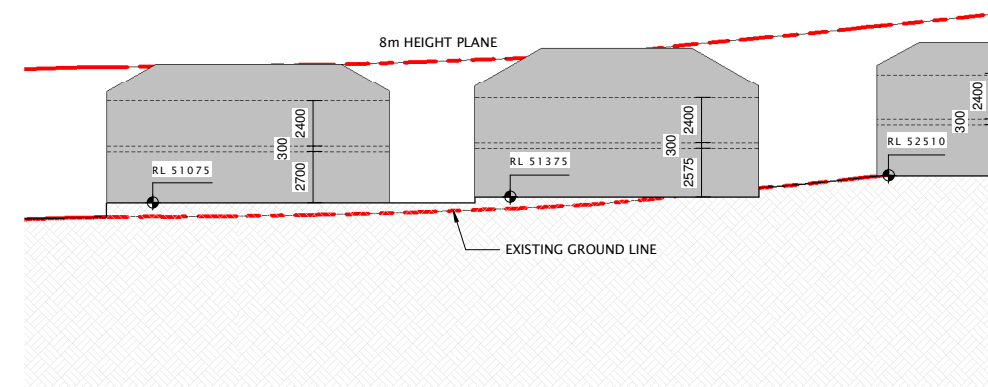
EE SITE SECTION E-E
 10.01 SCALE: 1 : 200



FF SITE SECTION F-F
 10.01 SCALE: 1 : 200



GG SITE SECTION G-G
 10.01 SCALE: 1 : 200



HH SITE SECTION H-H
 10.01 SCALE: 1 : 200

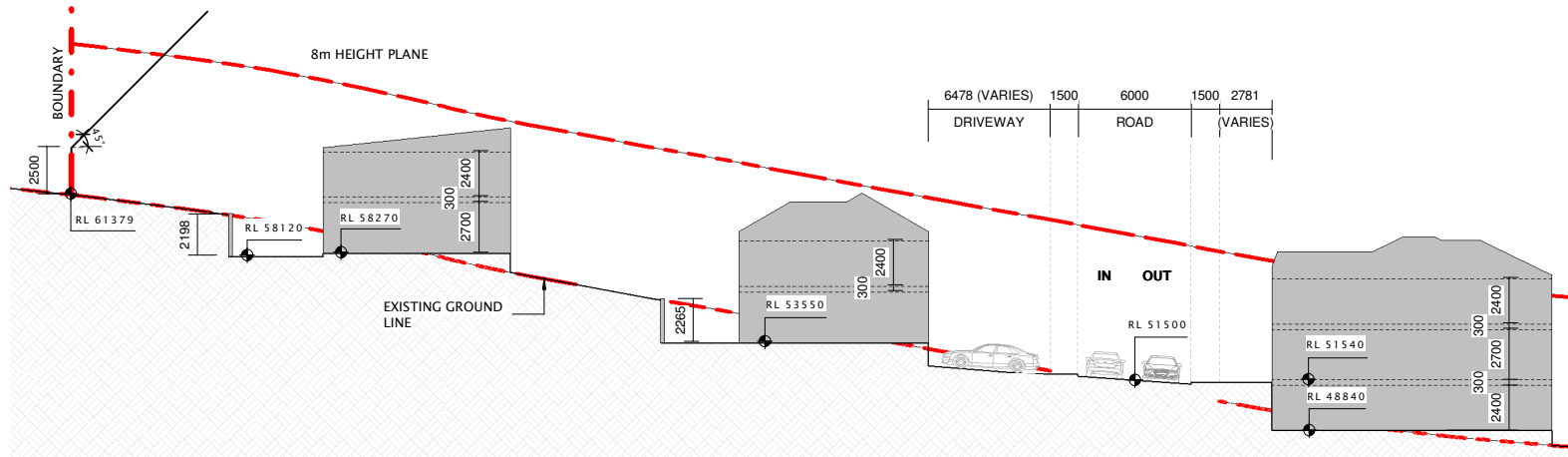
DRAFT

REV	DATE	INITIAL	AMENDMENT
REVISIONS			

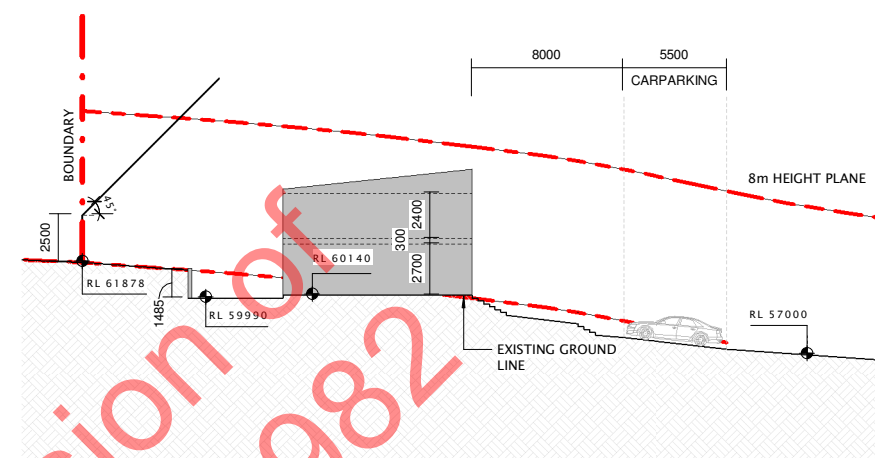
PROJECT STATUS
CLIENT MELIA DEVELOPMENT LIMITED
PROJECT 20 MELIA PLACE 20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE
SITE SECTIONS

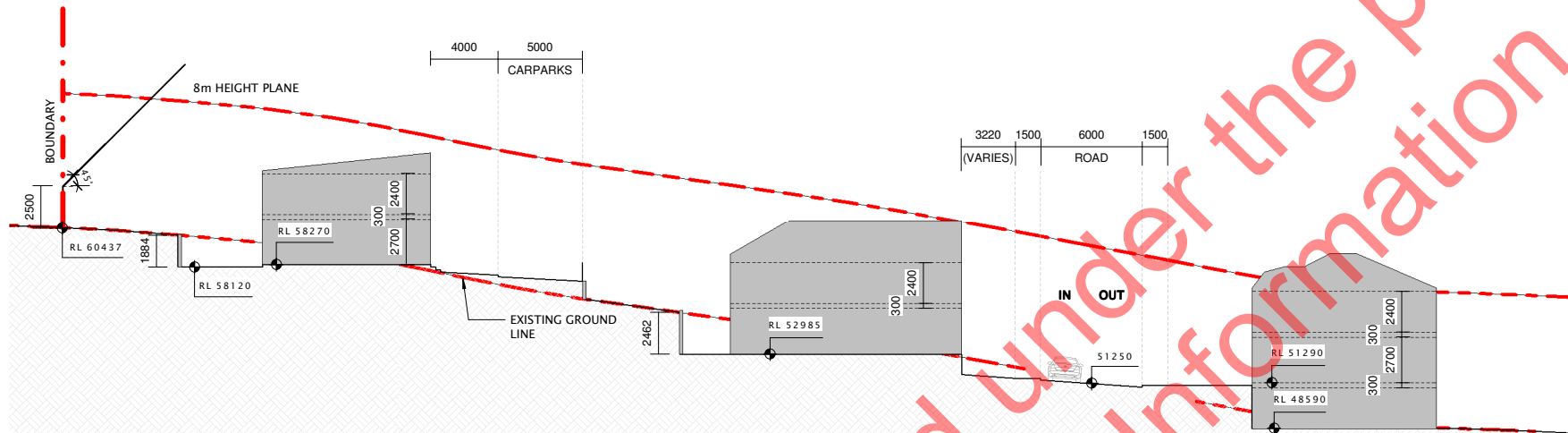
DESIGN	DRAWN
CM	LM
SCALE @ A1 (HALF SCALE IF PRINTED @ A3)	1 : 200
FIRST ISSUE DATE 01/25/21	PROJECT No. 20053
SHEET No. 40.02	REVISION



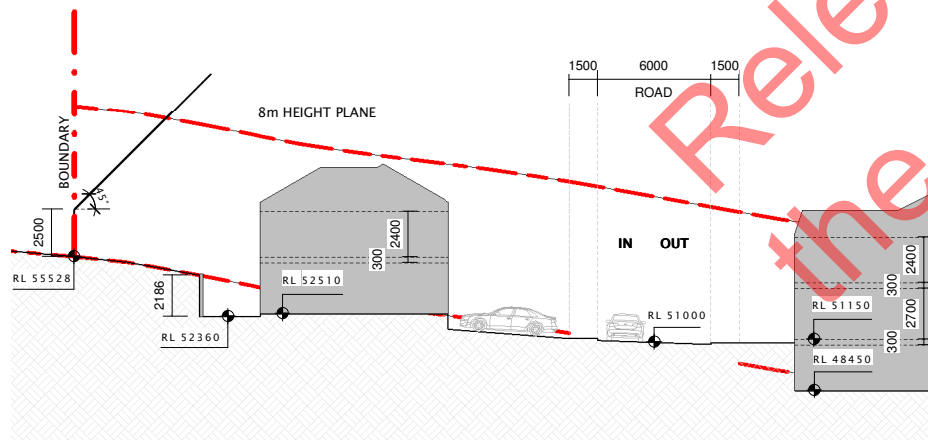
II SITE SECTION I-I
10.01 SCALE: 1 : 200



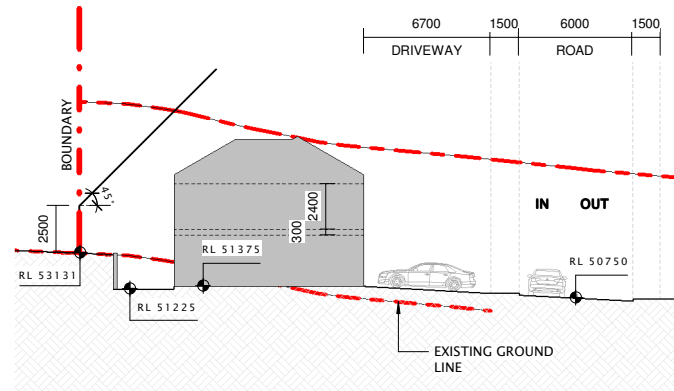
J SITE SECTION J-J
10.01 SCALE: 1 : 200



KK SITE SECTION K-K
10.01 SCALE: 1 : 200



LL SITE SECTION L-L
10.01 SCALE: 1 : 200



MM SITE SECTION M-M
10.01 SCALE: 1 : 200

DRAFT

REV	DATE	INITIAL	AMENDMENT
REVISIONS			

PROJECT STATUS

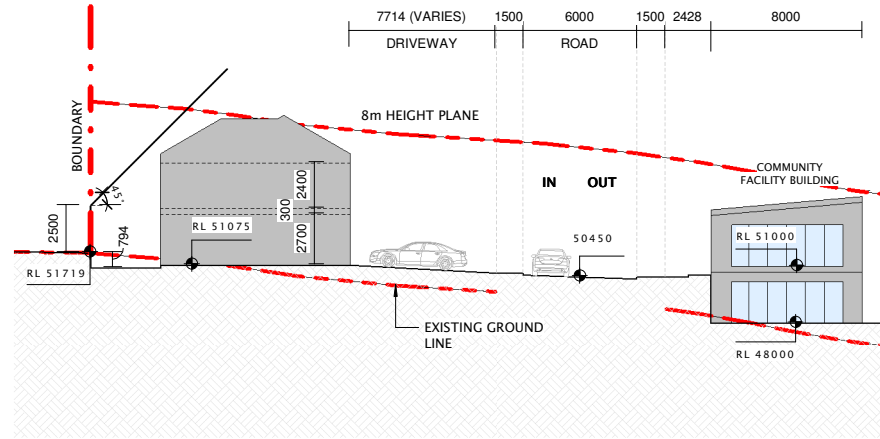
CLIENT
MELIA DEVELOPMENT LIMITED
PROJECT
20 MELIA PLACE
20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE
SITE SECTIONS

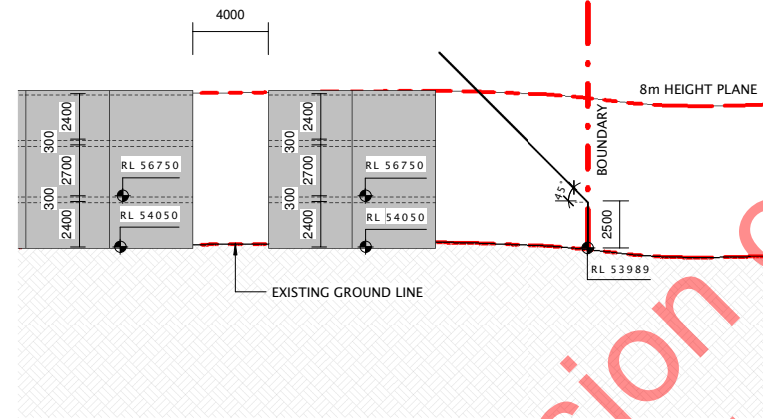
DESIGN	DRAWN
CM	LM
SCALE @ A1 (HALF SCALE IF PRINTED @ A3)	
1 : 200	
FIRST ISSUE DATE	PROJECT No.
01/25/21	20053
SHEET No.	REVISION
40.03	



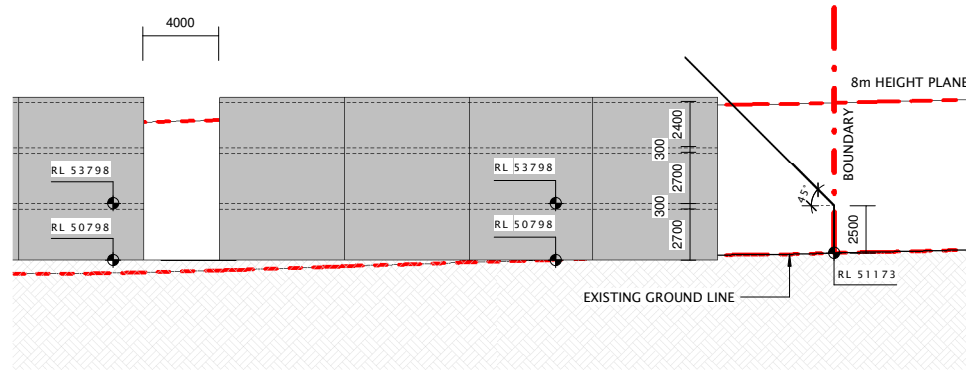
paterson +
cullen + archaus



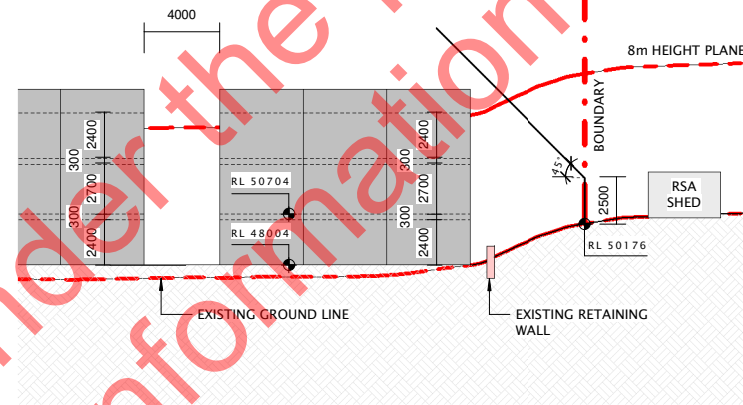
NN SITE SECTION N-N
10.01 SCALE: 1 : 200



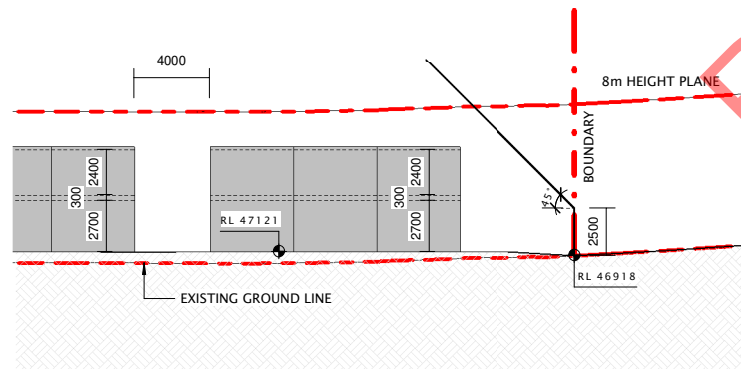
OO SITE SECTION O-O
10.01 SCALE: 1 : 200



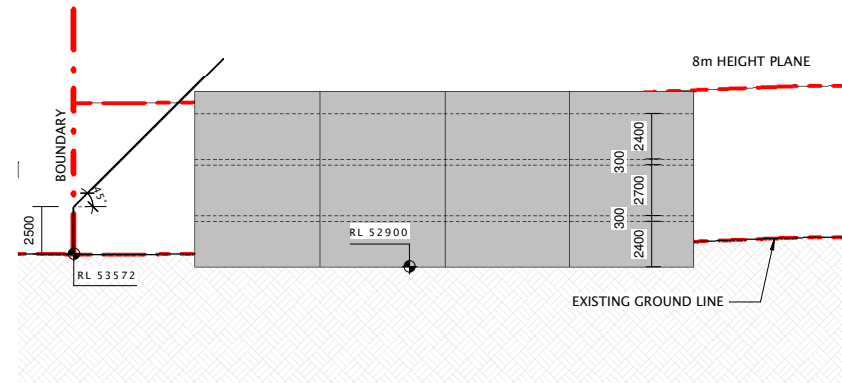
PP SITE SECTION P-P
10.01 SCALE: 1 : 200



QQ SITE SECTION Q-Q
10.01 SCALE: 1 : 200



RR SITE SECTION R-R
10.01 SCALE: 1 : 200



SS SITE SECTION S-S
10.01 SCALE: 1 : 200

DRAFT

REV	DATE	INITIAL	AMENDMENT
REVISIONS			

PROJECT STATUS

CLIENT
MELIA DEVELOPMENT LIMITED
PROJECT
20 MELIA PLACE
20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE
SITE SECTIONS

DESIGN	DRAWN
CM	LM
SCALE @ A1 (HALF SCALE IF PRINTED @ A3)	1 : 200
FIRST ISSUE DATE	PROJECT No.
02/18/21	20053
SHEET No.	REVISION
40.04	



paterson +
cullen + archaus

Appendix B

Investigation Logs

Released under the provision of
the Official Information Act 1982



JOB NUMBER: 2282
ADDRESS: 43A Vipond Road & 20 Melia Place,
Stanmore Bay
WEATHER: Fine
PROJECT: Multiple Units
CLIENT: KIPG

HAND AUGER NO.
HA01
SHEET 1 OF 1
LOGGED: TL
CHECKED: JS
DRILL DATE: 18/12/2020

DEPTH (m)	SOIL DESCRIPTION	LEGEND	GROUND WATER	UNDRAINED SHEAR STRENGTH (kPa)	SCALA (BLOWS PER 100mm)
				0 50 100 150	0 5 10 15
	Topsoil				
	[NON ENGINEERED FILL] silty CLAY, grey mottled orangey brown, dark grey and brown, very stiff, moist to dry				
	[ECBF] silty CLAY, orangey brown mottled grey, very stiff, moist				
1					
	@1.2m becomes grey mottled orangey brown, stiff				
	@1.6m becomes moist to wet				
2					
	@2.6m becomes grey mottled orangey brown and light brownish grey				
3					
	EOB @3.0m, Target depth reached				
4					
5					

NOTES: - No groundwater table was encountered.

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

09- 476 1417
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

021-134 3823



JOB NUMBER: 2282
ADDRESS: 43A Vipond Road & 20 Melia Place,
Stanmore Bay
WEATHER: Fine
PROJECT: Multiple Units
CLIENT: KIPG

HAND AUGER NO.
HA02
SHEET 1 OF 1
LOGGED: TL
CHECKED: JS
DRILL DATE: 18/12/2020

DEPTH (m)	SOIL DESCRIPTION	LEGEND	GROUND WATER	UNDRAINED SHEAR STRENGTH (kPa)	SCALA (BLOWS PER 100mm)
				0 50 100 150	0 5 10 15
	Topsoil				
	[ECBF] silty CLAY, grey mottled orangey brown, very stiff, moist				
1	@1.1m becomes moist to wet				
	@1.4m becomes silty CLAY minor basalt grit, orangey brown mottled grey and dark orangey brown, stiff				
	@1.6m becomes silty CLAY				
2	@2.1m becomes grey mottled orangey brown, wet				
	@2.3m becomes grey mottled orangey brown and light brownish grey, very stiff				
	@2.7m becomes clayey SILT, bluish grey				
3	EOB @3.0m, Target depth reached				
4					
5					

NOTES: - No groundwater table was encountered.

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

09- 476 1417
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

021-134 3823



JOB NUMBER: 2282
ADDRESS: 43A Vipond Road & 20 Melia Place,
Stanmore Bay
WEATHER: Fine
PROJECT: Multiple Units
CLIENT: KIPG

HAND AUGER NO.

HA03

SHEET 1 OF 1

LOGGED: TL

CHECKED: JS

DRILL DATE: 18/12/2020

DEPTH (m)	SOIL DESCRIPTION	LEGEND	GROUND WATER	UNDRAINED SHEAR STRENGTH (kPa)	SCALA (BLOWS PER 100mm)
				0 50 100 150	0 5 10 15
	Topsoil				
	[NON ENGINEERED FILL] silty CLAY minor gravel, orangey brown mottled grey, very stiff, moist				
	[ECBF] clayey SILT, grey mottled orangey brown, stiff, moist to wet				
	@0.6m becomes whitish grey, wet				
1	@0.8m becomes grey mottled orangey brown, very wet				
	@1.3m becomes very stiff				
	@1.6m becomes bluish grey, moist to wet				
2	@1.8m becomes bluish grey mottled dark bluish grey				
3					
	EOB @3.0m, Target depth reached				
4					
5					

NOTES: - No groundwater table was encountered.

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

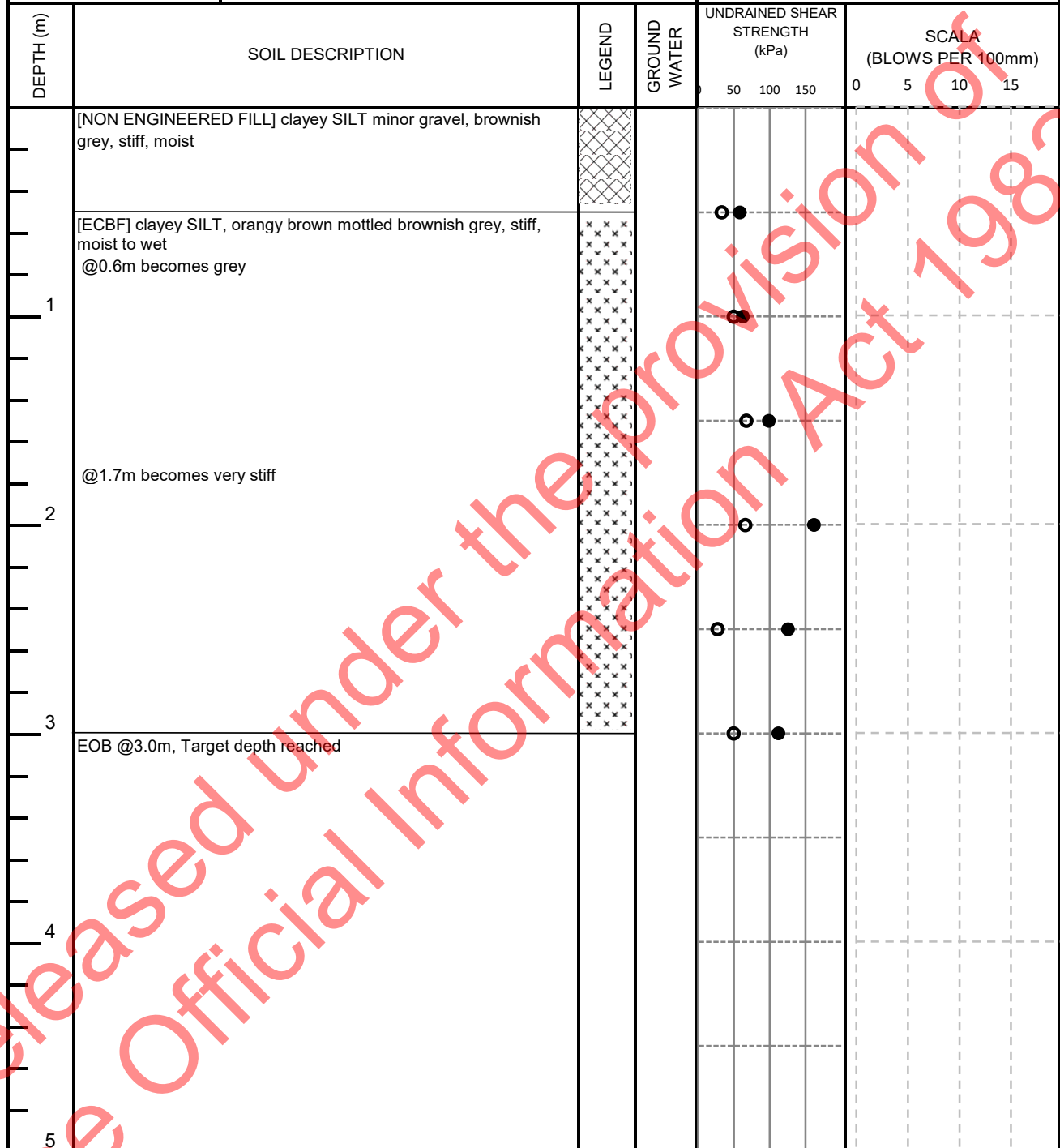
09- 476 1417
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

021-134 3823



JOB NUMBER: 2282
ADDRESS: 43A Vipond Road & 20 Melia Place,
Stanmore Bay
WEATHER: Fine
PROJECT: Multiple Units
CLIENT: KIPG

HAND AUGER NO.
HA04
SHEET 1 OF 1
LOGGED: DBT
CHECKED: JS
DRILL DATE: 18/12/2020



GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

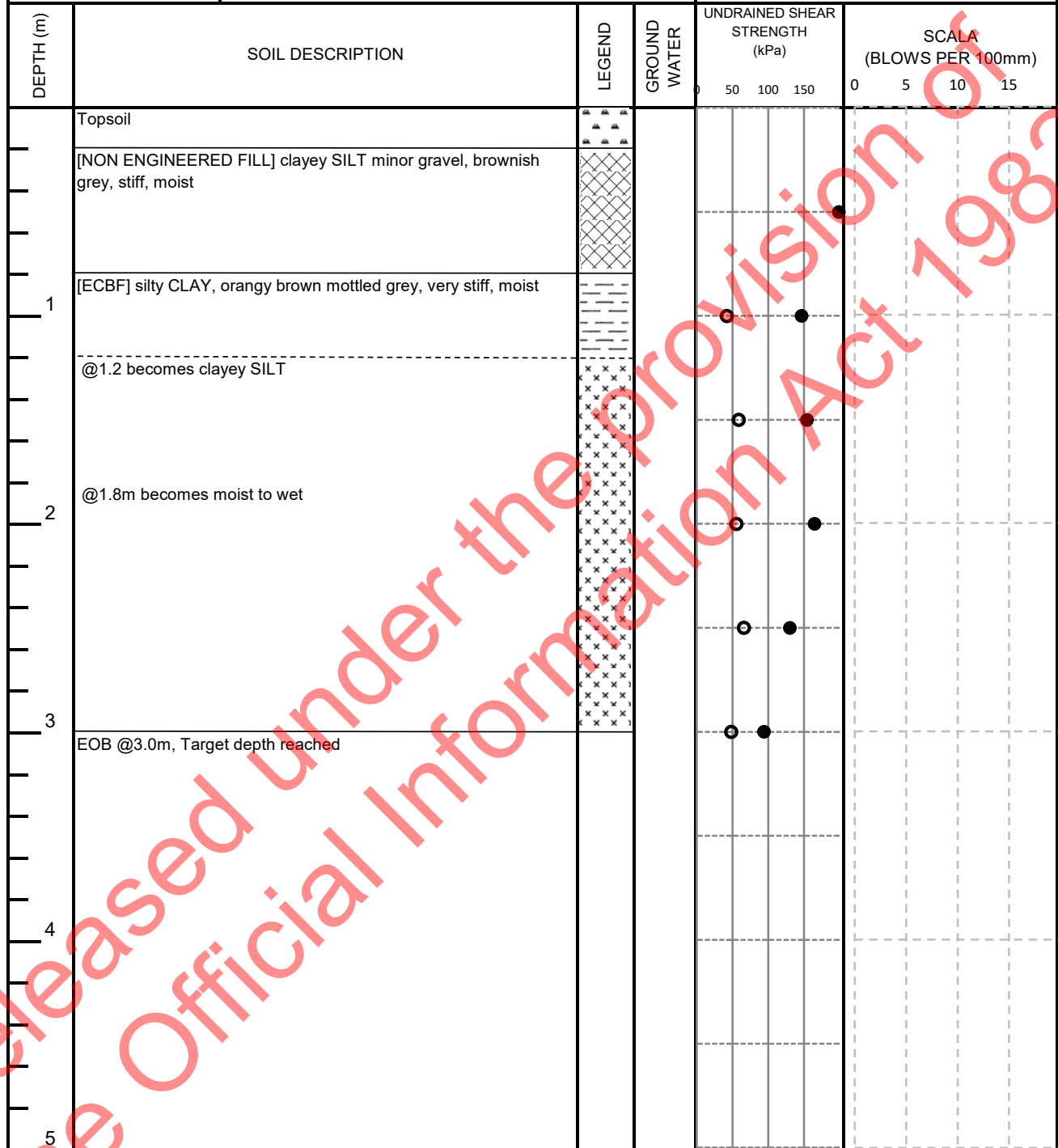
09- 476 1417
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

021-134 3823



JOB NUMBER: 2282
ADDRESS: 43A Vipond Road & 20 Melia Place,
Stanmore Bay
WEATHER: Fine
PROJECT: Multiple Units
CLIENT: KIPG

HAND AUGER NO.
HA05
SHEET 1 OF 1
LOGGED: DBT
CHECKED: JS
DRILL DATE: 18/12/2020



NOTES: - No groundwater table was encountered.

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

09- 476 1417
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

021-134 3823



JOB NUMBER: 2282
ADDRESS: 43A Vipond Road & 20 Melia Place,
Stanmore Bay
WEATHER: Fine
PROJECT: Multiple Units
CLIENT: KIPG

HAND AUGER NO.

HA06

SHEET 1 OF 1

LOGGED: DBT

CHECKED: JS

DRILL DATE: 18/12/2020

DEPTH (m)	SOIL DESCRIPTION	LEGEND	GROUND WATER	UNDRAINED SHEAR STRENGTH (kPa)	SCALA (BLOWS PER 100mm)
				0 50 100 150	0 5 10 15
	Topsoil				
	[ECBF] silty CLAY, orangy brown mottled grey, very stiff, moist				
1	@0.8m becomes grey mottled orangy brown				
2	@2.0m becomes moist to wet				
3	EOB @3.0m, Target depth reached				
4					
5					

NOTES: - No groundwater table was encountered.

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

09- 476 1417
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

021-134 3823



JOB NUMBER: 2282
ADDRESS: 43A Vipond Road & 20 Melia Place,
Stanmore Bay
WEATHER: Fine
PROJECT: Multiple Units
CLIENT: KIPG

HAND AUGER NO.
HA07
SHEET 1 OF 1
LOGGED: HQ
CHECKED: JS
DRILL DATE: 18/12/2020

DEPTH (m)	SOIL DESCRIPTION	LEGEND	GROUND WATER	UNDRAINED SHEAR STRENGTH (kPa)	SCALA (BLOWS PER 100mm)
				0 50 100 150	0 5 10 15
	Topsoil				
	[NON ENGINEERED FILL] clayey SILT, dark brown mottled light orangey brown, very stiff, moist to dry				
1	[ECBF] silty CLAY, grey mottled light orangey brown, very stiff, moist @0.8m becomes wet				
	@1.2m becomes clayey SILT, light brown mottled light grey @1.3m becomes stiff				
	@1.5m becomes silty CLAY				
2	@1.8m becomes very wet				
	@2.2m becomes light brownish grey mottled light brown, very stiff				
	@2.6m becomes clayey SILT, bluish grey				
3	EOB @3.0m, Target depth reached				
4					
5					

NOTES: - No groundwater table was encountered.

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

09- 476 1417
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

021-134 3823



JOB NUMBER: 2282
ADDRESS: 43A Vipond Road & 20 Melia Place,
Stanmore Bay
WEATHER: Fine
PROJECT: Multiple Units
CLIENT: KIPG

HAND AUGER NO.
HA08
SHEET 1 OF 2
LOGGED: HQ
CHECKED: JS
DRILL DATE: 18/12/2020

DEPTH (m)	SOIL DESCRIPTION	LEGEND	GROUND WATER	UNDRAINED SHEAR STRENGTH (kPa)	SCALA (BLOWS PER 100mm)
				0 50 100 150	0 5 10 15
	Topsoil				
	[NON ENGINEERED FILL] clayey SILT, light brown mottled light orangey brown, whitish grey & dark brown, very stiff, moist				
1	@0.8m becomes silty CLAY, light brown mottled whitish grey & light orangey brown, stiff, wet				
	@1.2m becomes silty CLAY minor gravel, light brown mottled orangey brown & black, very stiff, moist				
2	@1.8m becomes silty CLAY, light brown mottled light orangey brown, grey & brown				
	@2.2m becomes light brown mottled grey & dark brown				
	@2.6m becomes dark brown mottled light brown & grey				
3					
	@3.8m becomes clayey SILT, dark grey mottled dark brown				
4	@4.3m becomes dark brown mottled dark grey, whitish grey & orangey brown				
	@4.5m becomes dark brown mottled black & greenish black				
	@4.6m becomes light grey mottled dark brown & light brown				
	[ECBF] clayey SILT, light orangey brown mottled light grey, very stiff, moist to wet				
5	EOB @5.0m, Target depth reached				

NOTES: - No groundwater table was encountered.

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

09- 476 1417
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

021-134 3823



JOB NUMBER: 2282
ADDRESS: 43A Vipond Road & 20 Melia Place,
Stanmore Bay
WEATHER: Fine
PROJECT: Multiple Units
CLIENT: KIPG

HAND AUGER NO.

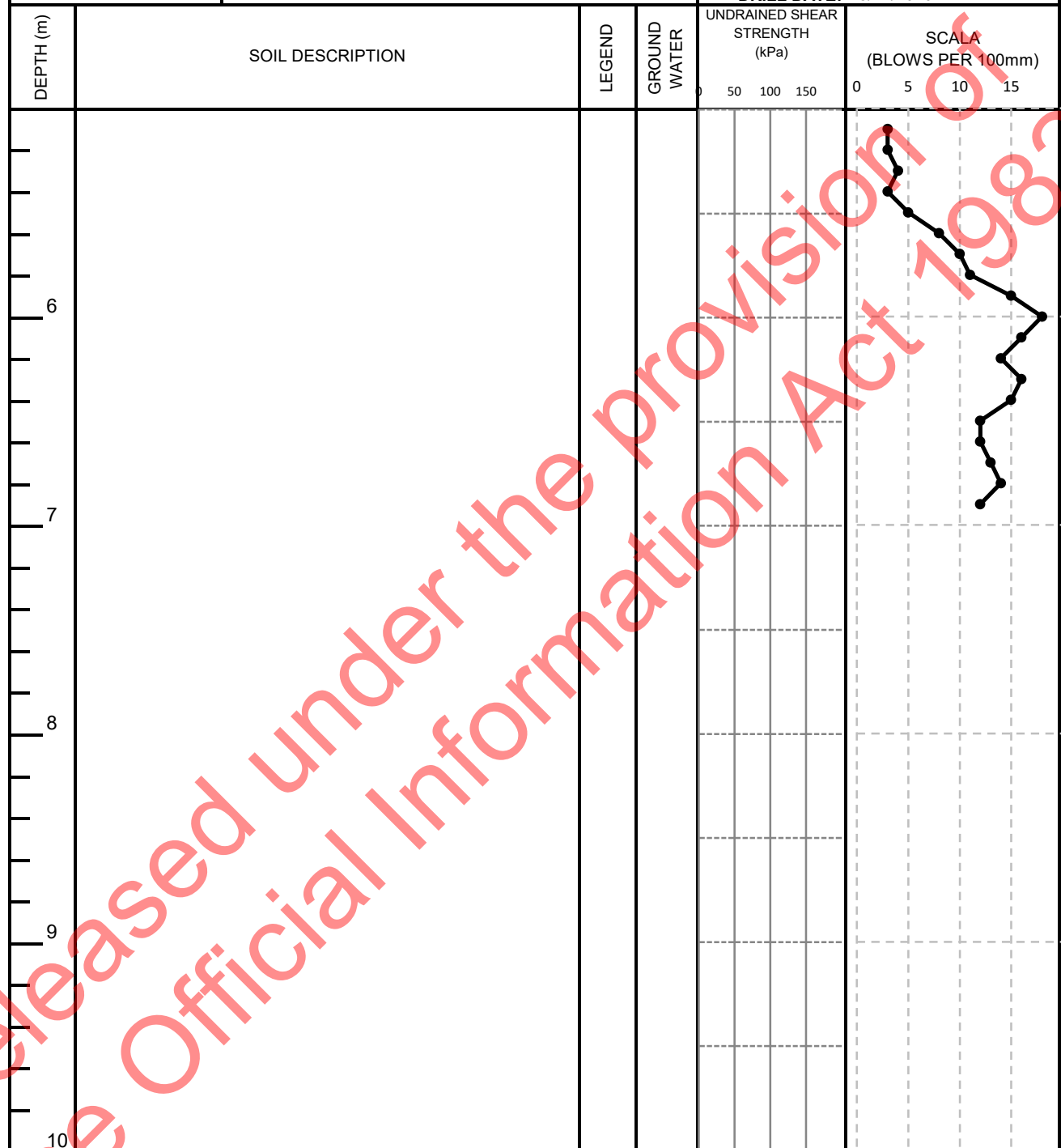
HA08

SHEET 2 OF 2

LOGGED: HQ

CHECKED: JS

DRILL DATE: 18/12/2020



NOTES: - No groundwater table was encountered.

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

09- 476 1417
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

021-134 3823



JOB NUMBER: 2282
ADDRESS: 2282_43A Vipond Road & 20 Melia Place, Stanmore Bay
WEATHER: Fine
PROJECT: Multi Unit development
CLIENT: KIPG

BOREHOLE NO.
MH01
SHEET 1 OF 3
LOGGED: JS
CHECKED: GK
DRILL DATE: 15/01/2021

DEPTH (m)	SOIL DESCRIPTION	LEGEND	Shear Vane (kPa)	Core Recovery (%)	SPT N-value	Notes
	Topsoil			20 40 60 80		
1	[NON ENGINEERED FILL] silty CLAY minor gravel, grey mottled brown				0 1 1 1 N=3	
2	@2.0m becomes brown mottled grey					
3					1 2 1 1 N=5	
4	@3.6m becomes brown mottled grey and dark brown					
5	@4.6m becomes silty CLAY minor gravel with traces of wood/organic matter				14 N=bounding for 35mm	

NOTES:

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ

09- 476 1417

021-134 3823

DIRECTOR: GEOFFREY KANG

BE(CIVIL), ME(CIVIL), CPENG, MIPENZ



JOB NUMBER: 2282
ADDRESS: 2282_43A Vipond Road & 20 Melia Place, Stanmore Bay
WEATHER: Fine
PROJECT: Multi Unit development
CLIENT: KIPG

BOREHOLE NO.
MH01
SHEET 2 OF 3
LOGGED: JS
CHECKED: GK
DRILL DATE: 15/01/2021

DEPTH (m)	SOIL DESCRIPTION	LEGEND	Shear Vane (kPa)	Core Recovery (%)	SPT N-value	Notes
6	@6.0m becomes silty CLAY minor gravel				2 2 2 2 N=8	
7	@6.5m becomes dark brown mottled brown and grey					
	[ALLUVIUM] clayey SILT, bluish grey mottled brown and orangey brown				3 2 3 6 N=14	
8	[ECBF] weathered SAND STONE, dark bluish grey with traces of orangey brown					
9	@9.0m becomes dark grey				13 14 14 19 N=50	
10						

NOTES:

GEOSTUDIO LIMITED

GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ

09- 476 1417

021-134 3823

DIRECTOR: GEOFFREY KANG

BE(CIVIL), ME(CIVIL), CPENG, MIPENZ



JOB NUMBER: 2282
ADDRESS: 2282_43A Vipond Road & 20 Melia Place, Stanmore Bay
WEATHER: Fine
PROJECT: Multi Unit development
CLIENT: KIPG

BOREHOLE NO.
MH01
SHEET 3 OF 3
LOGGED: JS
CHECKED: GK
DRILL DATE: 15/01/2021

DEPTH (m)	SOIL DESCRIPTION	LEGEND	Shear Vane (kPa)	Core Recovery (%)	SPT N-value	Notes
				0 20 40 60 80		
11	@10.0m becomes weathered SILT STONE, dark brown mottled dark grey				26 24 N=50+ for 55mm	
12						
	EOB @12.5m target depth reached.				N=50+ for 35mm	
13						
14						
15						

NOTES:

GEOSTUDIO LIMITED

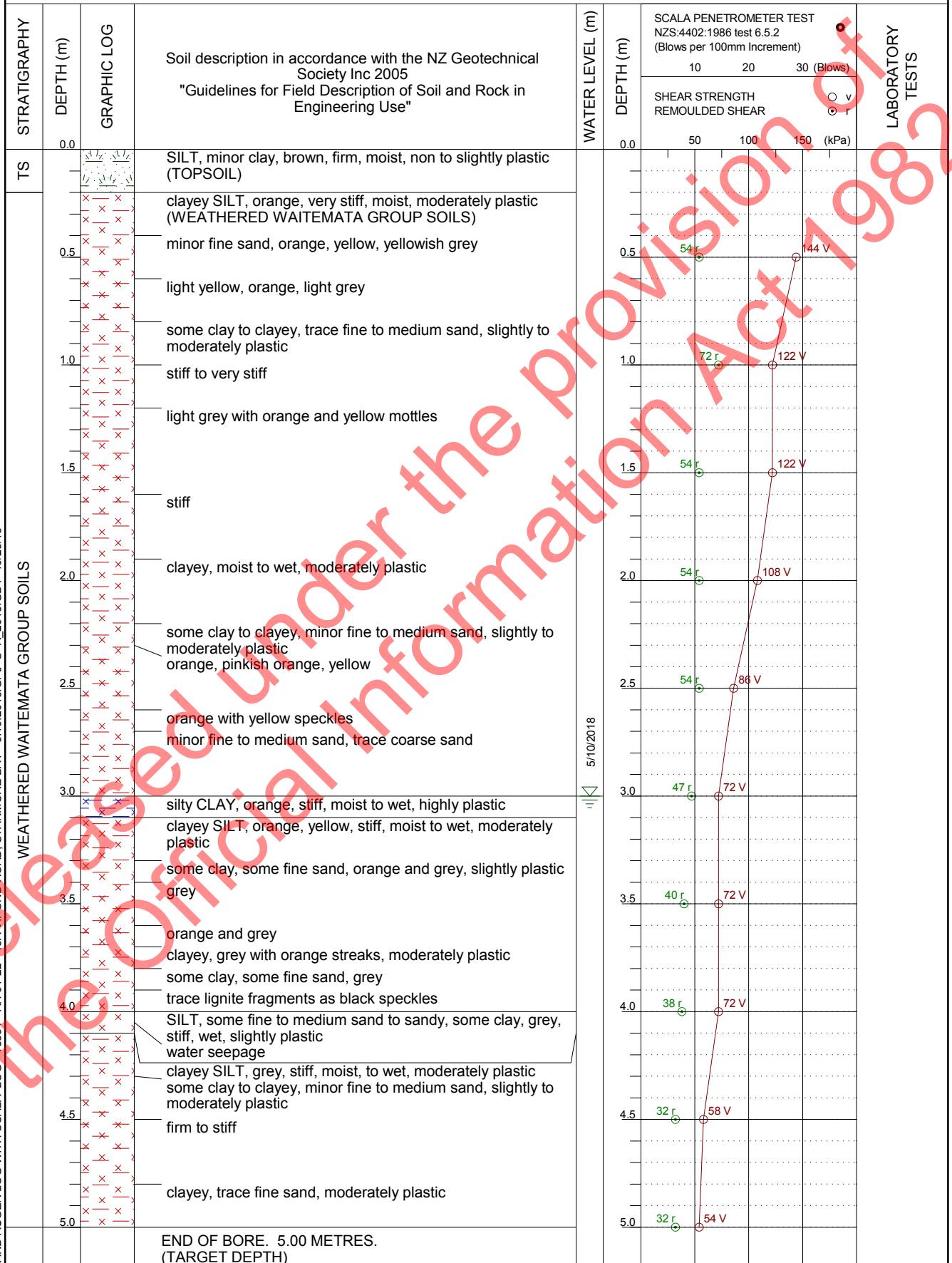
GEOTECHNICAL ENGINEERS

WWW.GEOSTUDIO.CO.NZ
DIRECTOR: GEOFFREY KANG

09- 476 1417 021-134 3823
BE(CIVIL), ME(CIVIL), CPENG, MIPENZ

Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 4/10/18
Date Finished: 4/10/18

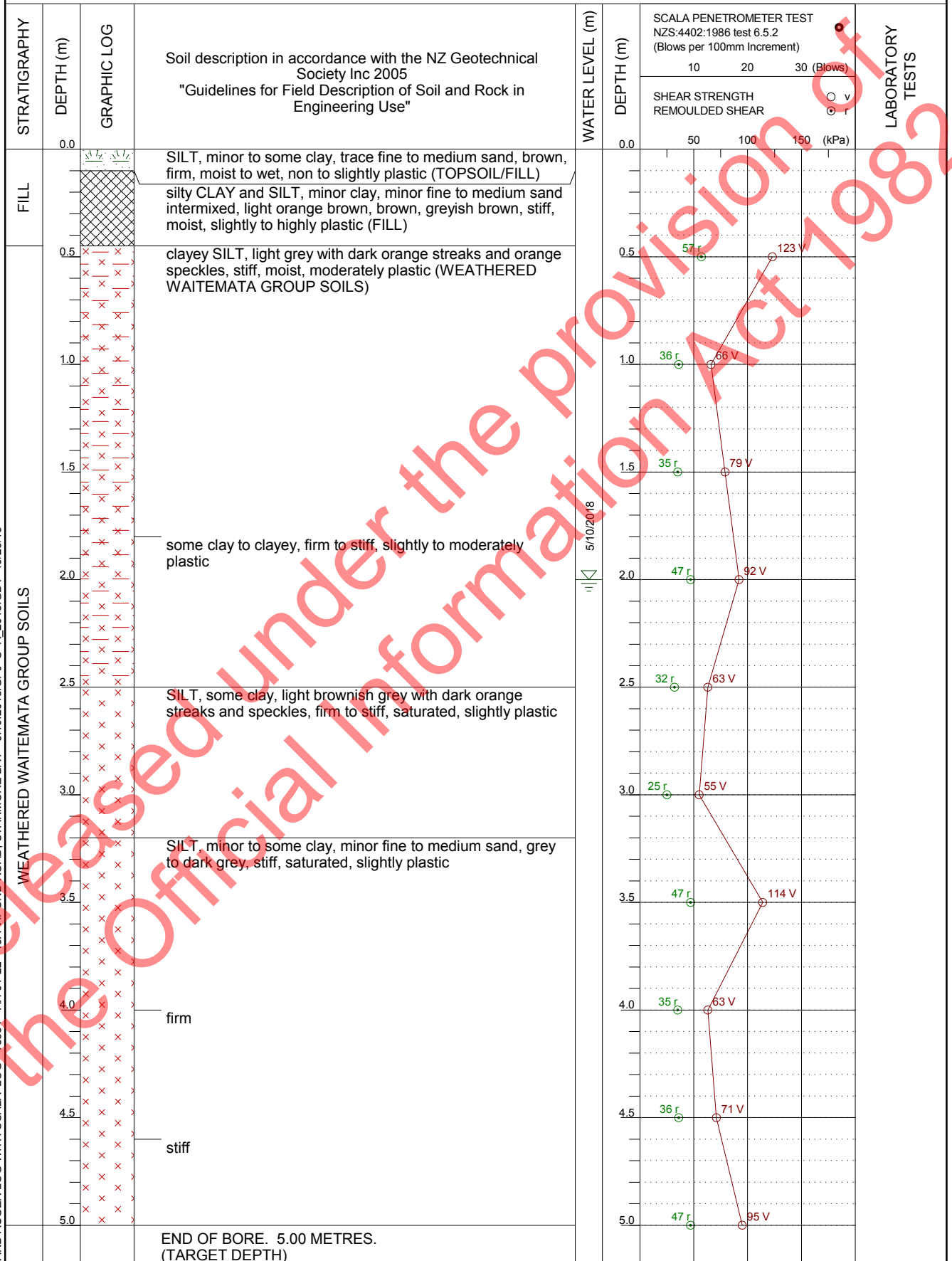
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 3.0m 5/10/2018

Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Slightly Sloping, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: CD
Date Started: 4/10/18
Date Finished: 4/10/18

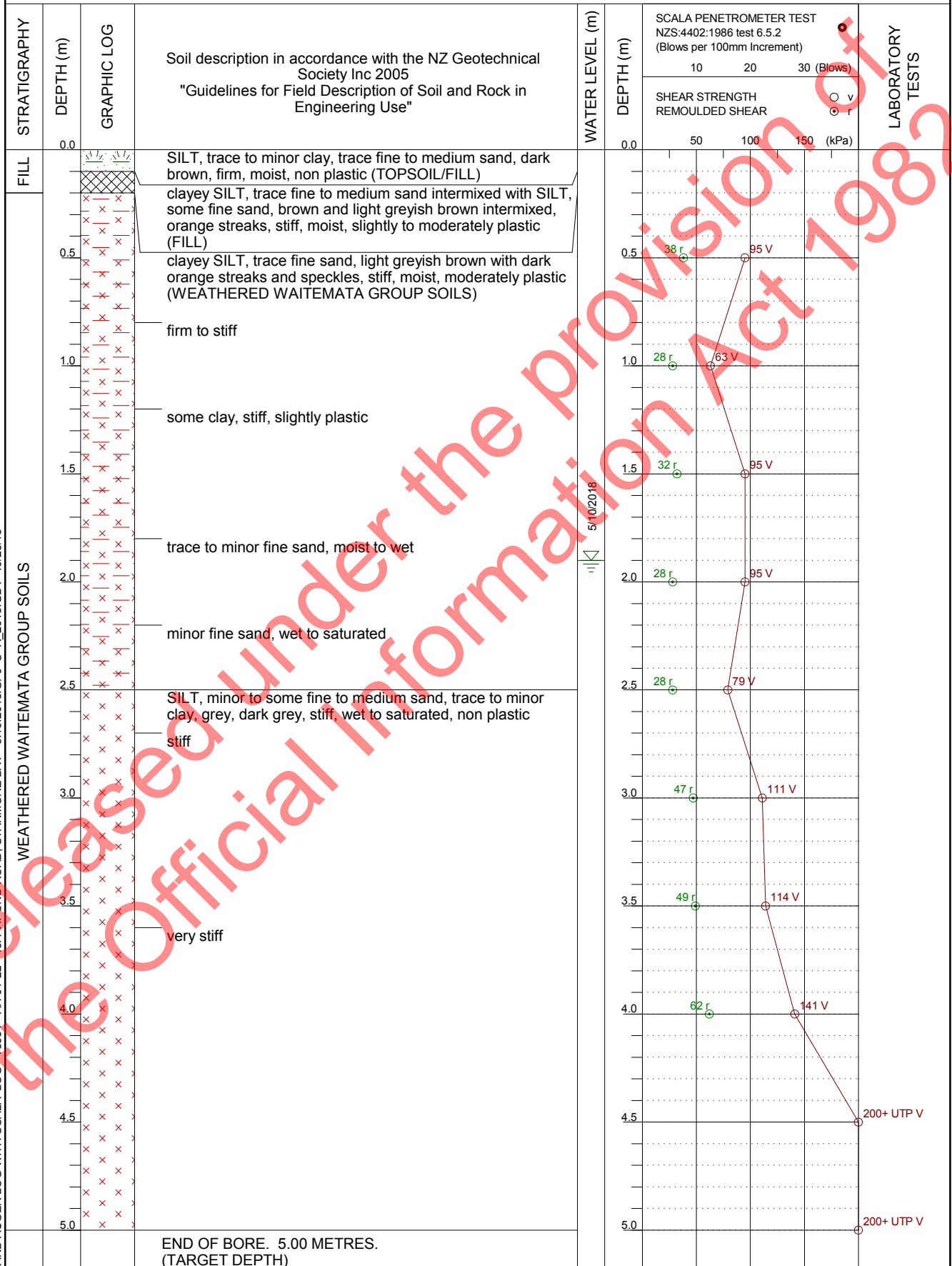
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 2.0m 5/10/2018

Logged By: CD
Shear Vane No - Calibration Date: GEO361 - 27/02/2018
Surface Conditions: Sloping, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: CD
Date Started: 4/10/18
Date Finished: 4/10/18

Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 1.9m 5/10/2018

Logged By: CD
Shear Vane No - Calibration Date: GEO361 - 27/02/2018
Surface Conditions: Sloping, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018, GPJ S+R 2013, GDT 10/25/18



Soil & Rock Consultants
For well-grounded solutions

CLIENT: Silverdale RSA

PROJECT: Geotechnical Investigation, 43A Vipond Road & 20 Melia Place, Stanmore Bay

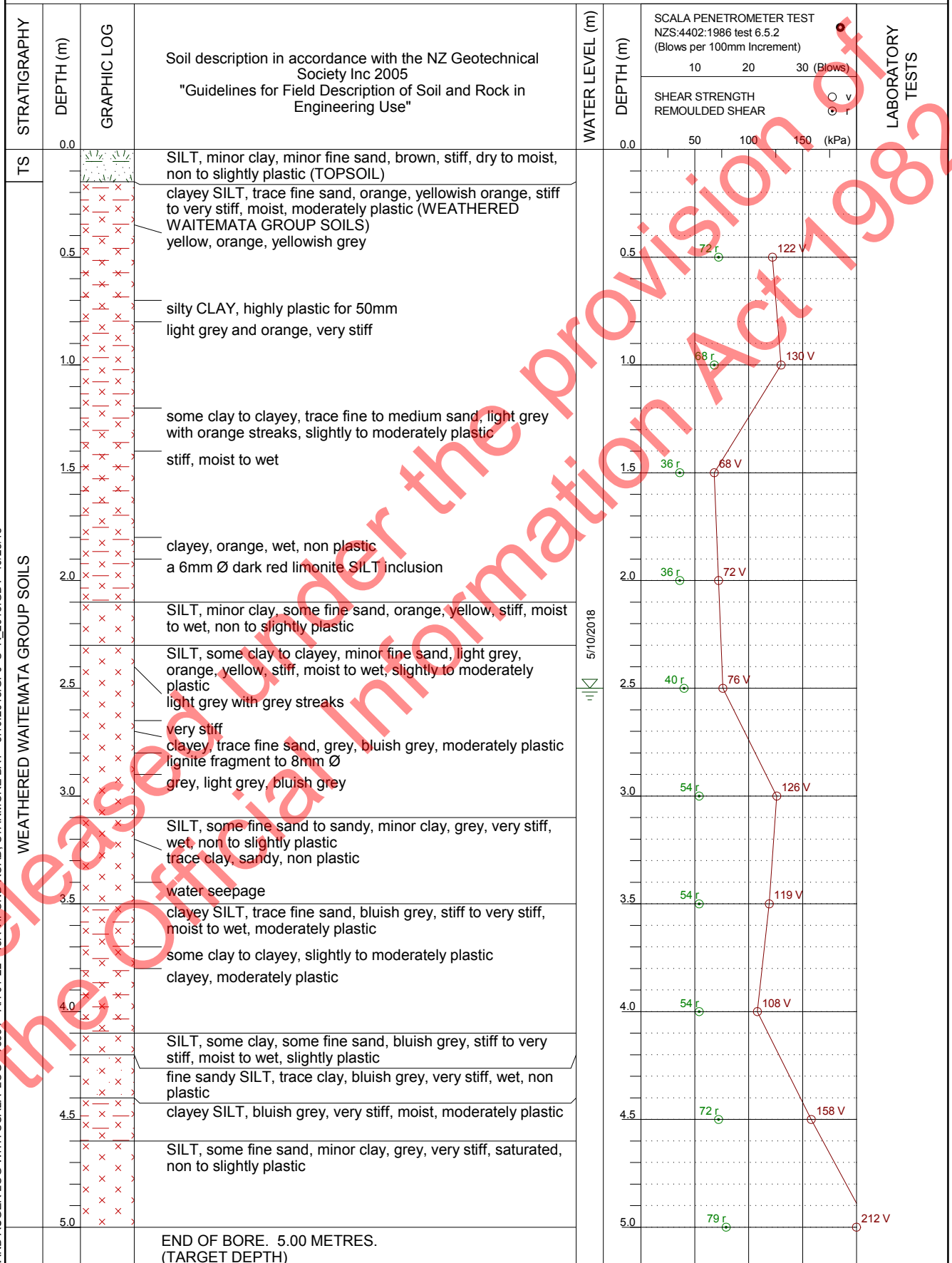
Auger Hole No: AH104

Sheet 1 of 1

Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 4/10/18
Date Finished: 4/10/18

Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 2.5m 5/10/2018

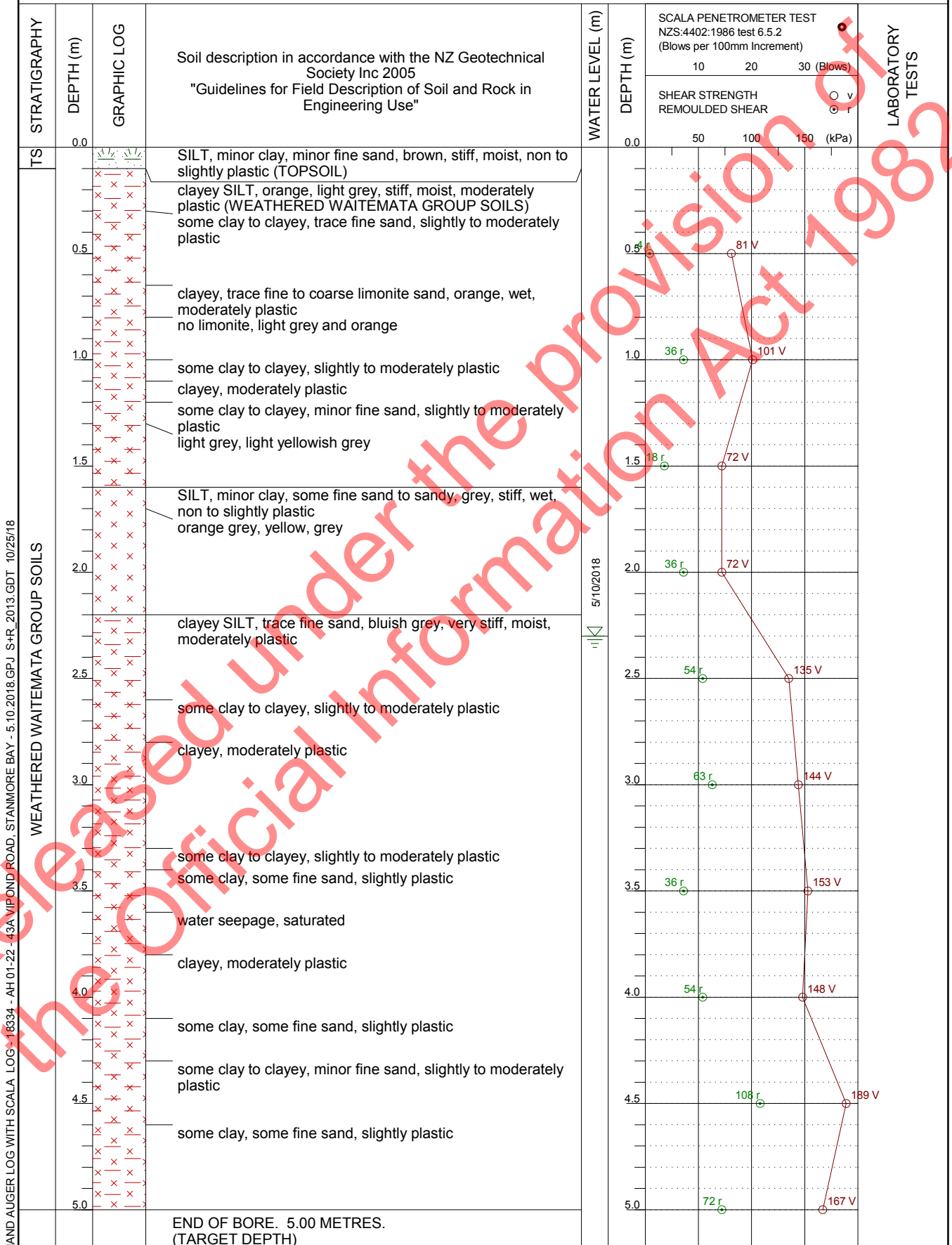
Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Slightly to Moderately Sloping, Grass



HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

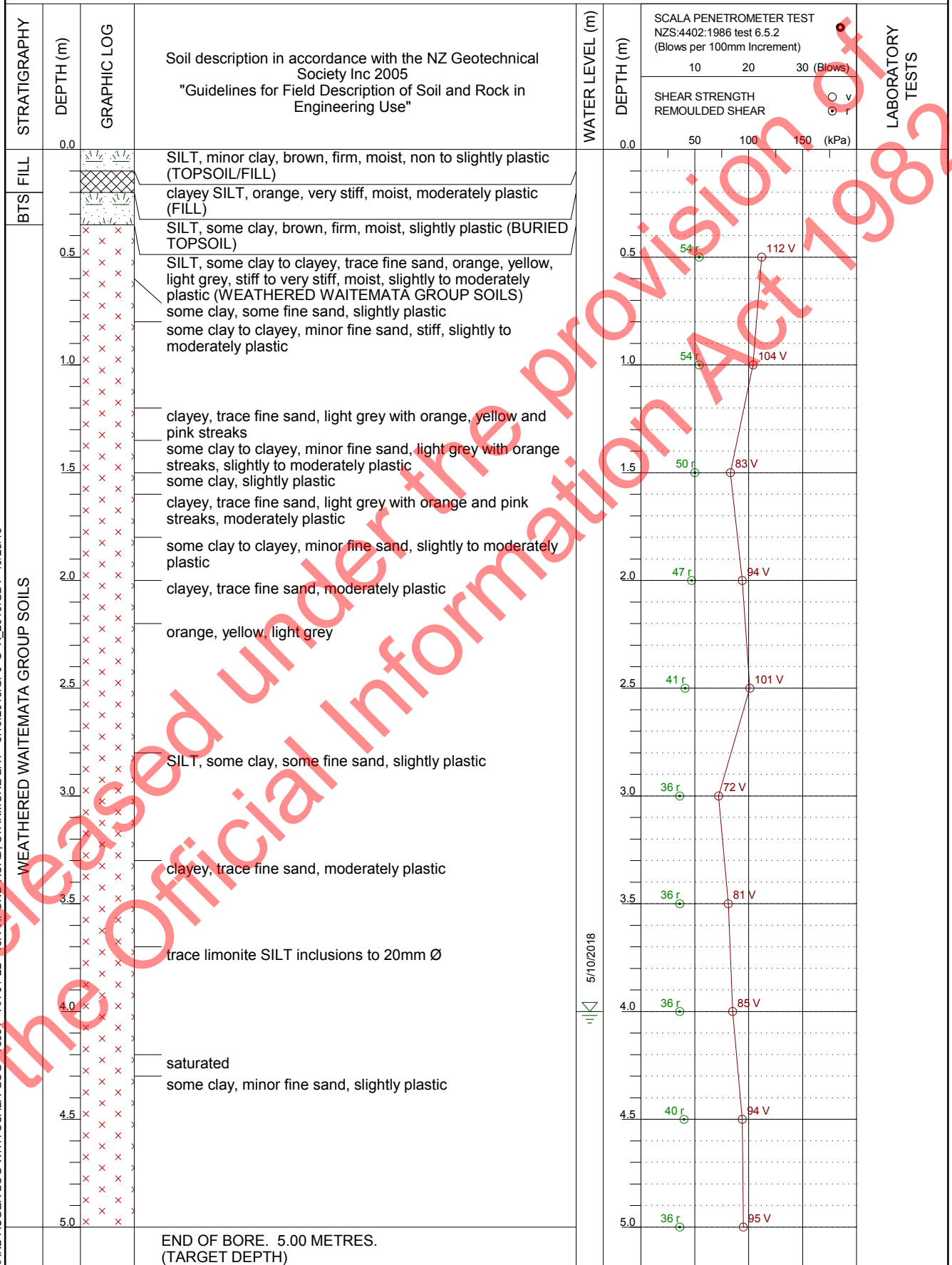
Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 4/10/18
Date Finished: 4/10/18

Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 2.3m 5/10/2018

Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Slightly to Moderately Sloping, Grass


Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 3/10/18
Date Finished: 3/10/18

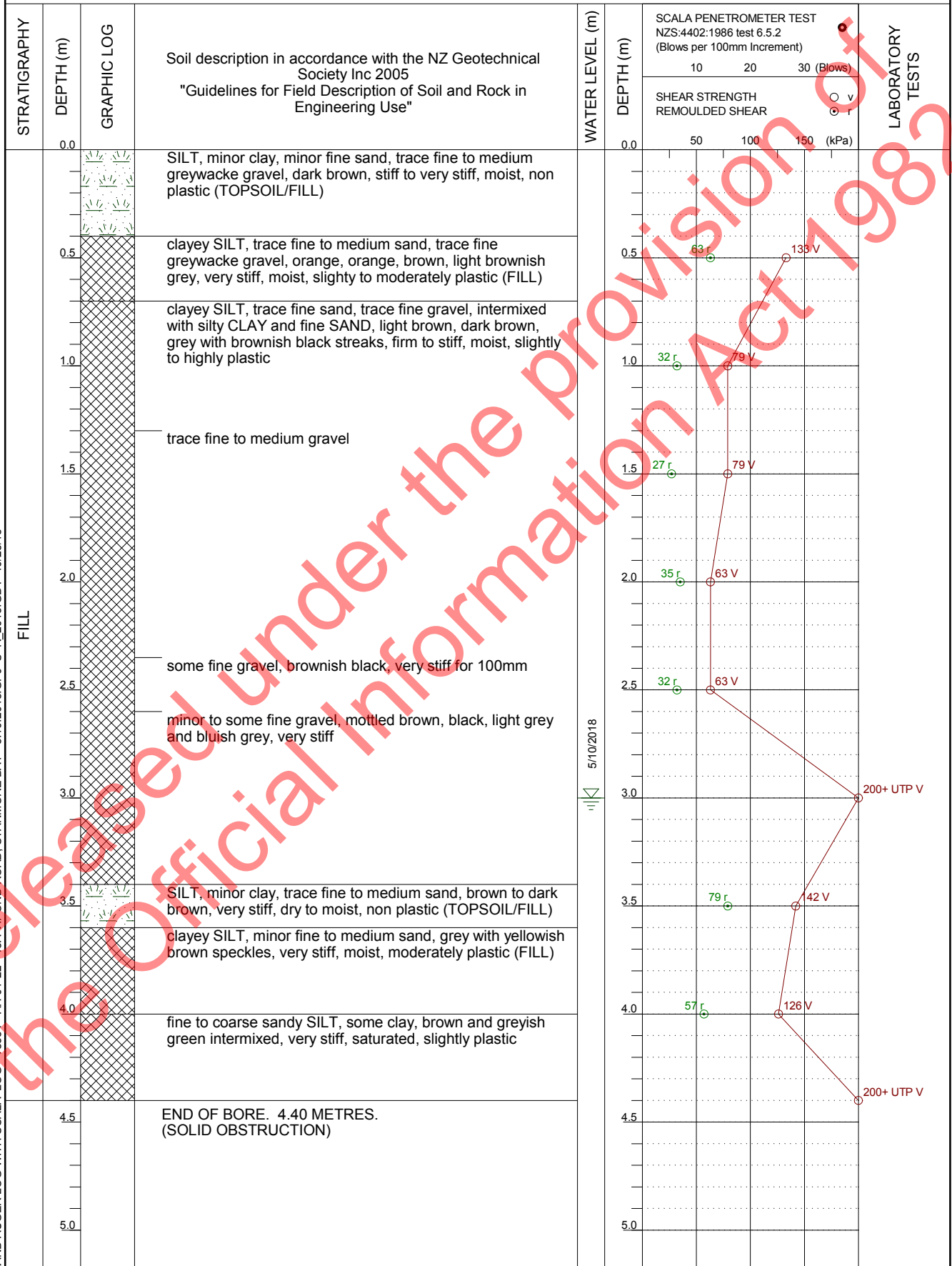
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 4.0m 5/10/2018

Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Slightly Sloping, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: CD
Date Started: 4/10/18
Date Finished: 4/10/18

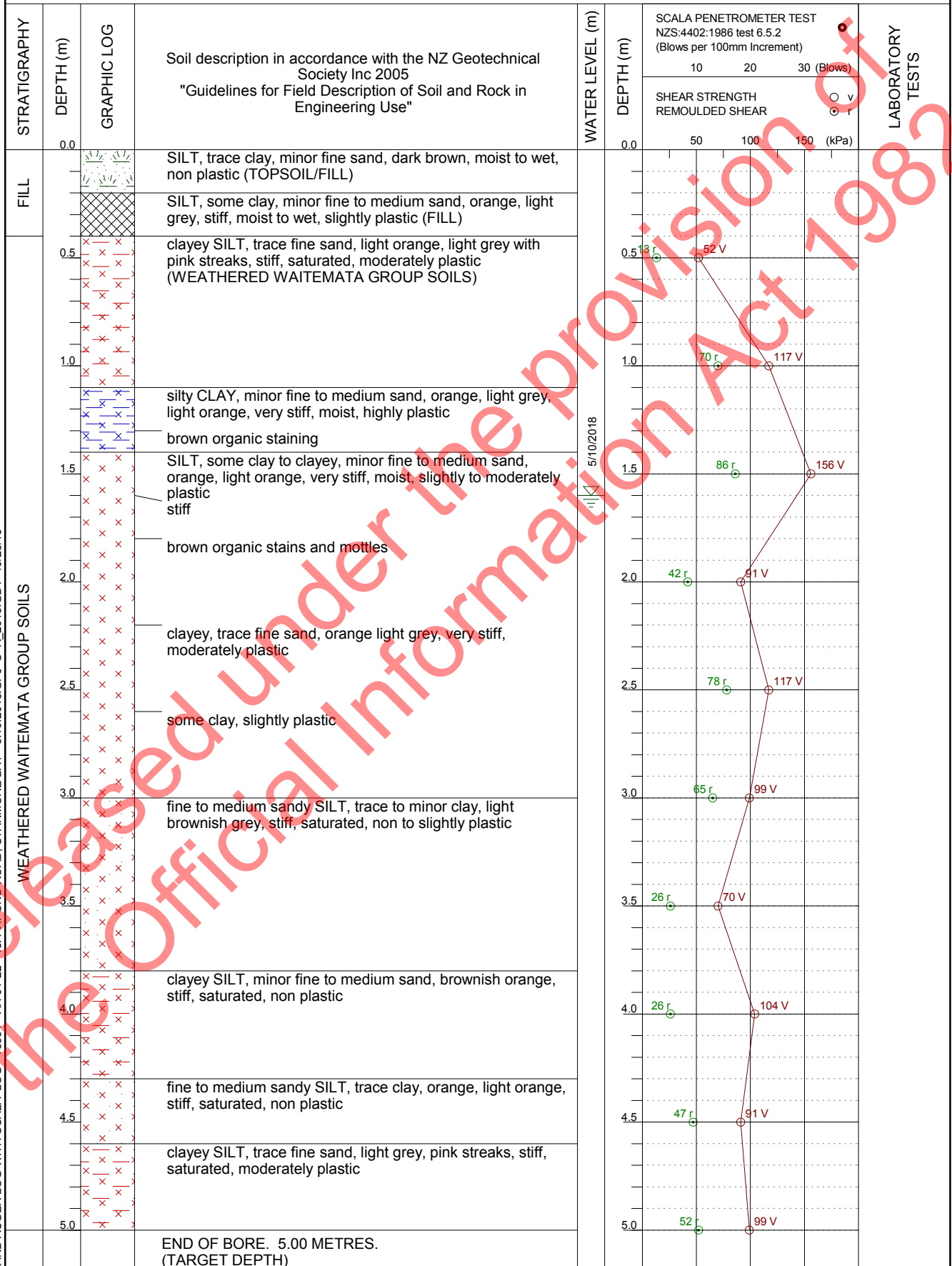
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 3.0m 5/10/2018

Logged By: CD
Shear Vane No - Calibration Date: GEO361 - 27/02/2018
Surface Conditions: Level, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH101-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: NC
Date Started: 3/10/18
Date Finished: 3/10/18

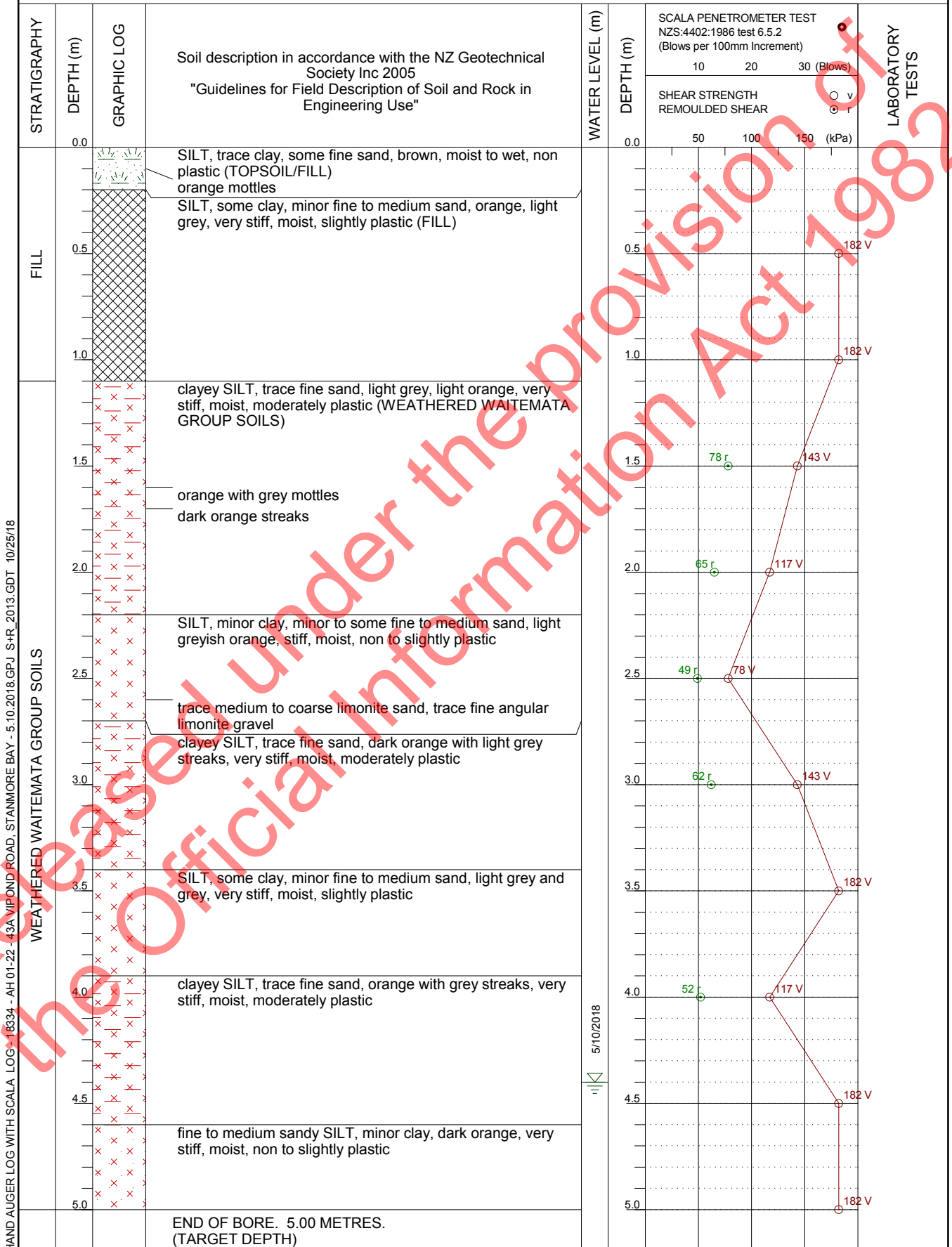
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 1.6m 5/10/2018

Logged By: NC
Shear Vane No - Calibration Date: GEO119 - 23/11/2017
Surface Conditions: Near Level, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: NC
Date Started: 3/10/18
Date Finished: 3/10/18

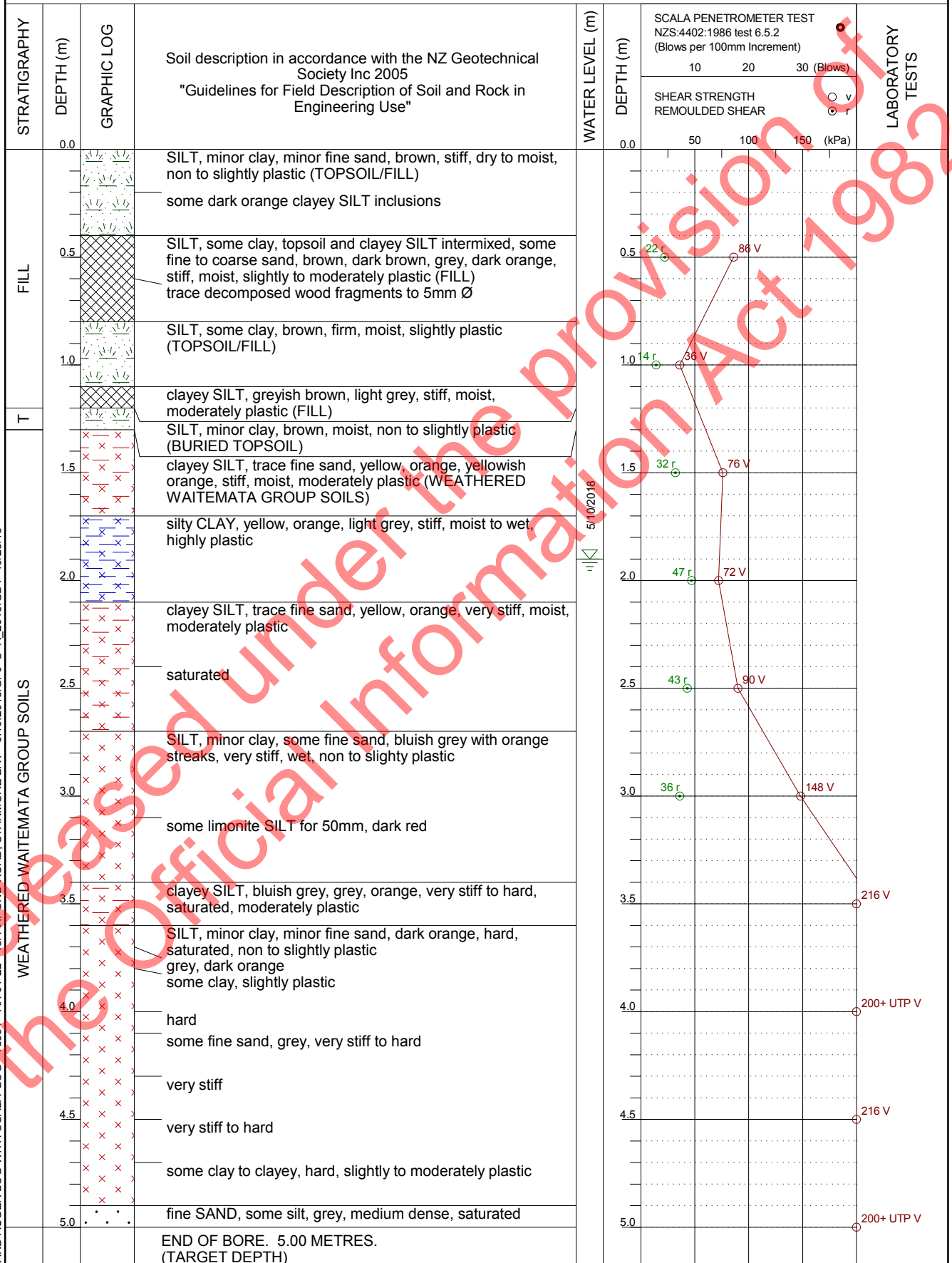
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 4.4m 5/10/2018

Logged By: NC
Shear Vane No - Calibration Date: GEO119 - 23/11/2017
Surface Conditions: Slightly Sloping, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 3/10/18
Date Finished: 3/10/18

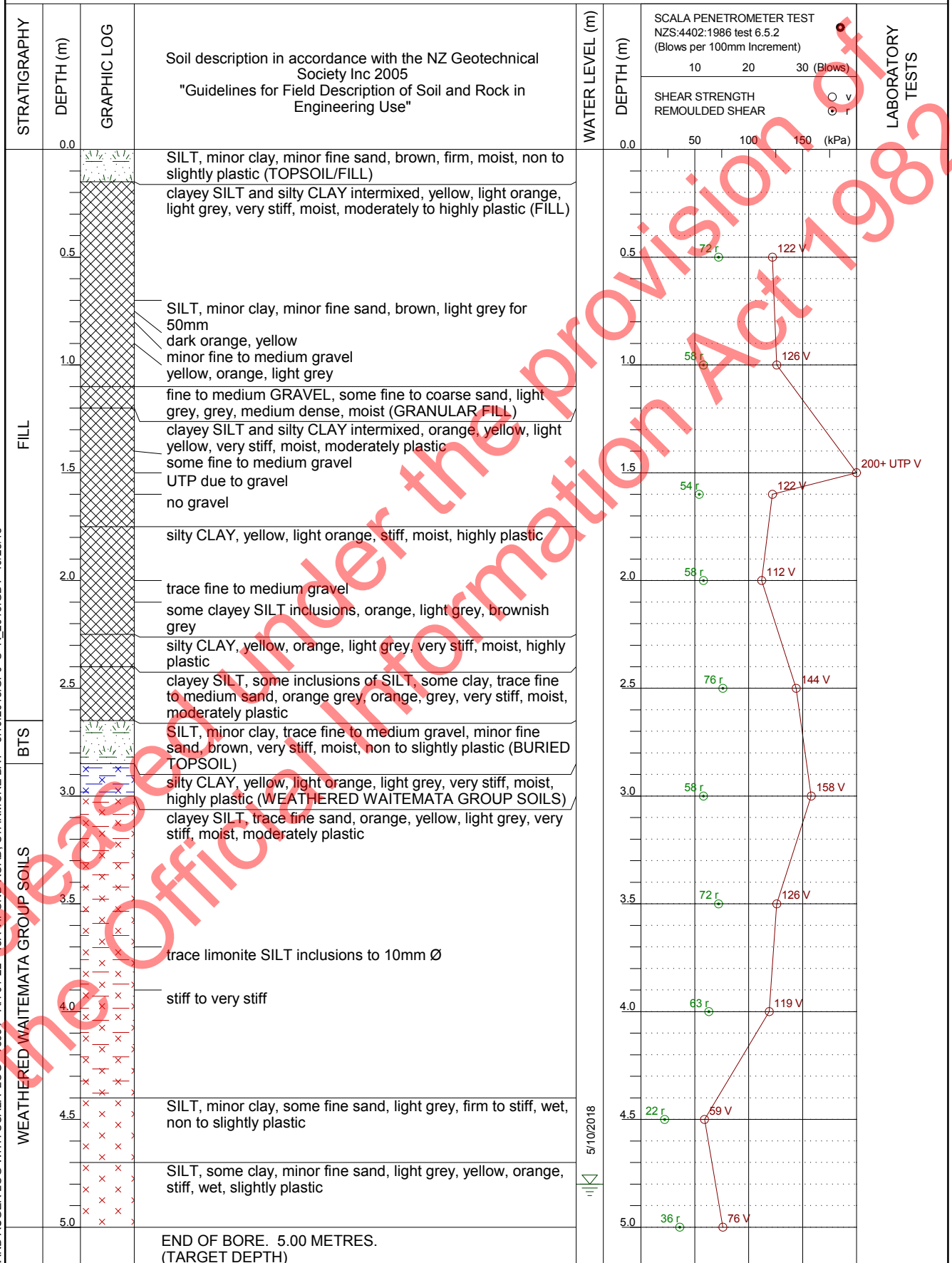
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 1.9m 5/10/2018

Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Slightly Sloping, Trees


HAND AUGER LOG WITH SCALA LOG - 18334 - AH101-22 - 43A VIPOND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R 2013.GDT 10/25/18

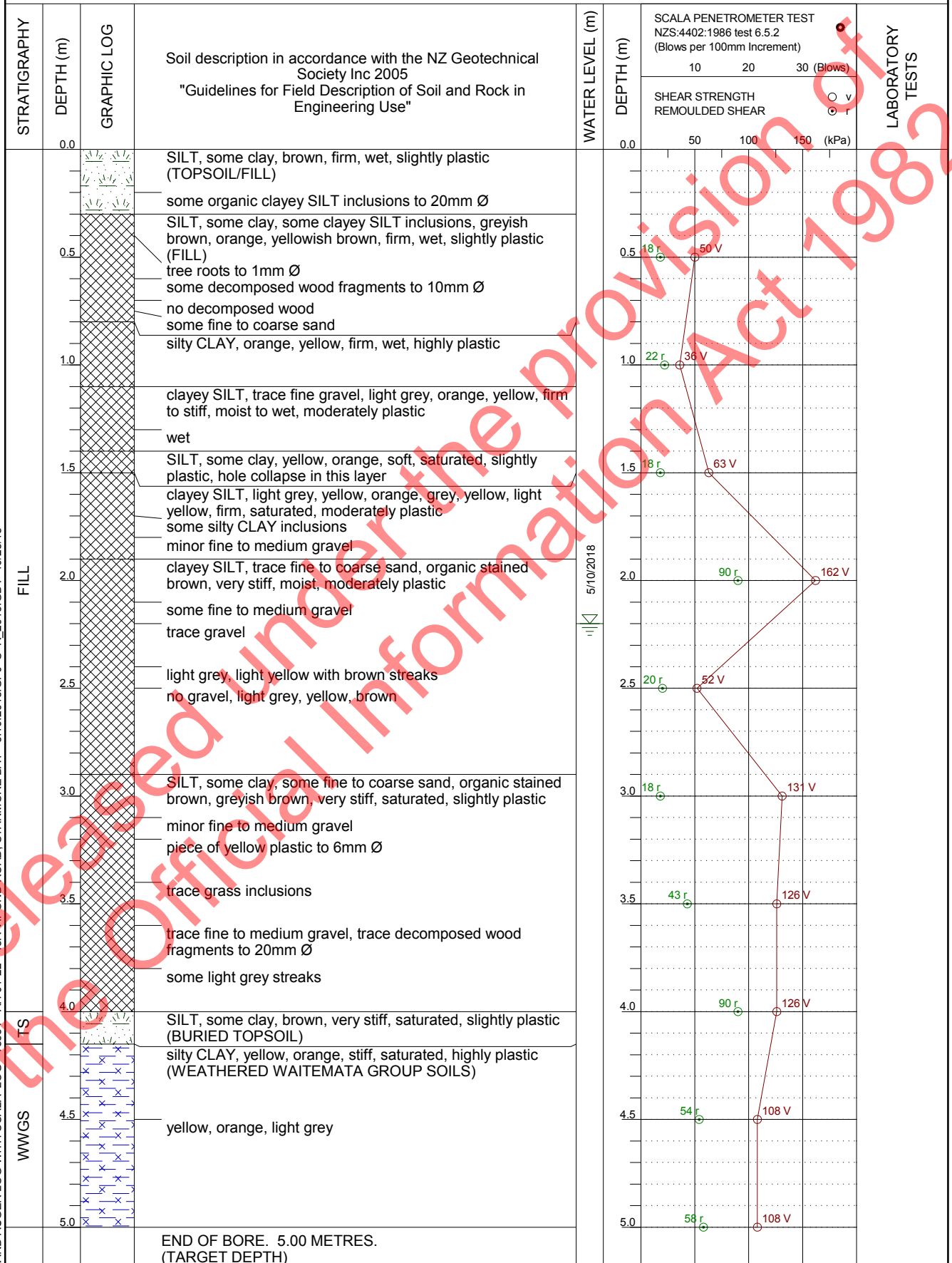
Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 4/10/18
Date Finished: 4/10/18

Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 4.8m 5/10/2018

Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Moderately Sloping, Grass


Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 3/10/18
Date Finished: 3/10/18

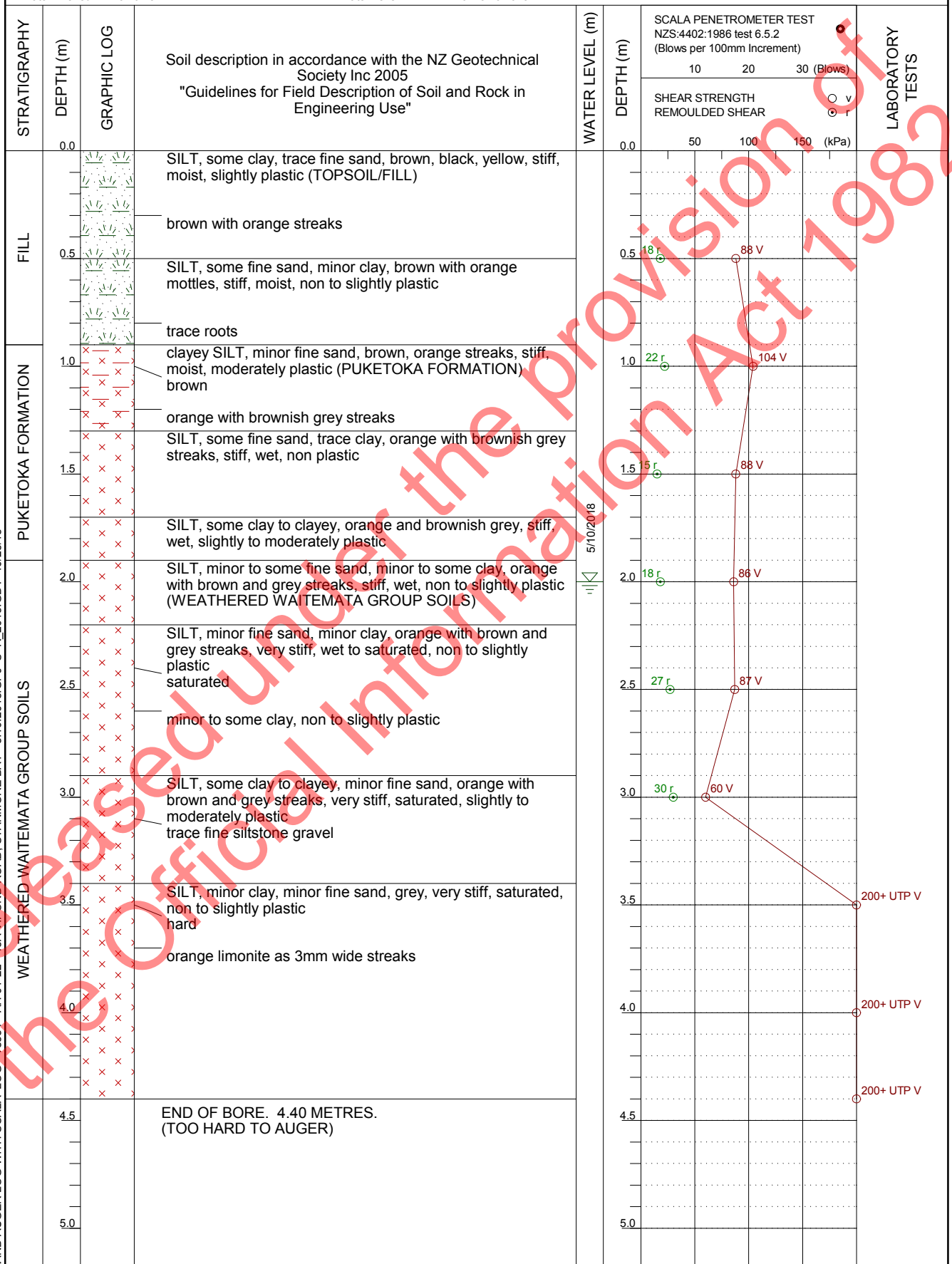
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 2.2m 5/10/2018

Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Moderately Sloping, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

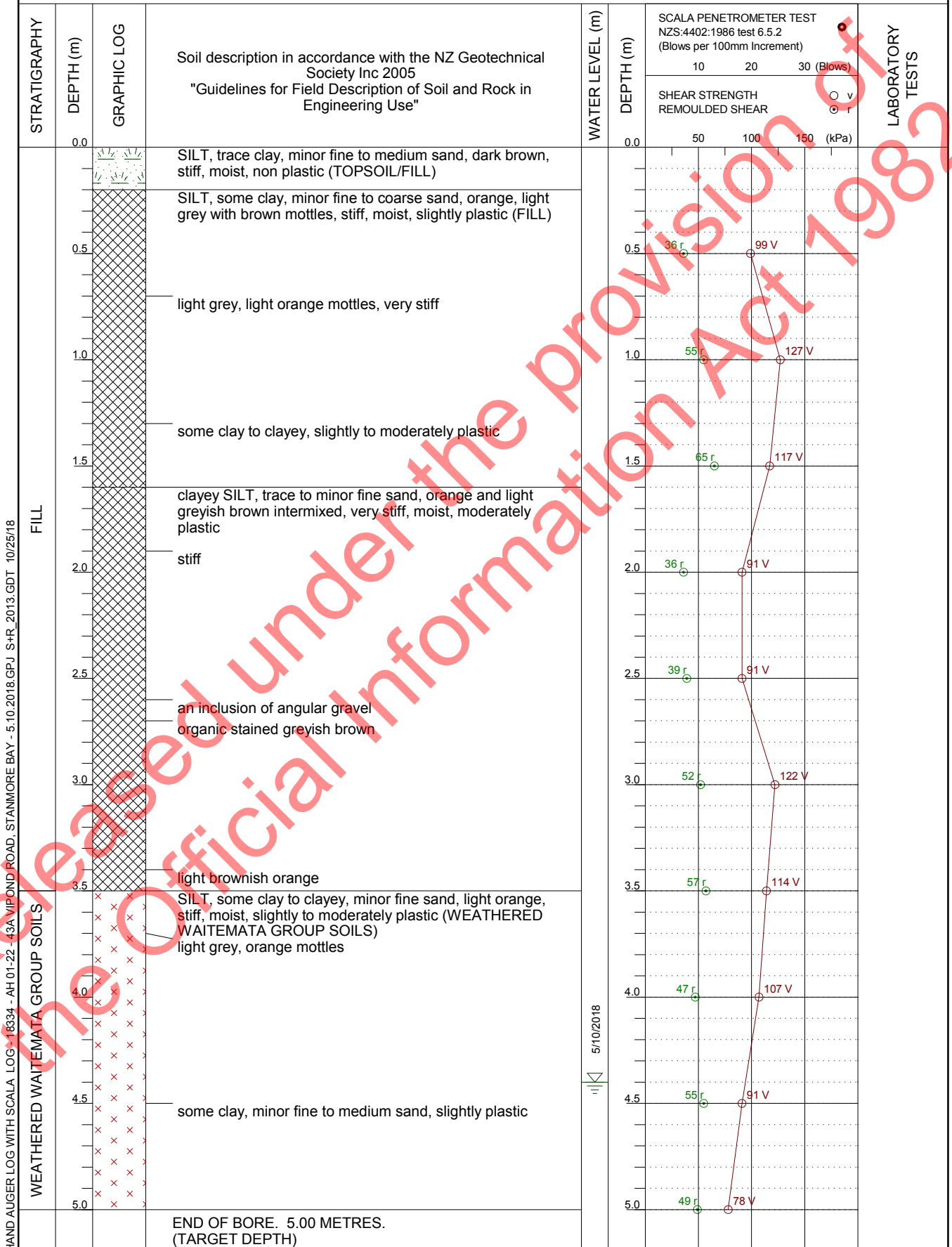
Drill Type: 50mm Hand Auger
Drilled By: NG
Date Started: 3/10/18
Date Finished: 3/10/18

Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 2.0m 5/10/2018

Logged By: NG
Shear Vane No - Calibration Date: GEO122 - 1/12/2017
Surface Conditions: Sloping, Soil


Drill Type: 50mm Hand Auger
Drilled By: NC
Date Started: 3/10/18
Date Finished: 3/10/18

Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 4.4m 5/10/2018

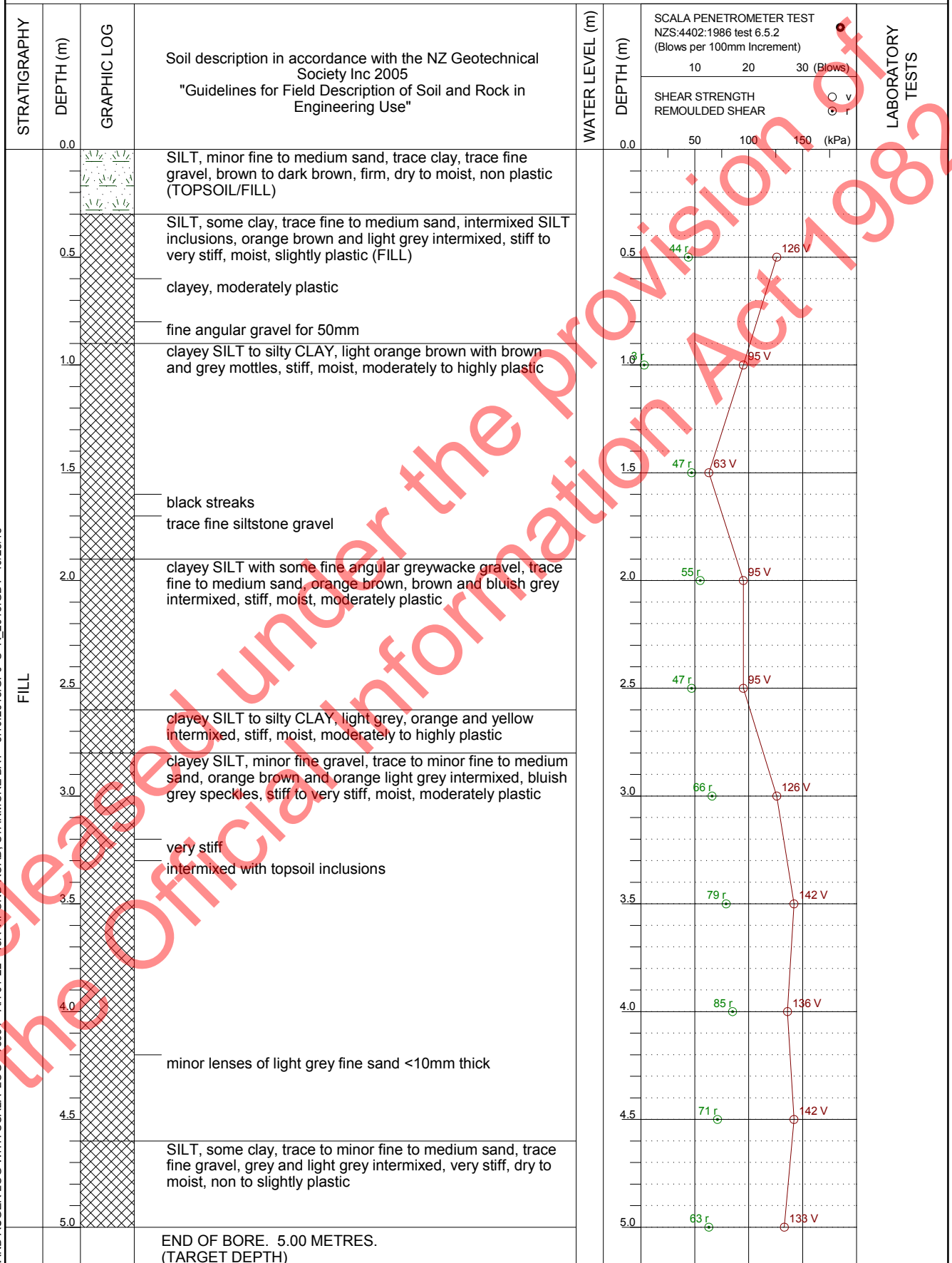
Logged By: NC
Shear Vane No - Calibration Date: GEO119 - 23/11/2017
Surface Conditions:


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

5/10/2018

Drill Type: 50mm Hand Auger
Drilled By: CD
Date Started: 3/10/18
Date Finished: 3/10/18

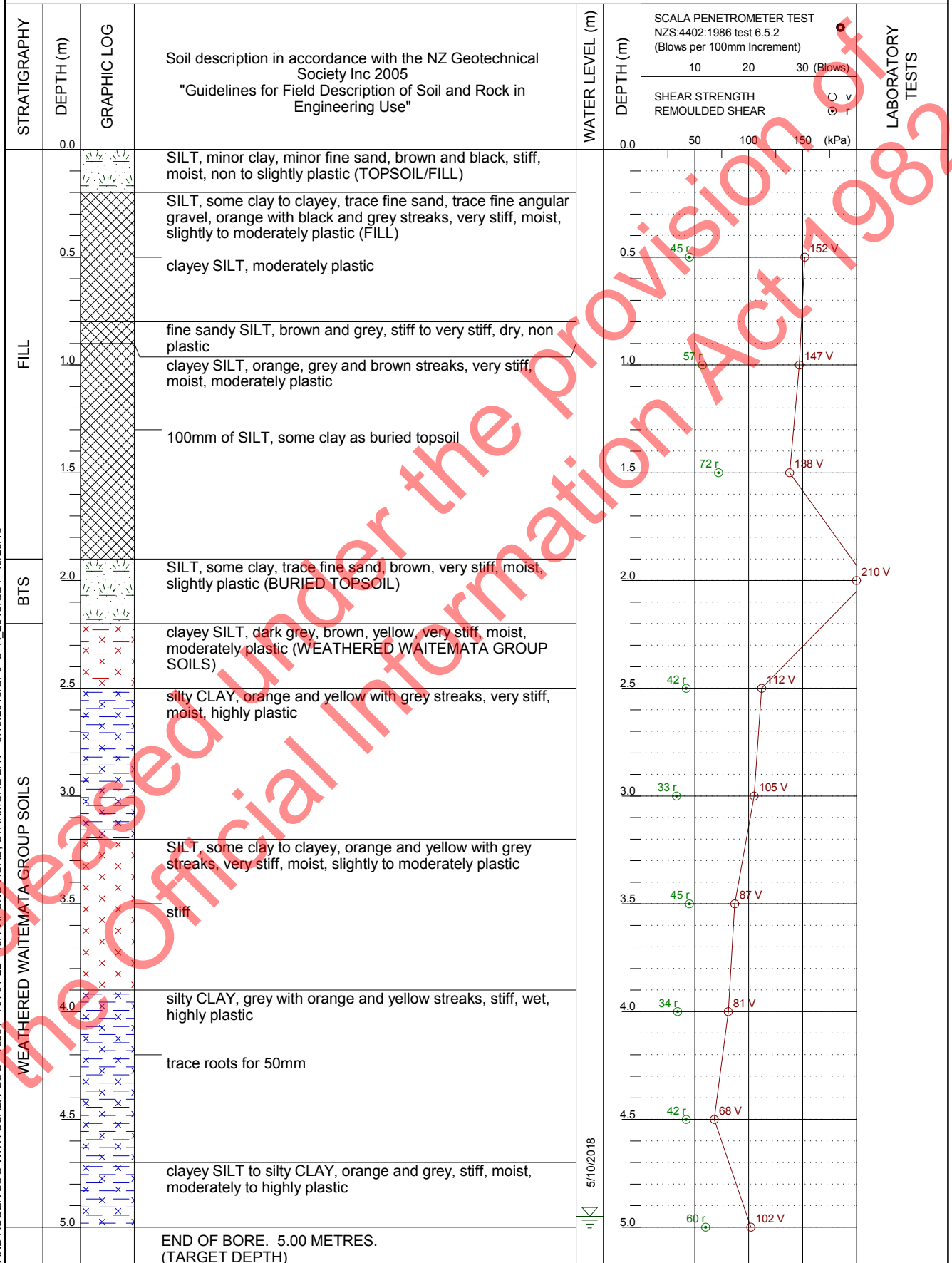
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: Not Encountered

Logged By: CD
Shear Vane No - Calibration Date: GEO361 - 27/02/2018
Surface Conditions: Sloping, Soil


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: NG
Date Started: 3/10/18
Date Finished: 3/10/18

Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 5.0m 5/10/2018

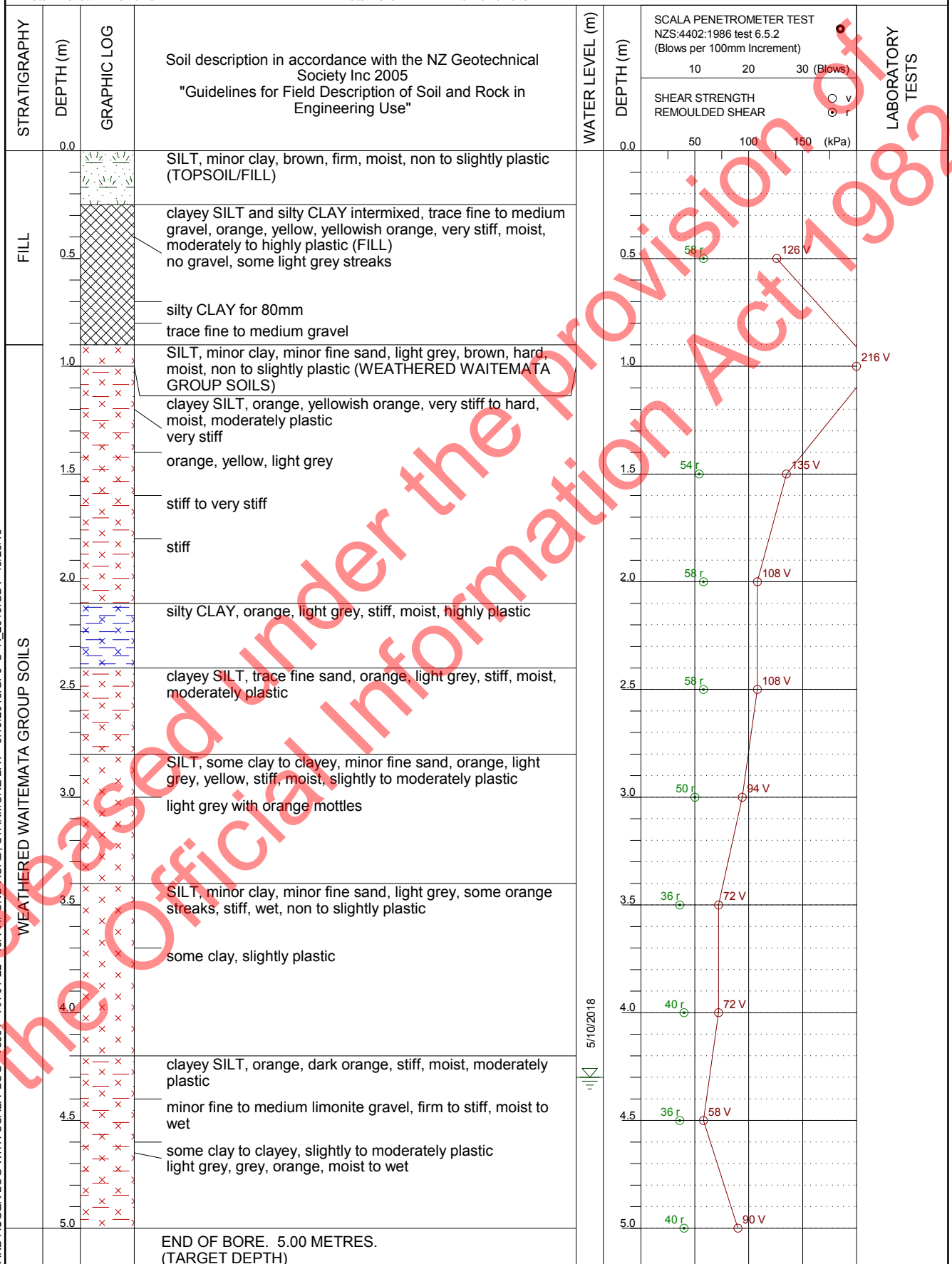
Logged By: NG
Shear Vane No - Calibration Date: GEO122 - 1/12/2017
Surface Conditions: Level, Grass


Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 3/10/18
Date Finished: 3/10/18

Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 4.3m 5/10/2018

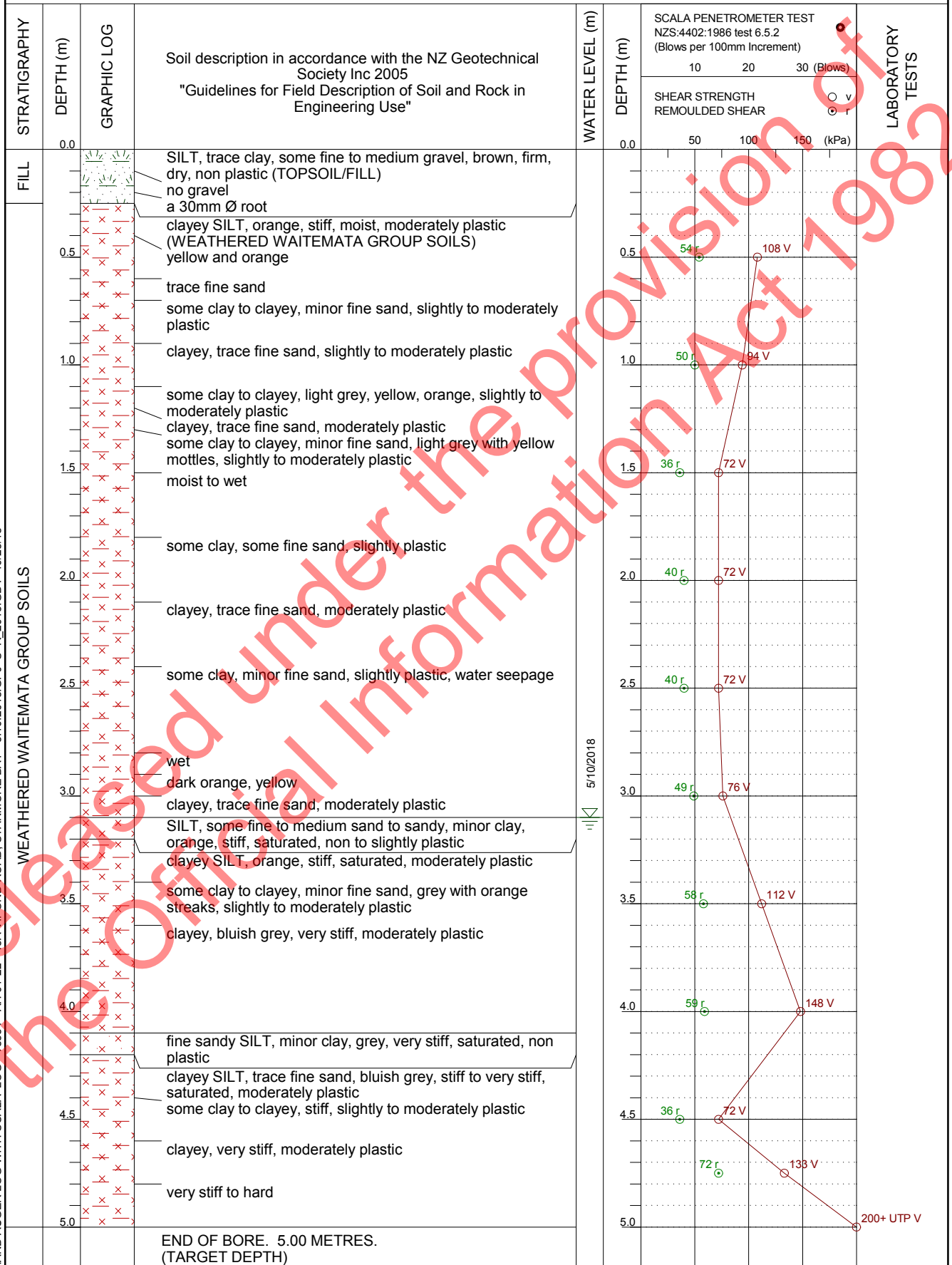
Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Slightly Sloping, Garden

HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R 2013.GDT 10/25/18



Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 5/10/18
Date Finished: 5/10/18

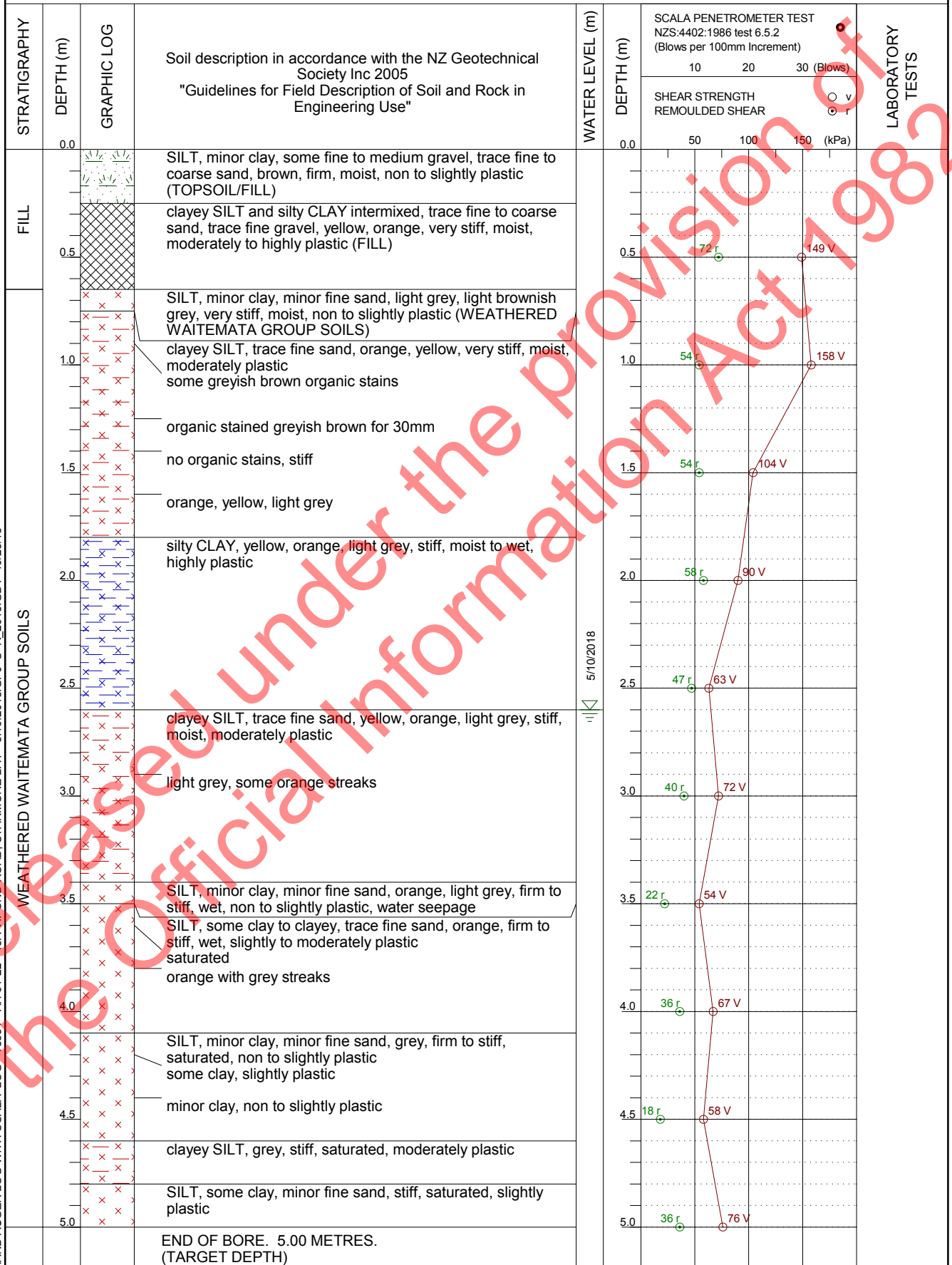
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 3.1m 5/10/2018

Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Slightly Sloping, Garden


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: DEG
Date Started: 3/10/18
Date Finished: 3/10/18

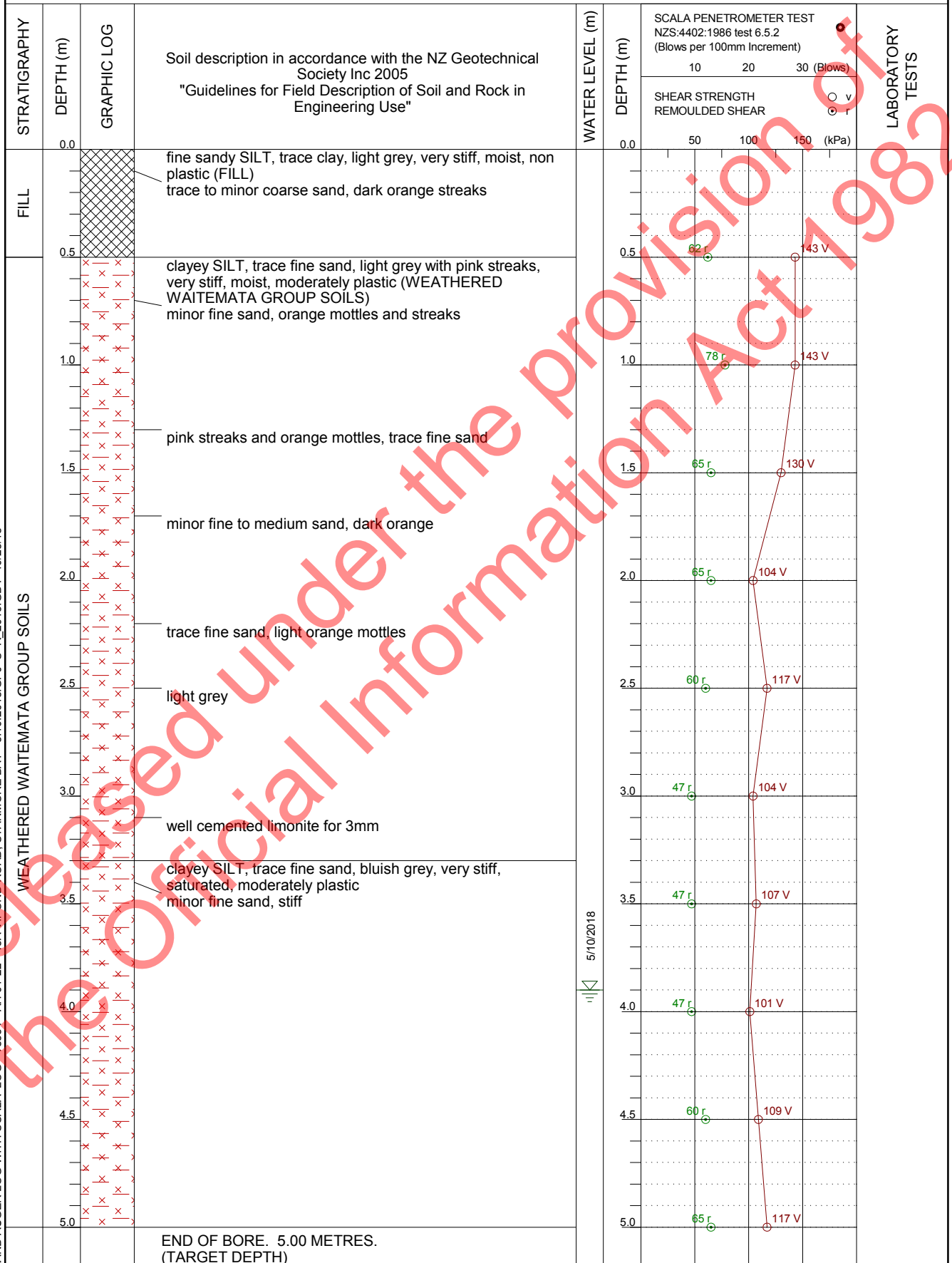
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 2.6m 5/10/2018

Logged By: DEG
Shear Vane No - Calibration Date: DR1768 - 21/12/2017
Surface Conditions: Slightly Sloping, Garden


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: NC
Date Started: 3/10/18
Date Finished: 3/10/18

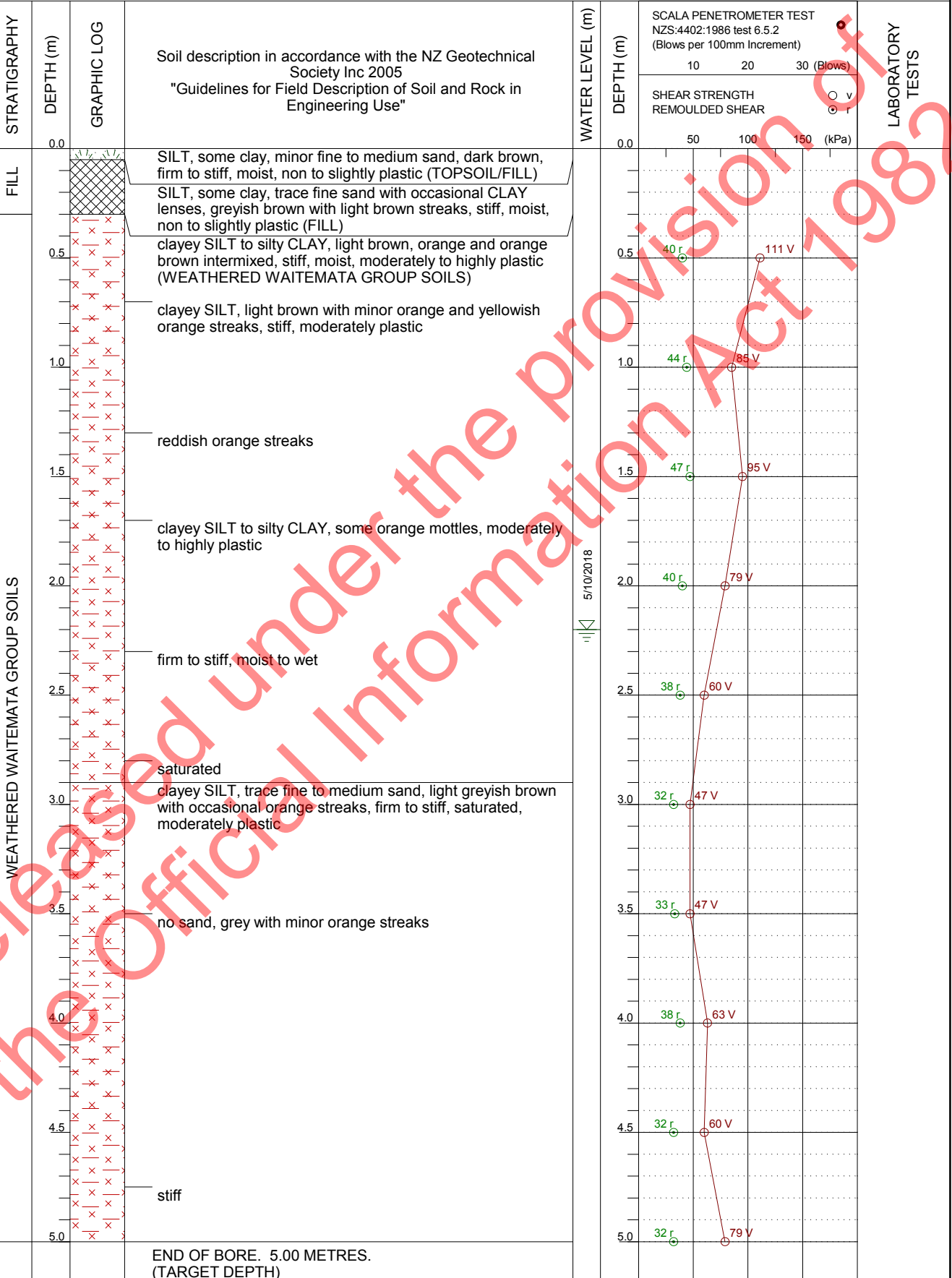
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 3.9m 5/10/2018

Logged By: NC
Shear Vane No - Calibration Date: GEO119 - 23/11/2017
Surface Conditions: Slightly Sloping, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018, GPJ S+R 2013, GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: CD
Date Started: 3/10/18
Date Finished: 3/10/18

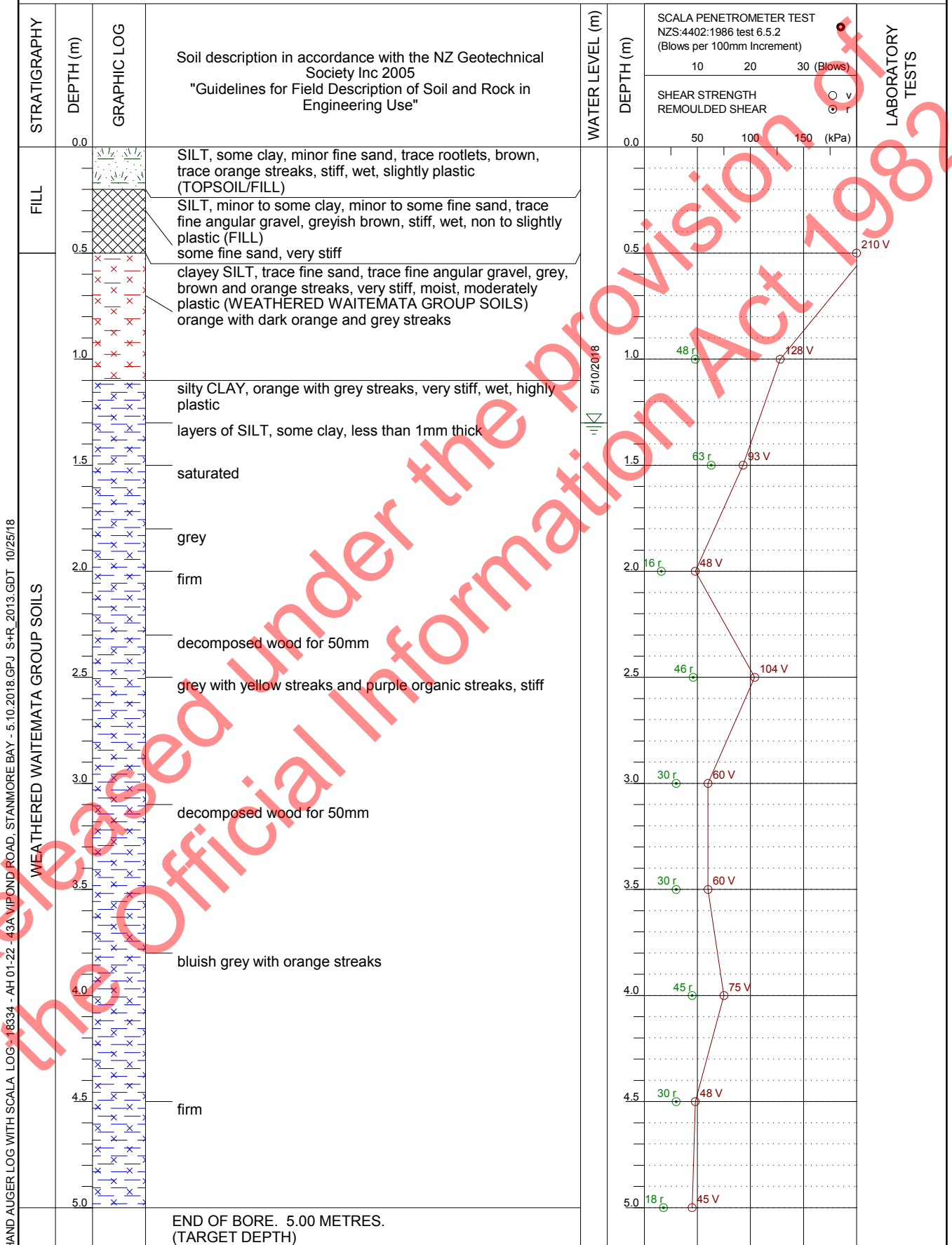
Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 2.2m 5/10/2018

Logged By: CD
Shear Vane No - Calibration Date: GEO361 - 27/02/2018
Surface Conditions: Sloping, Grass


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18

Drill Type: 50mm Hand Auger
Drilled By: NG
Date Started: 3/10/18
Date Finished: 3/10/18

Project No: 18334
Coordinates:
Ground Elevation:
Water Level: 1.3m 5/10/2018

Logged By: NG
Shear Vane No - Calibration Date: GEO122 - 1/12/2017
Surface Conditions: Near Level, Soil


HAND AUGER LOG WITH SCALA LOG - 18334 - AH 01-22 - 43A VIPOND ROAD, STANMORE BAY - 5/10/2018.GPJ S+R 2013.GDT 10/25/18


SCALA PENETROMETER SHEET - TABLE OF BLOWS PER INCREMENT

JOB NAME: 43A Vipond Rd & 20 Melia PI JOB NO: 18334 TESTED BY: CD/etc.. DATE: 3-5/10/18
Stanmore Bay

Depth of Penetration [mm]	AH101	AH102	AH103	AH104	AH105	AH106	cont..	AH108	AH109	AH110	AH111	AH112
DEPTH START [m] ➡	5.00	5.00	5.00	5.00	5.00	5.00	7.00	5.00	5.00	5.00	5.00	5.00
50 mm	0.5	3	3	2	1	1	10	1	2	1	1	1
100	0.5	4	4	3	2	1	10	1	2	2	2	1
150	1	5	4	3	3	1	10	1	2	2	2	1
200	1	5	6	4	4	1	10	2	2	2	2	1
250	1	5	8	4	4	1	10	2	3	3	2	1
300	1	8	10	5	6	1		3	2	3	2	1
350	2	8	8	7	7	1		3	3	3	3	1
400	2	10	6	7	8	2		3	3	9	3	1
450	3	8	1	7	7	2		3	4	10	3	2
500	2	8	1	8	8	3		5	5	12	6	3
550	3	10	12	8	10	3		5	5	20+	6	3
600	3	14	14	10	10	4		5	5		6	4
650	3	10	14	10	10	6		5	5		10	5
700	3	11		12	20+	6		6	5		8	6
750	3	11		12		6		8	6		9	9
800	3			11		6		8	5		9	6
850	5					6		6	5		10	7
900	6					7		8	4		10	6
950	5					7		10	10		10	8
1000	10					9		10	10		11	6
1050	7					10		12	10		10	7
1100	7					1		10	10			10
1150	1					10		10	11			13
1200	10					6						12
1250	10					6						12
1300	10					6						12
1350	11					7						
1400						7						
1450						7						
1500						7						
1550						7						
1600						10						
1650						10						
1700						7						
1750						6						
1800						7						
1850						10						
1900						10						
1950						6						
2000						6						
DEPTH END [m] ➡	6.35	5.75	5.65	5.80	5.70	7.00	7.25	6.15	6.15	5.55	6.05	6.30

Testing Method: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer


SCALA PENETROMETER SHEET - TABLE OF BLOWS PER INCREMENT

JOB NAME: 43A Vipond Rd & 20 Melia Pl JOB NO: 18334 TESTED BY: CD/etc.. DATE: 3-5/10/18
Stanmore Bay

Depth of Penetration [mm]	AH113	AH114	AH116	AH117	AH118	AH119	cont..	AH120	AH121	AH122		
DEPTH START [m] ➡	4.40	5.00	5.00	5.00	5.00	5.00	7.00	5.00	5.00	5.00		
50 mm	2	SUNK	SUNK	0.5	3	0.5	9	1	1	1		
100	4	↓	↓	0.5	5	0.5	10	2	1	1		
150	5	↓	1	1	5	1	10	2	2	1		
200	3	1	2	1	6	1	10	2	2	1		
250	4	1	2	1	8	1	10	2	2	1		
300	3	1	2	2	8	1	11	4	2	2		
350	5	1	2	2	10	2		3	3	2		
400	10	1	2	2	11	2		4	3	2		
450	9	2	3	3	14	2		4	2	2		
500	10	2	3	3	14	2		4	3	2		
550	10	3	3	3	13	2		4	4	2		
600	10	3	4	3		2		5	3	3		
650	6	2	3	3		3		6	3	3		
700	6	3	3	4		3		6	3	3		
750	10	5	3	4		3		6	4	3		
800	10	6	4	4		3		9	4	4		
850	10	5	4	4		4		11	4	4		
900	10	5	7	5		4		11	4	4		
950	10	5	10	5		4		10	4	4		
1000		5	10	7		4		12	5	9		
1050		12	6	7		5		12	5	7		
1100		7	7	7		6			5	11		
1150		7	7	7		5			8	8		
1200		7	7	9		6			5	12		
1250		7	7	9		6			7	10		
1300		7	7	7		6			6	10		
1350		7	7	7		7			7	10		
1400		10	8	10		8			6	10		
1450		10	10	10		7			7			
1500		10	10	10		7			8			
1550		11	10	10		7			6			
1600		10	10	10		7			7			
1650			10			8			10			
1700						8			10			
1750						9			10			
1800						9			10			
1850						10			10			
1900						10						
1950						9						
2000						7						
DEPTH END [m] ➡	5.35	6.60	6.65	6.60	5.55	7.00	7.30	6.05	6.85	6.40		

Testing Method: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

SS105 (AH105)

Shrink-Swell Test Results

Job Name:	43A Vipond Road, Stanmore Bay	Job No:	18334
Date:	04.10.18	Tested By:	SV
Sample Location:	SS105 (AH105)	Date Sampled:	03.10.18
Sampling method:	Push Tube	Sampled By:	CD
Sampling depth (m):	0.4-0.7	Inert inclusions (%):	0
Sample condition:	Good	Extent of cracking (%):	<5
		Extent of crumbling (%):	0

Sample description: Silty CLAY, trace fine sand, trace rootlet inclusions, light grey and orange, very stiff, moist, highly plastic (Natural)

Wet Density	γ (t/m ³) =	1.73
Dry Density	γ_d (t/m ³) =	1.14

Shrinkage Test

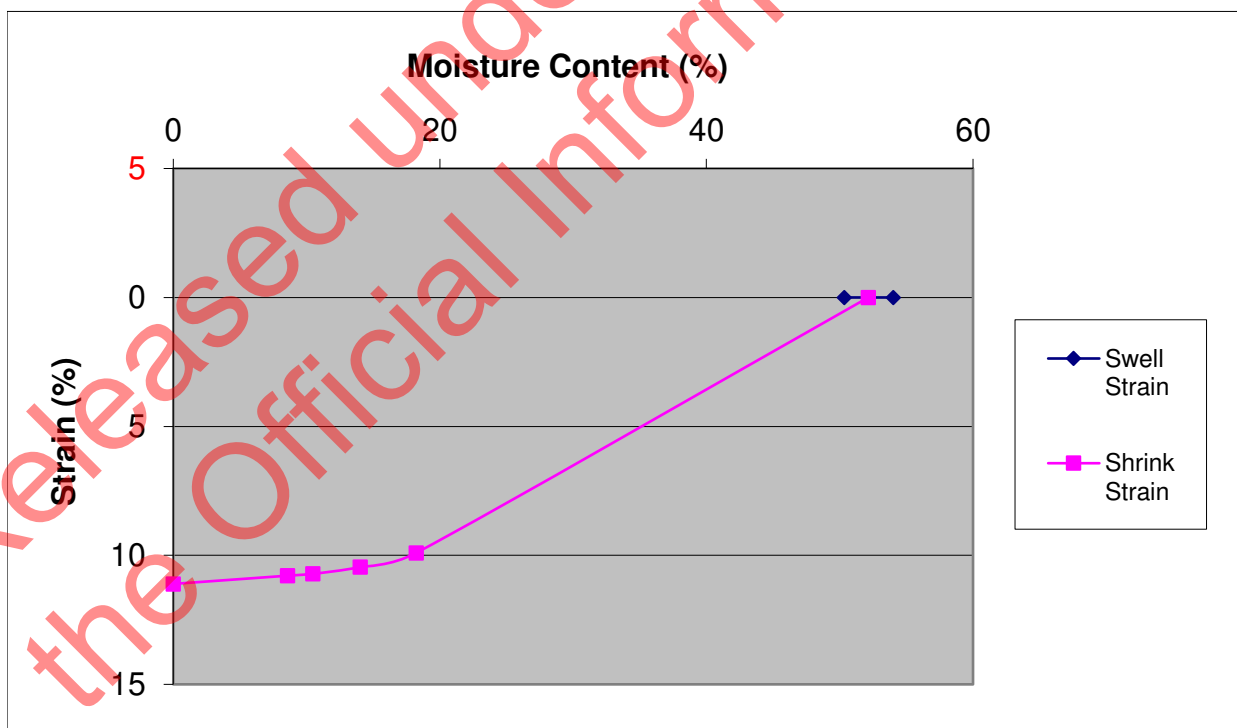
Initial moisture content (%) =	52.1
ϵ_{sh} = Magnitude of total shrinkage strain (%) =	11.1

Swell Test

ϵ_{sw} = Magnitude of the swelling strain (%) =	0.1
--	-----

(Note: The ϵ_{sw} value is negative if the sample has undergone consolidation)

Initial moisture content (%) =	54.0
Final moisture content (%) =	50.3



Shrink-Swell index

Iss = **6.2** Strain per ΔpF (%)

Testing Method: AS1289.7.1.1 - 1998 Soil reactivity tests

SS111 (AH111)

Shrink-Swell Test Results

Job Name: 43A Vipond Road, Stanmore Bay
 Date: 04.10.18
 Sample Location: SS111 (AH111)
 Sampling method: Push Tube
 Sampling depth (m): 0.4-0.7
 Sample condition: Good

Job No: 18334
 Tested By: SV
 Date Sampled: 03.10.18
 Sampled By: CD
 Inert inclusions (%): 0
 Extent of cracking (%): <5
 Extent of crumbling (%): 0

Sample description: Clayey SILT, trace fine sand, light grey and light orange, very stiff, moist, moderately plastic (Natural)

Wet Density

γ (t/m³) = 1.74

Dry Density

γ_d (t/m³) = 1.17

Shrinkage Test

Initial moisture content (%) = 48.9

ϵ_{sh} = Magnitude of total shrinkage strain (%) = 9.2

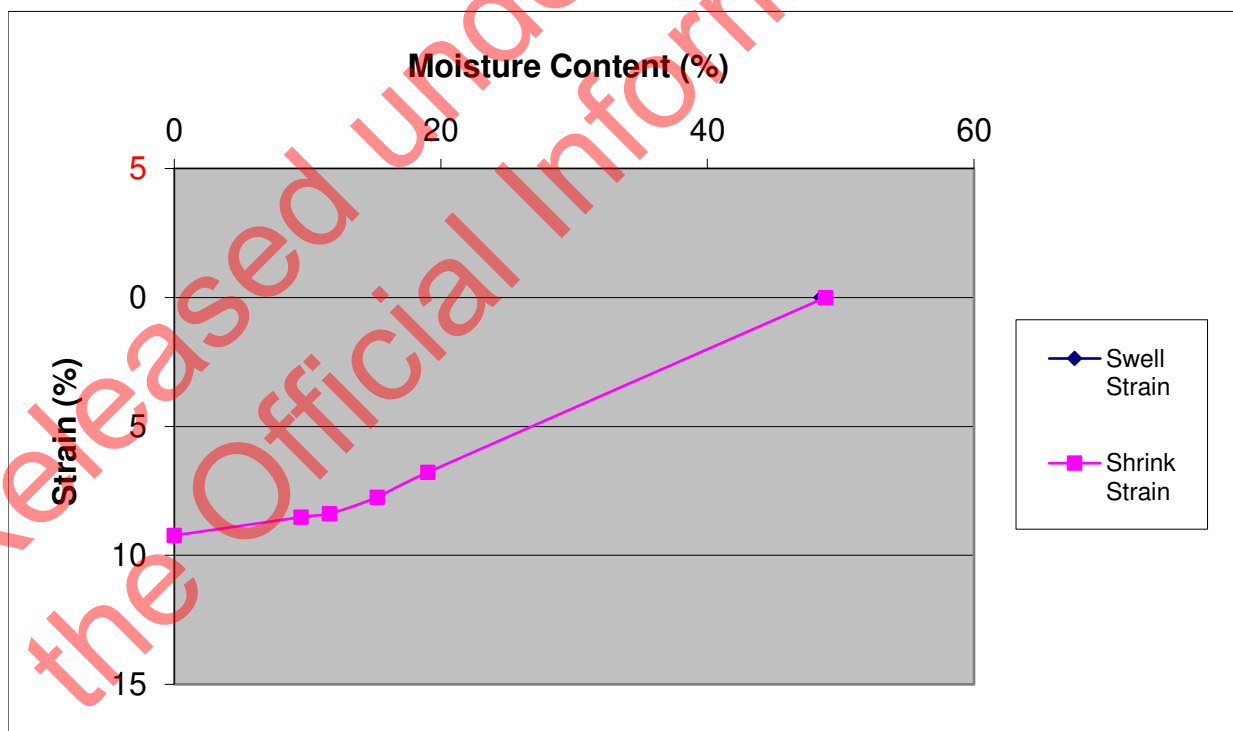
Swell Test

ϵ_{sw} = Magnitude of the swelling strain (%) = -0.1

(Note: The ϵ_{sw} value is negative if the sample has undergone consolidation)

Initial moisture content (%) = 48.5

Final moisture content (%) = 48.9



Shrink-Swell index

Iss = 5.1

Strain per ΔpF (%)

Testing Method: AS1289.7.1.1 - 1998 Soil reactivity tests

SS119 (AH119)

Shrink-Swell Test Results

Job Name: 43A Vipond Road, Stanmore Bay
 Date: 04.10.18
 Sample Location: SS119 (AH119)
 Sampling method: Push Tube
 Sampling depth (m): 0.5-0.9
 Sample condition: Good

Job No: 18334
 Tested By: SV
 Date Sampled: 03.10.18
 Sampled By: CD
 Inert inclusions (%): 0
 Extent of cracking (%): <5
 Extent of crumbling (%): 0

Sample description: Clayey SILT, trace fine to medium sand, orange with grey streaks, very stiff, moist, moderately plastic (Natural)

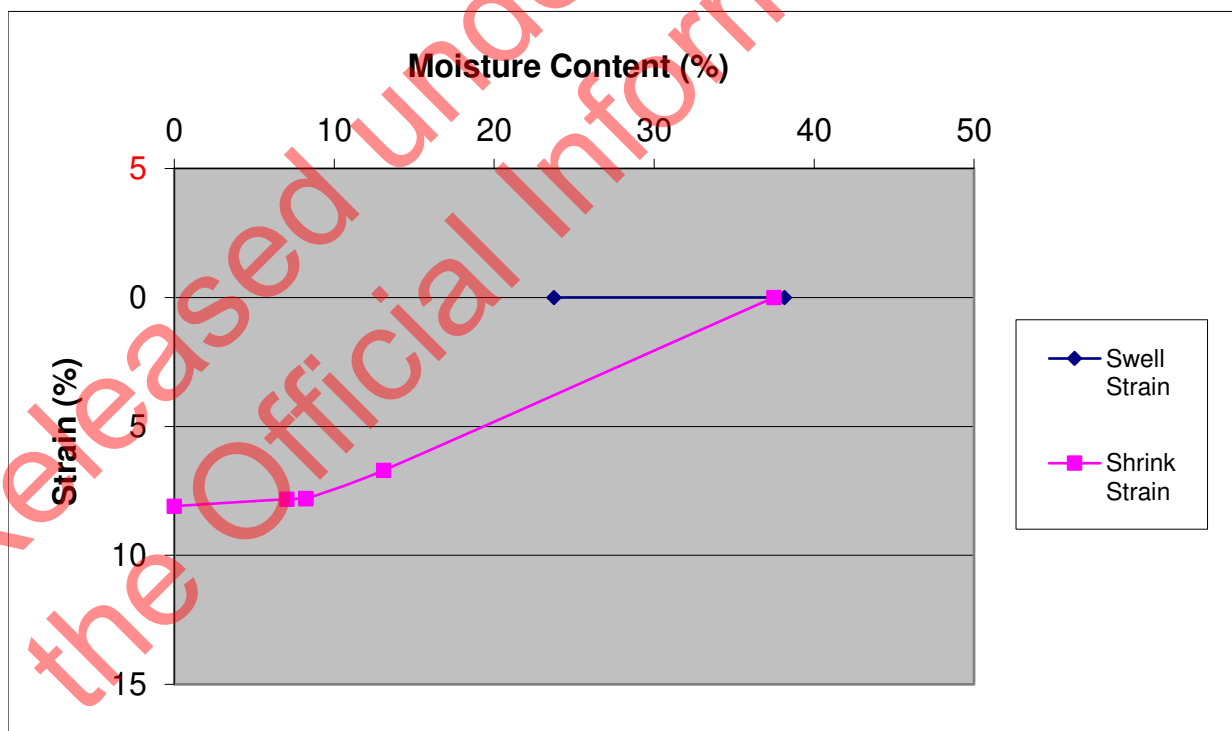
Wet Density	γ (t/m ³) = 1.84
Dry Density	γ_d (t/m ³) = 1.34

Shrinkage Test

Initial moisture content (%) =	37.5
ϵ_{sh} = Magnitude of total shrinkage strain (%) =	8.1

Swell Test

ϵ_{sw} = Magnitude of the swelling strain (%) =	-1.0
(Note: The ϵ_{sw} value is negative if the sample has undergone consolidation)	
Initial moisture content (%) =	38.2
Final moisture content (%) =	23.7



Shrink-Swell index

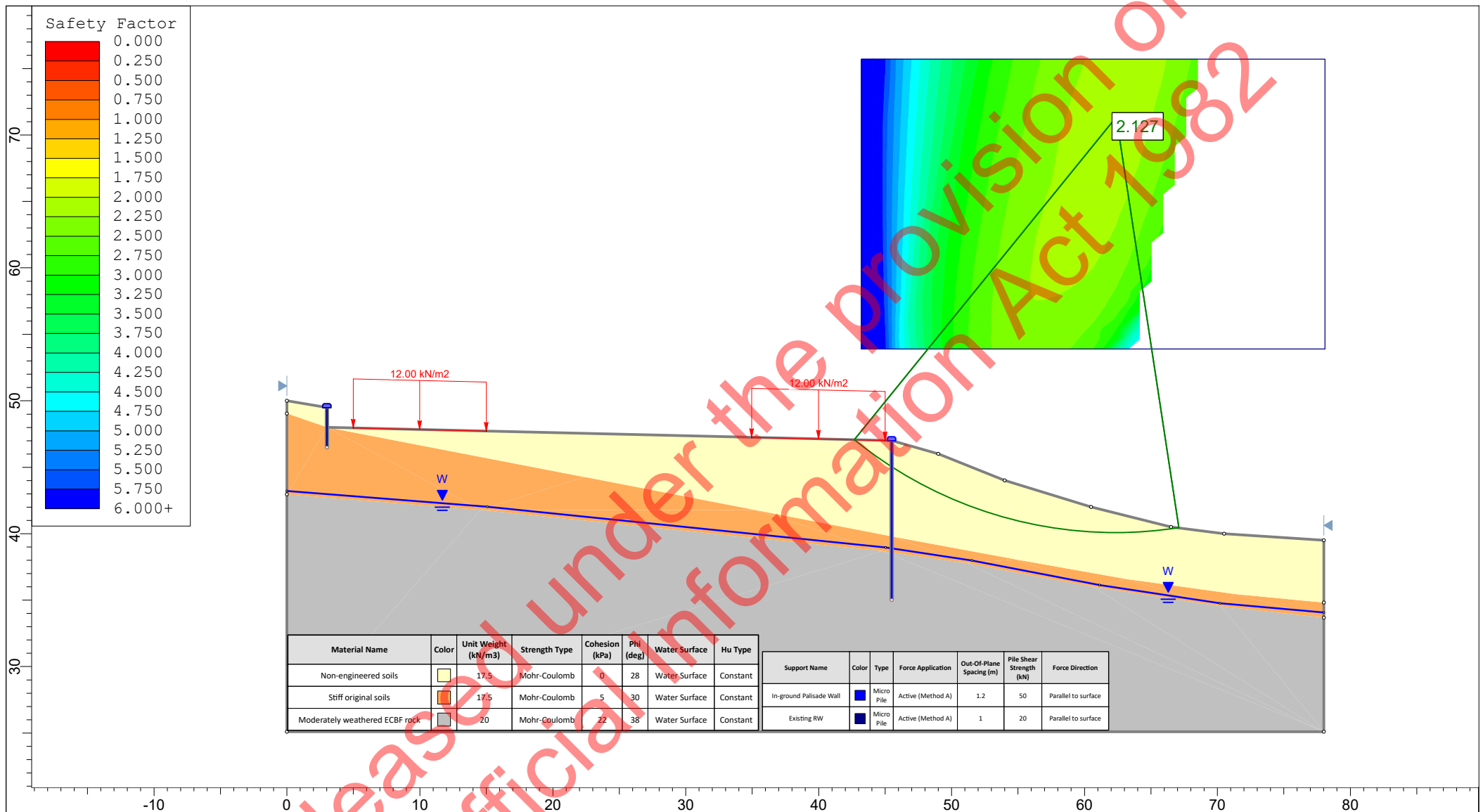
Iss = 4.5 Strain per ΔpF (%)

Testing Method: AS1289.7.1.1 - 1998 Soil reactivity tests

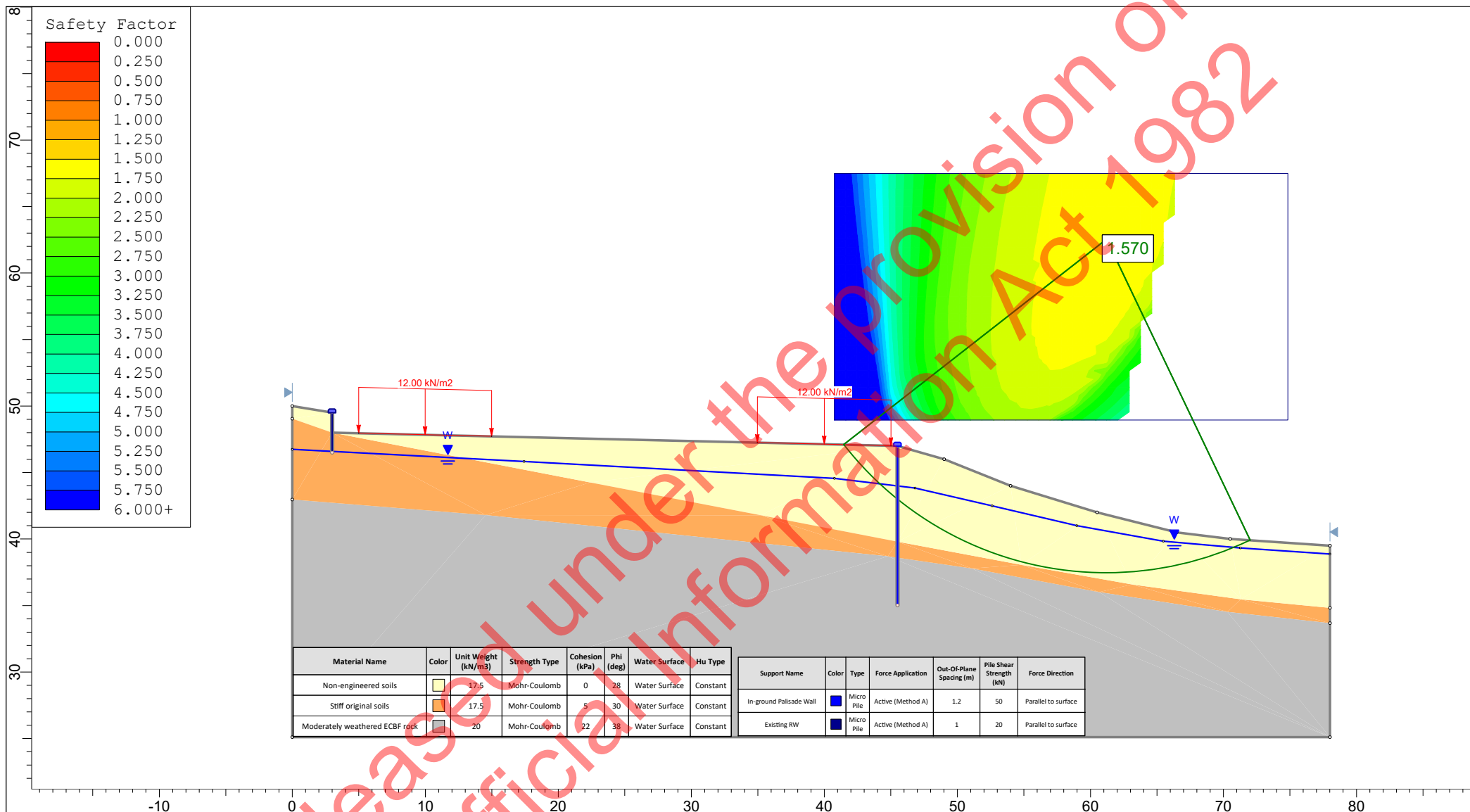
Appendix C

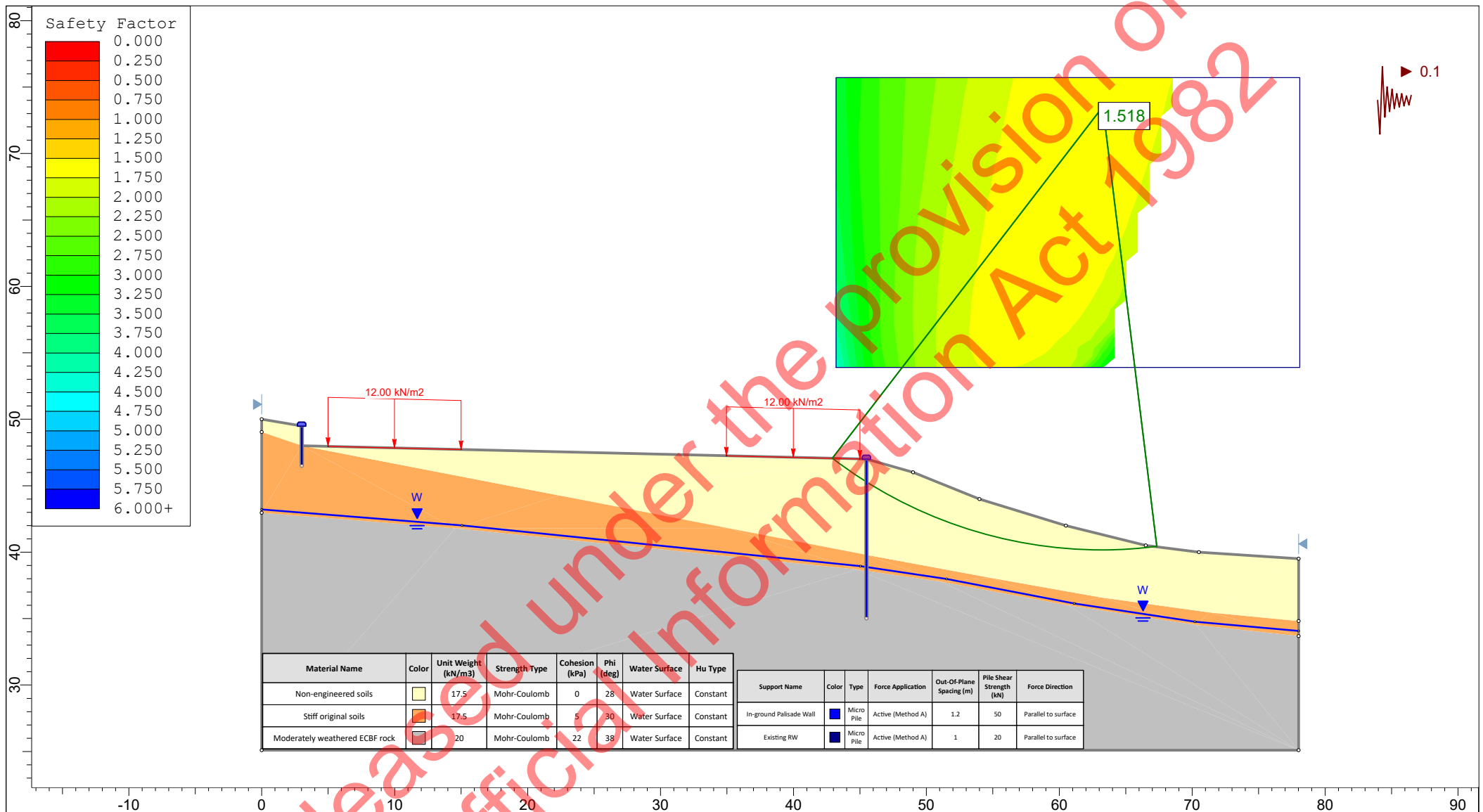
Slope Stability Analyses

Released under the provision of
the Official Information Act 1982



Project		43A Vipond Rd, & 20 Melia Pl, Stanmore Bay	
Analysis Description		Normal GW	
Drawn By	R.B.	Scale	1:400
Date	16/02/2021, 8:28:51 AM	Company	GeoStudio Ltd
		File Name	Normal GW.slim





Appendix D

SK-2282-01, SK-2282-02 & SK-2282-03

Released under the provision of
the Official Information Act 1982

SK-2282-01
 GeoStudio Ltd

DRAFT

REV	DATE	INITIAL	AMENDMENT

PROJECT STATUS

CLIENT
 MELIA DEVELOPMENT LIMITED
 PROJECT
 20 MELIA PLACE
 20 MELIA PLACE, WHANGAPARAOA

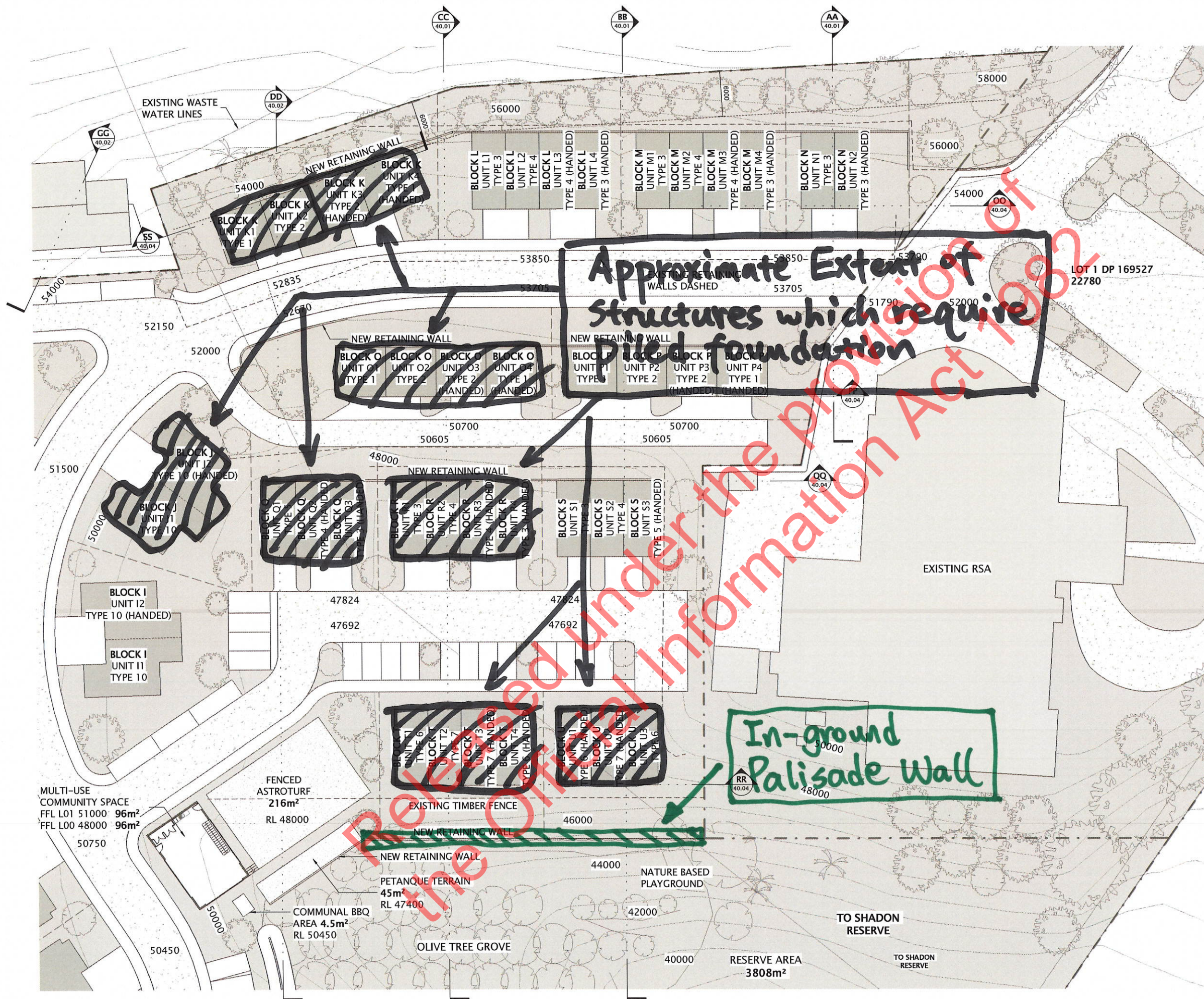
SHEET TITLE
 PROPOSED DEVELOPMENT
 ZONE 2

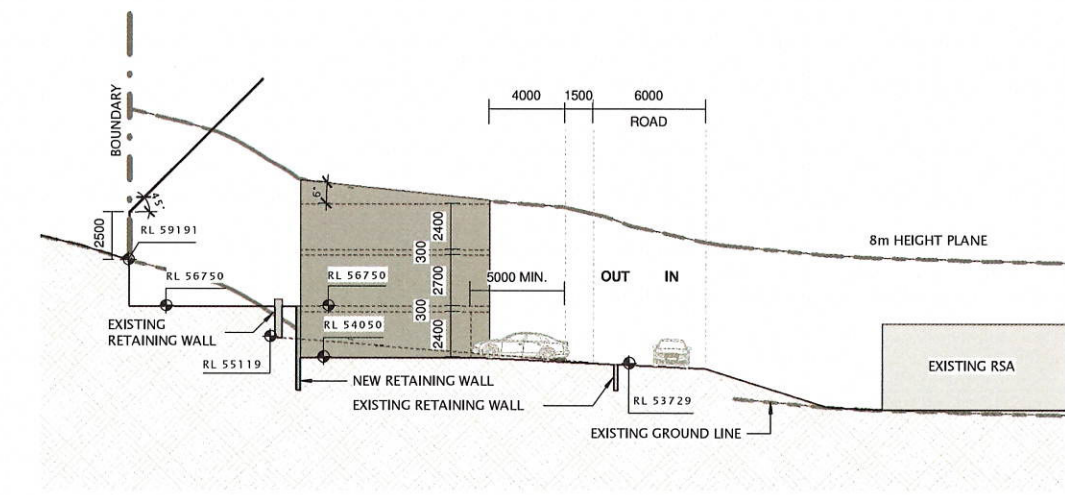
TRUE NORTH	DESIGN	DRAWN
N	CM	LM
	SCALE @ A1	SCALE @ A1
	1 : 250	1 : 250

FIRST ISSUE DATE
 01/27/21
 PROJECT No.
 20053
 SHEET No.
 10.03

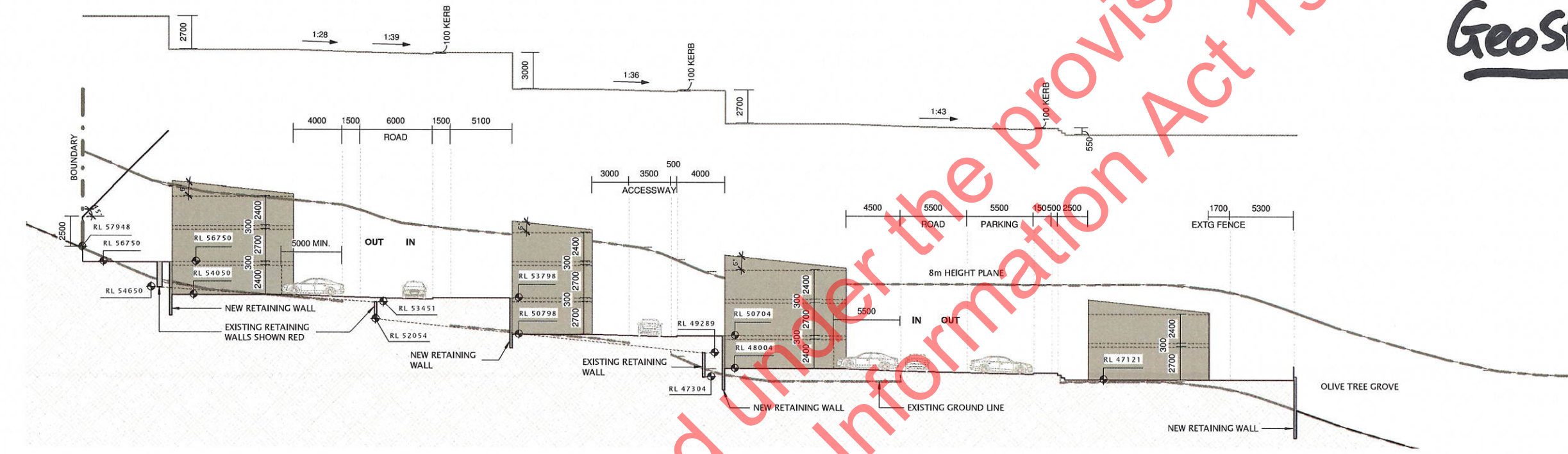


paterson +
 cullen + archaus

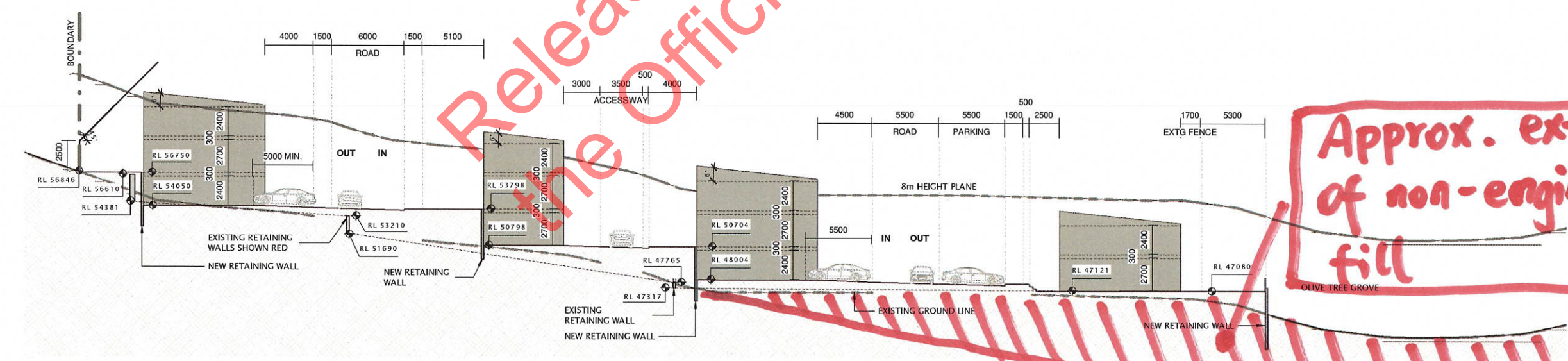




AA SITE SECTION A-A
10.01 SCALE: 1 : 200



BB SITE SECTION B-B
10.01 SCALE: 1 : 200



CC SITE SECTION C-C
10.01 SCALE: 1 : 200

SK-2282-02
GeoStudio Ltd

DRAFT

REV	DATE	INITIAL	AMENDMENT
1	01/25/21	CM	DESIGN
2	01/25/21	LM	DRAWN

PROJECT STATUS

CLIENT
MELIA DEVELOPMENT LIMITED

PROJECT
20 MELIA PLACE
20 MELIA PLACE, WHANGAPARAOA

DESIGN	DRAWN
CM	LM

SCALE @ A1
(HALF SCALE IF PRINTED @ A3)
1 : 200

FIRST ISSUE DATE
01/25/21

PROJECT No.
20053

SHEET No.
40.01

REVISION

Approx. extent
of non-engineered
fill



paterson +
cullen + archaeus



Approx. extent of non-engineered fill

DRAFT

PROJECT STATUS

SHEET TITLE
SITE SECTIONS

40.02

paterson +
cullen + archaus