

Geotechnical Investigation Report

43A Vipond Road & 20 Melia Place, Stanmore Bay For Melia Development Limited

Reference: 2282

Date: 21 May 2021

Revision: E



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Appendices

Appendix A: Draft Site Plans & Elevation

Appendix B: Investigation Logs

Appendix C: Slope Stability Analysis

Appendix D: SK-2282-01, SK-2282-02 & SK-2282-03



1 Introduction

GeoStudio Ltd has been engaged to undertake a subsoil investigation and provide geotechnical recommendations for the proposed new 2 to 3 story multi-unit development at 43A Vipond Road & 20 Melia Place, Stanmore Bay.

This geotechnical investigation has been conducted to provide recommendations with respect to slope stability and foundation design for the proposed new units. It aims to determine whether the land on which the new units have been proposed is likely to be subject to erosion, subsidence, or slippage; or whether the proposed development work itself is likely to accelerate, worsen, or result in instability of the land or any other property; and to recommend adequate provisions to protect the land or the proposed development or other property from instability in accordance with the provisions of the Building Act 2004.

2 Site Description

The site location is indicated on Figure 1. Its legal description is Lot 1 & Lot 2, DP 169527.

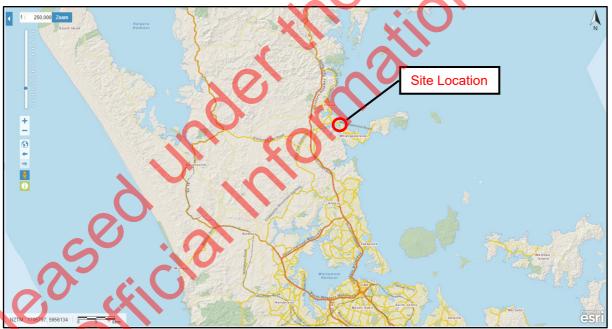


Figure 1: Site Location Plan (Courtesy of Auckland Council Geomaps)

The existing site layout is shown in Figure 2 and Photos 1 to 4.

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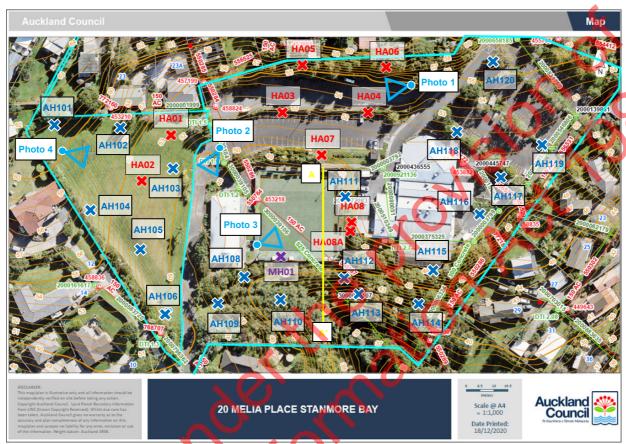


Figure 2: Site Layout and Investigation Plan (courtesy of Auckland Council Geomaps)



Photo 1: This photo was taken at 34A Vipond Rd near HA04 facing west. Its shows the existing car park and the existing bowling club building. The grass area in the bacground is the proposed building area at 20 Melia PI.





Photo 2: This photo was taken from the north eastern corner of 20 Melia PI facing south west. It shows the drive way/ parking area at 34A Vipond Rd on the left and the grass area of 20 Melia PI on the right.



Photo 3: This photo was taken from the south west corner of the exsiting bowling green facing north east. It shows the existing bowling green and existing RSA building in the bacground.

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Photo 4: This photo was taken from the western corner of 20 Melia PI facing east. It shows the grass area of 20 Melia PI, the existing bowling green and building on the rigth and existing RSA building and car park in the background.

Both properties can be accessed via a long driveway from Vipond Rd and Melia Pl. At the time of our investigation the properties consisted of two existing large buildings and one existing bowling green. The area surrounding the existing buildings and bowling green consisted mostly of grass and small to large trees. The ground in the area of the proposed new units at 20 Melia Pl slopes down towards east between 8-12°. The ground in the area of the proposed new units at 43A Vipond is terraced down towards the south with small slopes of 5-7° in between the terraces. Immediately south of the proposed units at the southern area of the bowling green, the ground becomes very steep with slopes of up to 22° to the south and east.

3 Proposed Development

According to the draft site plans provided by paterson + cullen + archaus, it is proposed that the existing bowling green will be removed. It is then proposed there will be earthwork excavation of up to **2.4m** and fill of up to **3.0m**, from the existing ground level for the construction of the new units and associated civil works.

The draft site plans are attached in Appendix A.

4 Geological Map

According to Geological Map of the Auckland Area, 1:250 000 geological Map 3 by Institute of Geological and Nuclear Sciences, the site is likely to be underlain by East Coast Bays Formation (ECBF) of Waitemata Group. These soils have alternating sandstone and mudstone with variable volcanic content and interbedded volcaniclastic grit beds.

The clay rich and cohesive silt soils may also be prone to shrinking and swelling following changes in natural moisture content.



5 Previous Geotechnical Report

We have also viewed the Geotechnical Completion Report prepared by Soil & Rock Consultants, Rev: 18334, dated 24 October 2018. The specific stability and foundation design recommendations in this report are summarised below.

- Based on the laboratory tests carried out, the soils present are considered to lie in two different site classes Site Class H1 (Highly Reactive) and Site Class H2 (Highly Reactive) in terms of BRANZ Addendum Study Report 102A (based on AS 2870:2011).
- A Dependable Bearing Capacity of 150kPa (Ultimate Bearing Capacity = 300kPa, Øbc = 0.5) is available for shallow foundations embedded 450mm into stiff natural ground or engineered fill or 600mm below final ground surface (whichever gives the deeper embedment).

6 Site investigation

We carried out a site walkover on 18th December 2020. Eight 50mm diameter hand augered exploratory holes were carried out in the area of the proposed fifty-eight new units. The locations of the exploratory holes are shown in Figure 2. In-situ Pilcon Shear Vane tests were carried out in the augered holes generally at 0.5m depth intervals. An additional Scala Penetrometer test was carried out at the base of HA08, and an additional 12.5m deep machine borehole was carried out at the edge of the steep slope along the southern edge of the existing bowling green on 18th December 2020. SPTs were carried out at 1.5m intervals. Hand auger investigation results from the previous geotechnical report are also attached herein and shown in Figure 2.

The graphical hand auger and machine borehole logs are presented in Appendix B.

7 Subsoil Conditions

The findings from our hand auger investigation generally agree with the Geological Map 3 of the Auckland area. During the day of our site investigation, we encountered non engineered fill soils which which extended from the existing ground level down to 0.4-0.8m bgl in HA01, HA03, HA04 & HA05, and down to 4.7m bgl in HA08. Underlain these non-engineered soils and from the existing ground level in HA02 & HA06, we encountered original East Coast Bays Formation (ECBF) soils which extended down to our hand auger termination depths of 3.0m bgl in HA01 – HA07 and 5.0m bgl in HA08. An additional Scala Penetrometer test was carried out at the base of HA08 which was terminated at 7.0m bgl. These original ECBF soils encountered during drilling varied from clay to silt. The shear vane tests suggest that these clayey to silty soils are generally stiff to very stiff in consistency. These stiff to very stiff original ECBF soils are considered suitable bearing strata for the proposed excavation and construction. We deem the soils encountered on-site during the day of drilling to be highly expansive (H1).

The machine bore hole carried out onsite encountered non-engineered fill soils which extended from the existing ground level down to 7.2m bgl. Underlain these non-engineered fill soils the bore hole encountered original alluvial soils which extended from 7.2m bgl down to 7.7m bgl. Underlain these original alluvial soils the bore hole encountered original ECBF, weathered sand and silt stone which extended from 7.7m bgl down to the bore hole termination depth of 12.5m bgl after encountering three consecutive SPT 50+ for 450mm.



A **(0.2-0.3m)** deep layer of topsoil and a **(0.4-4.7m)** deep layer of non-engineered fill was encountered in all hand auger holes during the day of exploratory drilling. This topsoil and non-engineered fill is not considered consistent bearing strata for the proposed development.

8 Groundwater

No ground water table was encountered during the day of exploratory drilling.

9 Ground Stability

On the day of our site visit, no obvious signs of global instability were observed.

We have carried out a slope stability analysis of the site with finished cut and fill levels for the proposed building platform (cross-section A-A' shown on our site plan in Appendix B). The calculated factors of safety for the proposed building platform for normal groundwater, elevated groundwater and seismic conditions are 2.13, 1.57 and 1.52 respectively. They are considered acceptable for the proposed development. The analysis results are presented in Appendix D.

10 Earthworks

The proposed depth of excavation (up to **2.4m**) and fill (up to **3.0m**) within the boundaries of the subject site is considered reasonable.

The proposed excavations are setback at least 2x excavation depth from any neighbouring structures. We consider that the proposed excavation posts low risk to neighbouring properties, provided that good retaining wall construction practice, i.e. safe temporary batter and good construction time management are implemented.

The proposed fills are mainly in areas underlain by original ECBF soils. These stiff to very stiff, silty to clayey soils are not overly sensitive to settlement. Some fill locations, namely in proximity of the existing 525mm diameter concrete pipe, would likely cover by existing fill. Although the fill encountered was tested to be stiff to very stiff and appeared to have been reasonably compacted a number of years ago. Some minor long term settlement is still expected for the proposed maximum 3m deep additional fill. This settlement is expected to be minor and relatively uniform. It would unlikely adversely affect the performance of driveways and underground services. However, building foundations shall be supported on piles founded below the fill layer as per Section 11.

It is our opinion that provided our recommendations in Section 11 are fully implemented the earthworks would be unlikely to cause any significant ground settlement.

11 Recommendations

Our recommendations are summarised as follows:

11.1 In-ground Palisade Wall

Our sketch (**SK-2282-01**, shown in Appendix D) shows the location of our recommend in-ground palisade wall along the crest of the existing non-engineered fill slope. This palisade wall shall be designed as follows:



- The palisade wall shall be constructed immediately to the south of the proposed units and extend a minimum of 2.0m beyond the width of the proposed building footprint. The final location of the palisade wall shall be confirmed by GeoStudio Ltd.
- The wall shall be designed by a chartered structural engineer and shall be a minimum of 0.6m diameter steel-reinforced concrete piles at a maximum of 3xD centre spacing. A concrete capping beam shall be included close to ground level.
- Minimum embedment depth = 10.0m below existing ground level or socketed at least 2.5m in moderately weathered ECBF rock whichever is greater.
- Lateral soil load depth = 5.0m for structural design of piles. Lateral soil loads may be calculated using γ=18kN/m³ and K₀=0.5 over a width of pile centre spacing.

11.2 Earthworks

- Detailed earthwork and retaining wall plans are not yet available at the time of preparing this
 report. Geostudio Ltd should be asked to review detailed earthwork and retaining wall plans to
 confirm our recommendations herein.
- Based on our subsoil investigation, the upper 1.5m to 2.3m depths soils were mainly moist.
 These moist soils would likely be able to be reused directly as engineered fill materials.
 Underlying these moist soils are wet to saturated soils which are unlikely to be suitable for direct use as engineered fill.
- For the benefits of utilising in situ cohesive soils for fill materials, we recommend that any major earth works are carried out during the summer months from November to March. This would reduce the risk of additional stabilisation of the fill material being required. However, the condition of the excavated material shall be inspected by a geotechnical engineer for its suitability as engineered fill. Additional air drying and/or lime stabilisation may still be required.
- We recommend that all cut slopes into natural ground and fill slopes using engineered fill should not be steeper than 2.5H:1V (21.8°). All retaining walls shall be designed by a chartered professional engineer.
- All topsoils and unsuitable soils shall be removed and benched into the slope prior to filling.
 The cleared ground shall be inspected by a chartered geotechnical engineer.
- Due to the complexity and volume of proposed earthwork, a Geotechnical Completion Report is recommended to ensure all earthworks are appropriately supervised during construction and are certified by a chartered professional geotechnical engineer.
- Any cohesive fill shall be constructed using a suitable sized pad-foot roller in layers no more than 200mm thick. We provide the following specifications for cohesive fill compaction.
 - Undrained shear strength minimum single value = 110kPa.
 - Undrained shear strength average value = 140kPa.
 - Maximum single value air void =12%.
 - Average value air void < 10%.



- All engineered fill shall be inspected by a geotechnical engineer every 0.5m layer, or otherwise instructed by the engineer on site.
- Granular fill (hardfill) may be used for earthworks outside summer months. It shall be constructed using suitably sized vibrating drum roller or vibrating plat compactor in suitable thicknesses depending on compactor size. We provide the following specifications for granular fill compaction:
 - Granular fill shall be certified aggregates (GAP40 or GAP65). Other aggregate type i.e. crushed concrete/Soft Pit Run (SPR) may be accepted for fill in non-critical areas subject to specific engineer approval.
 - Clegg Hammer Impact Value (CIV) >25.
 - Achieve 95% maximum dry density and ±3% of optimum moisture content.
 - All engineered fill shall be inspected by a geotechnical engineer every 0.5m layer, or otherwise instructed by the engineer on site.

11.3 Foundations

11.3.1 Suspended floors on Pile Foundations

Due to the presence of non-engineered fill encountered, all load bearing footings foundations as shown as shaded areas in **SK-2282-01** shall be supported on piled foundations extending below the existing non-engineered fill as follows.

- Geotechnical ultimate bearing capacity (unfactored) = 720kPa for piles through fill with a minimum of 0.6m into stiff original soils. The expected pile depths range from 1.2m to 8.0m below the existing ground level.
- Geotechnical ultimate bearing capacity (unfactored) = 4000kPa for piles through fill with a minimum of 0.6m into moderately weathered ECBF rock (SPT=50+). The expected pile depths range from 8.0m to 11.0m below the existing ground level.
- Geotechnical ultimate skin friction (unfactored) of 30kPa, the friction contribution of the upper 0.6m below ground level, or non-engineered fill depth should be ignored, whichever is greater.
- Due to risk of long term settlement of the underlying non-engineered fill, concrete slabs or subfloor shall be designed as **fully suspended**, i.e. not relying on ground for support.
- Based on the investigation results and our experience with similar sites, the site may be classified as subsoil Class C – shallow soil in accordance with ASNZS1170.5.

11.3.2 Concrete Slab Foundations

For areas outside the shaded areas in **SK-2282-01**, shallow foundations are considered suitable. We provide the following foundation design recommendations:

 The natural soils, i.e. excluding topsoil and any non-engineered fill, are generally considered suitable for NZS3604:2011 type foundations, except that due to the potential for shrinkage and swelling, the subsoil lies outside of the definition of 'good ground' as defined by NZS3604:2011.
 The perimeter footings shall have a minimum embedment depth of 750mm below the cleared



ground level, or alternatively the foundations shall be specifically designed by a chartered professional engineer for expansive soil **Class H1** in terms of AS2780:2011.

- For design of shallow foundations on competent natural ground, a geotechnical ultimate bearing capacity (unfactored) of 300kPa may be assumed.
- Based on the investigation results and our experience with similar sites, the site may be classified as subsoil Class C – shallow soil in accordance with ASNZS1170.5.
- All unsuitable soils (topsoil, non-engineered fill, soft original soil) shall be removed and replaced
 with compacted engineered fill (GAP40) under all concrete slabs. The expected undercut depth
 is approximately 0.1m to 0.5m below the existing ground level.
- For the use of other pile foundations (if required), our recommendations are as follows:
 - Minimum pile depth = 0.6m into stiff original soils.
 - Geotechnical ultimate bearing capacity (unfactored) = 300kPa or 540kPa for piles
 >2.5m deep
 - Geotechnical ultimate skin friction (unfactored) of 30kPa. The friction contribution of the upper 0.6m below ground level.

11.4 Retaining Walls

- All new retaining walls shall be specifically designed by a chartered professional engineer for the actual retaining height and any surcharge (i.e. additional load from slopes, upper retaining walls or site boundary).
- Furthermore, we recommend that any vertical cut faces which exceed heights of 1.5m should be benched at 1V:1H or to be supported by additional in-ground piles prior to bulk excavation. Along with good practice during construction (covering cut faces, constructing of retaining walls within seven days of excavation), this method is to further ensure safety during works close to, or below cut faces.
- We provide the following general soil parameters for retaining wall design:

Bulk unit weight for retaining material 18.0kN/m³

Soil internal friction (φ')

Undrained shear strength (s_u) 50kPa

Geotechnical ultimate bearing capacity (unfactored)
 300kPa

- All retaining wall shall be constructed with appropriate subsoil drainage system discharged as per council's requirements.
- It is the builder's/owner's responsibility to ensure that temporary stability of any soil cut face is maintained during construction of retaining walls. Where possible, all retaining walls should be constructed in dry weather conditions over a short period of time (typically within 10 days) to reduce the risk of temporary instability.

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11.5 Construction Inspections

- A pre-construction meeting is highly recommended prior to commence of earthworks.
- All soil subgrade (after stripping of topsoil) shall be inspected by a chartered professional engineer (geotechnical) prior to commencement of engineered filling (if required).
- The compaction of all engineered fill shall be inspected by a chartered professional engineer (geotechnical).
- All foundation pile drill holes (if applicable) shall be inspected by a chartered professional engineer (geotechnical) prior to installation of timber poles and pouring of concrete.



12 Limitations

This report is the property of our client and GeoStudio Ltd.

Our professional services are performed using a degree of care and skill normally exercised, under similar circumstances, by reputable consultants practicing in this field at this time. No other warranty, expressed or implied, is made as to the professional advice presented in this report; in regard to its accuracy or completeness.

The recommendations and opinions contained in this report are based on our visual reconnaissance of the site, information from geological maps and field investigation(s) at discrete locations. Inferences are made about the nature and continuity of ground conditions away from the investigation(s) which cannot be guaranteed. The descriptions detailed on the exploratory hole logs are based on the field descriptions of the soils encountered at the time of investigation(s).

This report has been prepared for the particular project described to us and no responsibility is accepted for the use of any part of this report in any other context or for any other purposes. Except as required by law, no third party (excluding the local authority) may use or rely upon this report unless authorised by GeoStudio Ltd in writing. To the extent permitted by law, GeoStudio Ltd expressly disclaims and excludes liability for any loss, damage, cost or expense suffered by any third party relating to or resulting from the use of, or reliance upon any information contained in this report. It is the responsibility of third parties to independently make enquiries or seek advice in relation to their particular requirements.

All appendices should be read in conjunction with the main body of the report and this report should not be considered complete without them.

REPORT PREPARED BY:

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MACO (

Engineering Geologist

REPORT REVIEWED BY:

Geoffrey Kang

BE(civil), ME(civil), CPEng, MIPENZ

STUDIO

Director / Geotechnical Engineer



Appendix A

Draft Site Plans and Sections



PROPOSED OVERALL DEVELOPMENT SCALE: 1:500

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SITE AREA:

SITE CALCULATIONS - 20 MELIA PLACE LOT 2, DP 169527

BUILDING COVERAGE:

IMPERVIOUS AREA:

4640.9m² (25.4 %)

LANDSCAPED AREA:

10489m² (57.5%)

18250.6m²

3120.7m² (17.1%)

PROJECT STATUS

MELIA DEVELOPMENT LIMITED

PROIECT 20 MELIA PLACE

20 MELIA PLACE, WHANGAPARAOA

PROPOSED OVERALL DEVELOPMENT

TRUE NORTH PROJECT NORTH DESIGN DRAWN CM LM

SCALE @ A1 (HALF SCALE IF PRINTED @ A3) 1:500

FIRST ISSUE DATE PROJECT No. 03/25/21

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10.01





1 PROPOSED OVERALL DEVELOPMENT - ZONE 1

10.01 SCALE: 1 : 250

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PROJECT STATUS

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20 MELIA PLACE

20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE

PROPOSED DEVELOPMENT ZONE 1

TRUE NORTH

DESIGN DRAWN CM LM SCALE @ A1 (HALF SCALE IF PRINTED @ A3) 1:250

FIRST ISSUE DATE PROJECT No 01/27/21

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10.02

paterson +

cullen + archaus (09) 309 8931

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58000 EXISTING WASTE 56000 WATER LINES 56000 BLOCK K UNIT K4 BLOCK K UNIT K3 TYPE 1 54000 (HANDED) BLOCK K TYPE 2 BLOCK K UNIT K2 (HANDED) TYPE 2 UNIT K1 TYPE 1 53790 53850--53850-LOT 1 DP 169527 EXISTING RETAINING 52835 22780 WALLS DASHED 53705 53705 52000 52670 52150 NEW RETAINING WALL NEW RETAINING WALL 52000 BLOCK O BLOCK O BLOCK O UNIT O1 UNIT O2 UNIT O3 UNIT O4 TYPE 1 TYPE 2 TYPE 2 (HANDED) (HANDED) BLOCK P BLOCK P BLOCK P UNIT P1 UNIT P2 UNIT P3 UNIT P4 TYPE 1 TYPE 2 TYPE 2 (HANDED) (HANDED) 50700 50700 50605 50605 BLOCK J UNIT J2 51500 NEW RETAINING WALL TYPE 10 (HANDED) BLOCK J UNIT J1 TYPE 10 **EXISTING RSA** BLOCK I 47824 47824 UNIT 12 TYPE 10 (HANDED) 47692 BLOCK I UNIT I1 TYPE 10 50000 FENCED MULTI-USE COMMUNITY SPACE FFL L01 51000 96m² FFL L00 48000 96m² **ASTROTURF** 48000 216m² EXISTING TIMBER FENCE RL 48000 46000 NEW RETAINING WALL 50750 NEW RETAINING WALL 44000 NATURE BASED PETANQUE TERRAIN PLAYGROUND RL 47400 COMMUNAL BBQ 42000 TO SHADON AREA 4.5m² RESERVE RL 50450 **OLIVE TREE GROVE** TO SHADON RESERVE 40000 RESERVE AREA 3808m²

1 PROPOSED OVERALL DEVELOPMENT - ZONE 2 SCALE: 1:250

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PROPOSED DEVELOPMENT ZONE 2

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VIPOND ROAD EXISTING WASTE WATE LINES LOT 1 DP 169527 22780 NEW RETAINING WAL BLOCK O BLOCK O BLOCK O
UNIT O1 UNIT O2 UNIT 03 UNIT 04
TYPE 1 TYPE 2 TYPE 2 TYPE 1

BLOCK P BLOCK P BLOCK P
UNIT P3 UNIT P3
UNIT P4
TYPE 1 TYPE 2 TYPE 2 TYPE 1

TYPE 1 TYPE 2 TYPE 2 TYPE 1 BLOCK E UNIT E2 TYPE 8 (HANDED BLOCK E UNIT E1 TYPE 8 BLOCK J UNIT 12 BLOCK D UNIT D2 TYPE 8 (HANDE EXISTING RSA BLOCK I UNIT I2 PE 10 (HAND BLOCK I UNIT II TYPE 10 BLOCK C BLOCK C UNIT C1 TYPE 8 216m² RL 48000 NATURE BASED PLAYGROUND 40000 RESERVE AREA 3808m² TO SHADON RESERVE LOT 2 DP 169527 5267 MELIA PLACE

PROPOSED OVERALL DEVELOPMENT - TYPOLOGIES SCALE: 1:500

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2 LEVEL UNITS 3 LEVELS UNITS

COMMUNITY FACILITIES

2 LEVEL UNITS **27** (45.8%)

32 (54.2%)

TOTAL NO. OF UNITS 59

3 LEVEL UNITS

PROJECT STATUS

MELIA DEVELOPMENT LIMITED

PROIECT

20 MELIA PLACE 20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE

PROPOSED OVERALL DEVELOPMENT -**TYPOLOGIES**

TRUE NORTH PROJECT NORTH DESIGN DRAWN

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1:500

10.04

paterson + cullen + archaus

4000 1500 6000 ROAD 8m HEIGHT PLANE WERE CONTRACTOR OF THE PROPERTY OF THE PROPERT RL 56750 EXISTING RETAINING WALL RL 55119 EXISTING RSA NEW RETAINING WALL EXISTING RETAINING WALL EXISTING GROUND LINE — AA SITE SECTION A-A SCALE: 1:200 ROAD ACCESSWAY EXTG FENCE 8m HEIGHT PLANE RL 54650 RL 53451 EXISTING RETAINING WALLS SHOWN RED RL 52054 NEW RETAINING WALL RL 47121 EXISTING RETAINING OLIVE TREE GROVE RL 47304 EXISTING GROUND LINE NEW RETAINING WALL BB SITE SECTION B-B SCALE: 1:200 EXTG FENCE ROAD PARKING 8m HEIGHT PLANE RL 54381 EXISTING RETAINING RL 47765 RL 48004 NEW RETAINING WALL RL 47080 RL 47121 NEW RETAINING WALL OLIVE TREE GROVE - EXISTING GROUND LINE EXISTING RETAINING WALL NEW RETAINING WALL NEW RETAINING WALL -

CC SITE SECTION C-C

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REV DATE INITIAL AMENDMEN

PROJECT STATUS

CLIENT

MELIA DEVELOPMENT LIMITED

20 MELIA PLACE

20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE

SITE SECTIONS

DESIGN DRAWN

CM LM

SCALE @ A1 (HALF SCALE IF PRINTED @ A3) 1:200

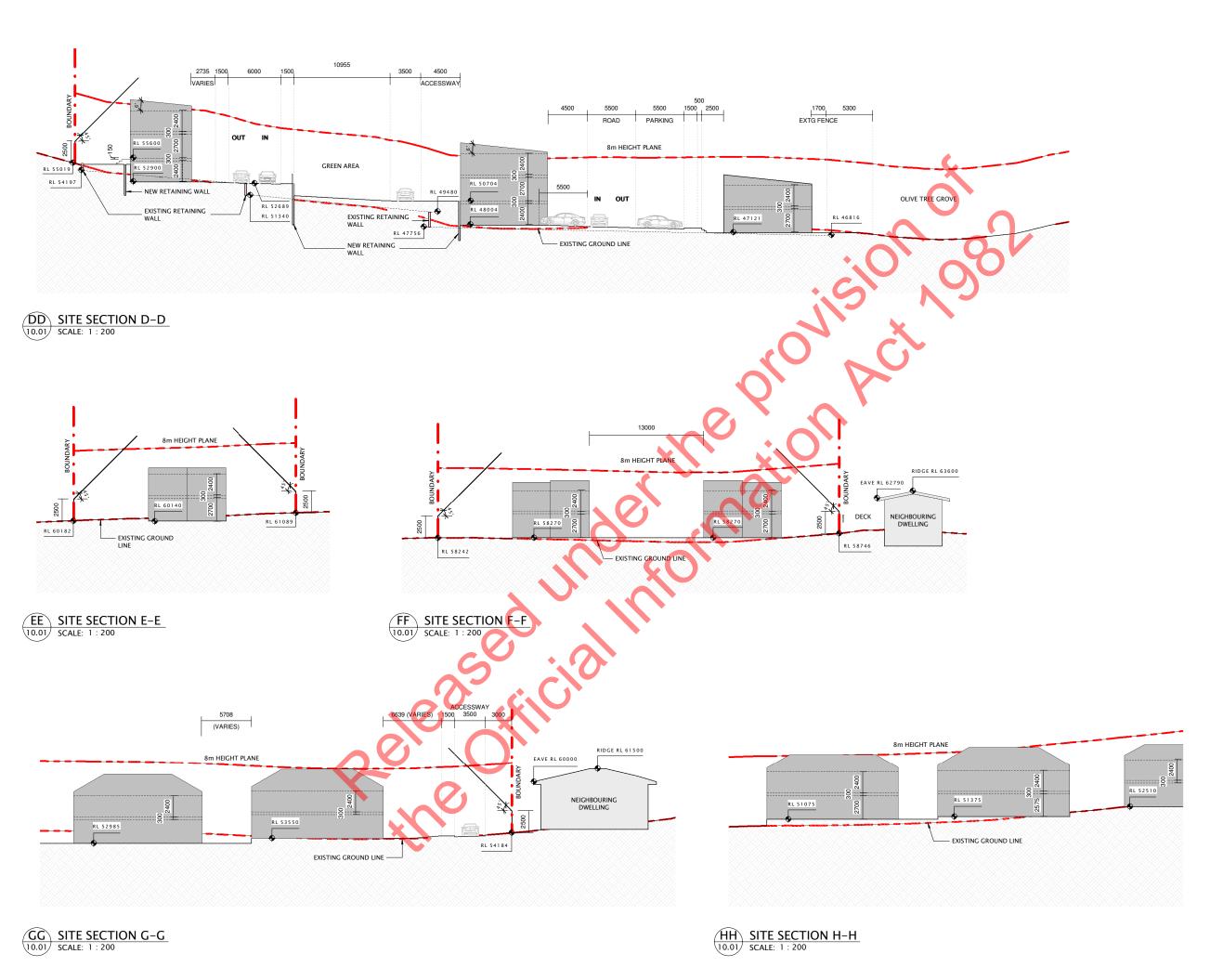
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MELIA DEVELOPMENT LIMITED

PROIECT

20 MELIA PLACE 20 MELIA PLACE, WHANGAPARAOA

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8m HEIGHT PLANE 6478 (VARIES) 1500 6000 DRIVEWAY ROAD EXISTING GROUND RL 51500 RL 51540 SCALE: 1:200 II SITE SECTION I-I 10.01 SCALE: 1 : 200 5000 8m HEIGHT PLANE (VARIES) RL 60437 RL 58120 EXISTING GROUND RL 52985 $\begin{array}{c|c} \hline KK \\ \hline 10.01 \\ \hline SCALE: \ 1:200 \\ \end{array}$ ROAD DRIVEWAY 8m HEIGHT PLANE RL 53131 RL 52360 RL 51225 - EXISTING GROUND LINE

MM\ SITE SECTION M-M

10.01 SCALE: 1 : 200

LL SITE SECTION L-L SCALE: 1:200

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5500 CARPARKING

EXISTING GROUND

8m HEIGHT PLANE

REV DATE INITIAL

PROJECT STATUS

SHEET TITLE

MELIA DEVELOPMENT LIMITED PROIECT

20 MELIA PLACE

20 MELIA PLACE, WHANGAPARAOA

SITE SECTIONS

DESIGN DRAWN CM LM

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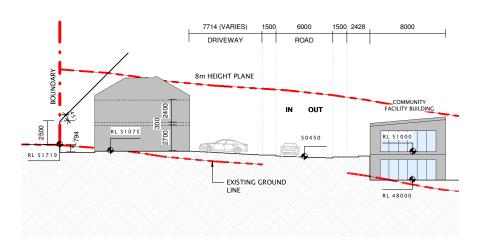
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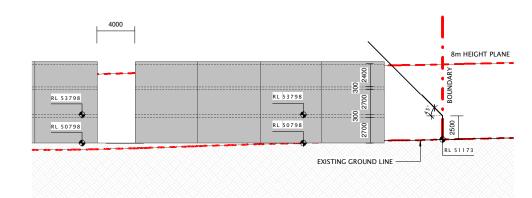
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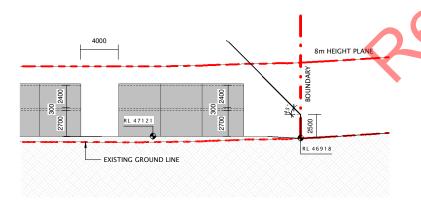
cullen + archaus



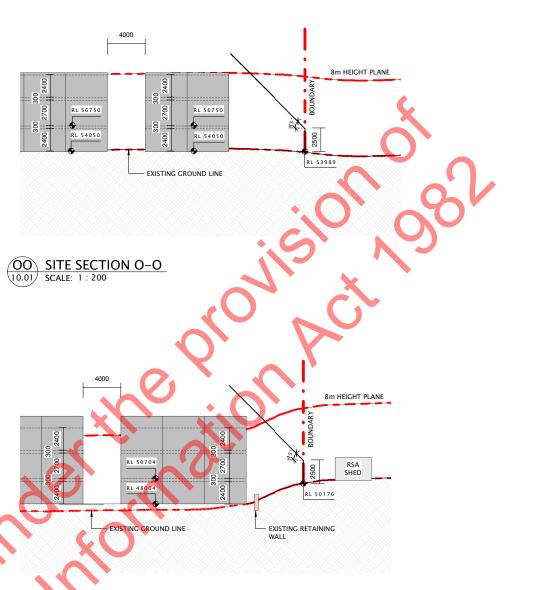
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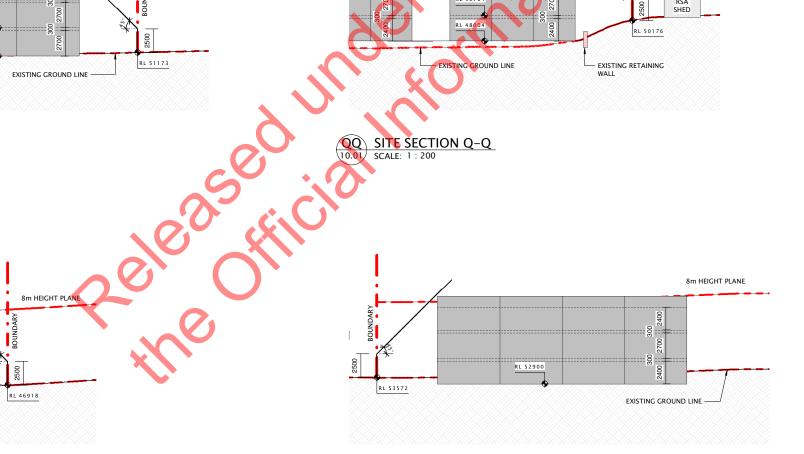


PP SITE SECTION P-P SCALE: 1:200



(RR) SITE SECTION R-R 10.01 SCALE: 1 : 200





SS SITE SECTION S-S 10.01 SCALE: 1 : 200

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DO NOT SCALE OFF DRAWING



REV DATE INITIAL

PROJECT STATUS

MELIA DEVELOPMENT LIMITED

PROIECT

20 MELIA PLACE

20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE

SITE SECTIONS

DESIGN DRAWN CM LM

SCALE @ A1
(HALF SCALE IF PRINTED @ A3 1:200

FIRST ISSUE DATE PROJECT No 20053

02/18/21

SHEET No.

40.04

paterson + cullen + archaus



Appendix B

Investigation Logs

	JOB NUMBER:	2282			4	HANE	AUGER NO.
	ADDRESS:	43A Vipond Road Stanmore Bay	l & 20 Melia I	Place,		Н	IA01
	WEATHER:	Fine			SHE	EET 1 OF	1
STUDIO	PROJECT:	Multiple Units			L	OGGED:	TL
	CLIENT:	KIPG				HEKCED:	
						L DATE: ED SHEAR	18/12/2020
DЕРТН (m)	SOIL DESCRI	PTION	LEGEND	GROUND	STRE (kF		SCA (BLOWS PE 0 5 1
Topsoil			مثد مثد ند مثد	<u> </u>			
brown, dark grey [ECBF] silty CLA 1 @1.2m become @1.6m become 2	and brown, very stif Y, orangey brown most segrey mottled orang ses moist to wet	ottled grey, very stiff, n	noist				

		JOB NUMBER:	2282			HAN	D AUGER N	٥.
		ADDRESS:	43A Vipond Road & 20 Stanmore Bay	Melia Pla	ice,	F	HA02	
		WEATHER:	Fine			SHEET 1 OF	1	
ST	TIDIO	PROJECT:	Multiple Units			LOGGED:	TL	
31		CLIENT:	KIPG			CHEKCED:		
						DRILL DATE:		
<u> </u>		ı				UNDRAINED SHEAR		
DEPTH (m)				LEGEND	GROUND	STRENGTH	S	CALA
Ļ		SOIL DESCR	PTION	99	SOL ATI	(kPa)	(BLOWS	PER 100mm)
DE				쁘	R ≥	0 50 100 150	0 5	10 15
	Topsoil							
	r opcon			<u> </u>				(
				44 44 44 44 44				
_	[ECBF] silty CLAY	, grey mottled oran	gey brown, very stiff, moist	===				
				===				
4								
├─ [`]						 	 	
	@1.1m becomes	moist to wet						
	230011100				5			
L								1 1
			asalt grit, orangey brown) ~		. [
—	morued gre <u>y and .</u> @1.6m becomes	da <u>rk orangey brow</u> r silty CLAY	ı, sum					
	@1.om becomes	Sity OL/ (1						
2					*. (-		
	@2 1m hecomes	grey mottled orang	ev brown wet		K		li i	i i
	@2. IIII becomes	grey motiled orang	cy blown, wet				i i	1 1
		grey mottled orang	ey brown and light brownish					
	grey, very stiff			1		ldl		
_							1	
·	@2.7m becomes	clayey SILT, bluish	arev	×J×J×			li i	i i
	O			×Ŷ×Ŷ×			1 1	
3	50D 00 0 T		· (() ·	× × × ×		├		
	EOB @3.0m, Tar	get depth reached						
							1 1	1 1
_							li i	i i
						l		
—								
		• (2)						1 1
							1 1	1 1
— ⁴						l	ii	
	0 .0						i i	1 1
		•				l		1 1
_							li i	į į
	7.							
5								
N 7								
NOTES:	- No groundwater	r table was encount	ered.					
G	EOSTUD	IO LIMITE	ED			GEOTECHNICA	L ENGINEER	RS
	/W.GEOSTUDIO.0		09- 476 1417	021-13	4 3823			
	ECTOR: GEOFFF		BE(CIVIL), ME(CIVIL), C					

		JOB NUMBER:	2282					HA	ND AUGE	R NO.
		ADDRESS:	43A Vipond Ro Stanmore Bay	ad & 20 Mel	lia Pla	ce,		Ī	HA0	3
		WEATHER:	Fine				SI	HEET 1 O		
ST	UDIO	PROJECT:	Multiple Units					LOGGED		
		CLIENT:	KIPG					CHEKCED		
	1			<u> </u>	1			NED SHEAF	: 18/12/20	020
DEPTH (m)		SOIL DESCR	IPTION		LEGEND	GROUND WATER	STR (ENGTH kPa) 100 150	(BLO	SCALA WS PER 1
	Topsoil			4	* *			T		
111233	[NON ENGINEER mottled grey, very [ECBF] clayey SIL @0.6m becomes @0.8m becomes @1.3m becomes @1.6m becomes @1.8m becomes	estiff, moist T, grey mottled oranges whitish grey, wet grey mottled oranges		_						
4	O									
	21									
5						1				·
5										

	JOB NUMBER:	2282			HAI	ND AUGER NO.
	ADDRESS:	43A Vipond Road & 20 Stanmore Bay	Melia Pla	ice,		HA04
	WEATHER:	Fine			SHEET 1 O	
STUDIO	PROJECT:	Multiple Units			LOGGED	
0.00.0	CLIENT:	KIPG			CHEKCED	
DEРТН (m)	SOIL DESCRIF	PTION	LEGEND	GROUND	UNDRAINED SHEAF STRENGTH (KPa) 50 100 150	SCALA (BLOWS PER 100mm 0 5 10 15
[NON ENGINEER grey, stiff, moist [ECBF] clayey SII moist to wet @0.6m becomes 1	LT, orangy brown mo	T minor gravel, brownish ttled brownish grey, stiff,	X			

	JOB NUMBER:	2282			HA	ND AUGER NO.
	ADDRESS:	43A Vipond Road Stanmore Bay	& 20 Melia Pl	ace,		HA05
	WEATHER:	Fine			SHEET 1 O	F 1
STUDIO	PROJECT:	Multiple Units			LOGGE	
	CLIENT:	KIPG			CHEKCEI	D: JS E: 18/12/2020
DЕРТН (m)	SOIL DESCR	IPTION	LEGEND	GROUND	UNDRAINED SHEAI STRENGTH (kPa)) 50 100 150	
Topsoil			216. 216. 216. 216. 216.			
grey, stiff, mois	st	T minor gravel, brown				
@1.2 become	s clayey SILT		×××× ×××× ×××××		0	
2 @1.8m becom	nes moist to wet	W.	* * * * * * * * * * * * * * * * * * *			
3 3 	Target depth reached	Jen C	X X X X X X X X X X X X X X X X X X X		•	
- - - - - - - - - - - - - - - - - - -	KICI/S					

		JOB NUMBER:	2282			HAI	ND AUGER NO.
		ADDRESS:	43A Vipond Roa Stanmore Bay	id & 20 Melia F	Place,		1A06
		WEATHER:	Fine			SHEET 1 O	
S1	UDIO	PROJECT:	Multiple Units			LOGGED	
		CLIENT:	KIPG			CHEKCED	
DEPTH (m)		SOIL DESCR	IPTION	LEGEND	GROUND	UNDRAINED SHEAF STRENGTH (kPa)	SCALA (BLOWS PER 1 0 5 10
	Topsoil [ECBF] silty CLA	.Y, orangy brown mo	ttled grey, very stiff, r	noist	## ##	30 100 130	50
_ _ 1	@0.8m become	s grey mottled oranç	y brown				
-					1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
2 	@2.0m become	es moist to wet	X		111111		
- - -			76,				- 1
3 	EOB @3.0m, Ta	rget depth reached	M) \ ===		•	-
4	25	c. CIO					
	Ŏ						- 1 1 1
5	2						
NOTES	- No groundwate	er table was encount	ered.				
		DIO LIMITI				GEOTECHNIC	

STI	(60	ADDRESS:	43A Vipond Ro	oad & 20 Me						
ST			Stanmore Bay		ella Pla	ice,		ŀ	TOAL	7
STI		WEATHER:	Fine				SH	HEET 1 OF		
	JDIO	PROJECT:	Multiple Units					LOGGED	: HQ	
		CLIENT:	KIPG					HEKCED		
_				1				ILL DATE: NED SHEAR	18/12/202	20
DEPTH (m)		SOIL DESCR	IPTION		LEGEND	GROUND	STR (I	ENGTH kPa) 100 150		SCALA S PER 1
	psoil				* * *		П	T		
		RED FILL] clayey SI ery stiff, moist to dr	LT, dark brown mott	tled light	*********			. (
	CBF] silty CLAY oist	, grey mottled light	orangey brown, ver	y stiff,						
_ 1	00.8m becomes	wet					0		.	
	01.2m becomes 01.3m becomes		prown mottled light g	grey	* * * * * * * * * * * * * * * * * * *	1		>	O	
)1.5m becomes	silty CLAY								
2)1.8m becomes	very wet	N.	C						
_ @)2.2m becomes	light brownish grey	mottled light brown	i, very stiff						1
_)2.6m becomes	clayey SILT, bluish	i gr e y	1	× × × × × × × × ×			+		
3	OB @3.0m, Tar	get depth reached	×()	*** ****		o			
		5,	11.					ļ		
	5	.0								
-4 -	<i>y</i>									
	\bigcirc									
) ,									
							L	4		

		JOB NUMBER:	2282			HA	ND AUGER NO	
		ADDRESS:	43A Vipond Road 8 Stanmore Bay	k 20 Melia Pla	ce,	I	80AH	
		WEATHER:	Fine			SHEET 1 O	F 2	
ST	TUDIO	PROJECT:	Multiple Units			LOGGED): HQ	
	ODIO	CLIENT:	KIPG			CHEKCED): JS	
							: 18/12/2020	
(n		-			0	UNDRAINED SHEAF		
DEPTH (m)		SOIL DESCR	IPTION	LEGEND	GROUND WATER	STRENGTH (kPa)) 50 100 150		ALA ER 100mm) 10 15
	Topsoil			** ** ** ** ** ** ** **				
			LT, light brown mottled lique in the lique i	ght				8
_ _ 1	@0.8m becomes light orangey brow		own mottled whitish grey					
- - -		silty CLAY minor g black, very stiff, mo	ravel, light brown mottled					
2	@1.8m becomes brown, grey & bro		own mottled light orange			•		
_ _ _		•	d grey & dark brown			•••		
3 			1010			O		
_ _ 4	@3.8m becomes	clayey SILT, dark	grey mottled dark brown			UTP		
6	@4.3m becomes orangey brown	dark brown mottle	d dark grey, whitish grey	&				
) - -	@4.6m becomes [ECBF] clayey SIL stiff, moist to wet	light grey mottled of T, light orangey br	I black & greenish black dark brown & light brown own mottled light grey , v	ery ×××		∪†P		
5	EOB @5.0m, Targ	get depth reached		: x x x x x x x x x x x x x x x x x x x			<u> </u>	1 1
NOTES	: - No groundwater	table was encoun	ered.					
G	EOSTUD	IO LIMITI	ED			GEOTECHNIC	AL ENGINEERS	3
	VW.GEOSTUDIO.C RECTOR: GEOFFR		09- 476 1417 BE(CIVIL), ME(CIVII	021-134 L), CPENG, M				

	JOB NUMBER:	2282			HAN	ID AUGER NO.	
	ADDRESS:	43A Vipond Road & 20 Stanmore Bay	Melia Pla	ice,	ŀ	80AH	
	WEATHER:	Fine			SHEET 2 OF		
STUDIO	PROJECT:	Multiple Units			LOGGED		
010010	CLIENT:	KIPG			CHEKCED	: JS	
			1		DRILL DATE UNDRAINED SHEAR	: 18/12/2020	•
(E) T	0011 DE00DIE	TION	QN	JND ER	STRENGTH (kPa)	SCA	ALA
DΕΡΤΗ (m)	SOIL DESCRIF	TION	LEGEND	GROUND) 50 100 150	(BLOWS PE	ER 100mi 10 15
	table was encounte	ared.					

		JOB NUMBER:	2282					BOREHOLE NO.
		ADDRESS:	2282_43A Vipond Road Place, Stanmore Bay	& 20 Meli	ia			MH01
		WEATHER:	Fine			SH	IEET 1 OF	3
61	TUDIO	PROJECT:	Multi Unit development			LOG	GED:	JS
31	ODIO	CLIENT:	KIPG			CHE	KCED:	GK
					İ	DRILL	DATE:	15/01/2021
DЕРТН (m)		SOIL DESCRIP	TION	LEGEND	Shear Vane (kPa)	Core Recovery (%)	SPT N- value	Notes:
	Topsoil							
1	@2.0m becomes	prown mottled grey	and dark brown				0 1 1 1 2 1 1 N=5	
M				<u> </u>				
NOTES:								
	EOSTUDI	O LIMITE	D 09- 476 1417	021-134	3823		GEOTE	ECHNICAL ENGINEERS
	ECTOR: GEOFFRE		BE(CIVIL), ME(CIVIL), C					

		JOB NUMBER:	2282					BOREHOLE NO.
		ADDRESS:	2282_43A Vipond Road Place, Stanmore Bay	& 20 Me	lia			MH01
		WEATHER:	Fine			SH	HEET 2 OF	: 3
67	TUDIO	PROJECT:	Multi Unit development			LOG	GED:	JS
31	ODIO	CLIENT:	KIPG			CHE	KCED:	GK
						DRILL	DATE:	15/01/2021
DEPTH (m)		SOIL DESCRI	PTION	LEGEND	Shear Vane (kPa)	Core Recovery (%)	SPT N- value	Notes:
	[ECBF] weathered of orangey brown	dark brown mottled y SILT, bluish grey				20 40 60 80	2 2 2 2 2 N=8 3 6 N=14	
G	EOSTUD	O LIMITE	=D				GEOTE	ECHNICAL ENGINEERS
	W.GEOSTUDIO.C		09- 476 1417	021-134	1 3823		GEOTE	LOTHIOAL LINGINEERS
	ECTOR: GEOFFR		BE(CIVIL), ME(CIVIL), C					

ADDRESS: 2282_43A Vipond Road & 20 Mells PROJECT: ADDRESS: Pine SHEET 3 OF 3 SHEET		JOB NUMBER: 2282				BOREHOLE NO.			
WEATHER: Fine SHEET 3 OF 3 LOGGED: JS CLIENT: KIPG SOIL DESCRIPTION BY GROUP STANDARD SOIL DESCRIPTION GROUP STANDARD		ADDRESS:		& 20 Me	lia			MH01	
CHEKCED: GK DRILL DATE: 15/01/2021 SOIL DESCRIPTION BY 10.0m becomes weathered SILT STONE, dark brown mottled dark grey 11 EOB @12.5m target depth reached. N=50+ for 35mm 13		WEATHER:	Fine			SH	HEET 3 OF	3	
SOIL DESCRIPTION SOIL DESCRIPTION General Section Solid Description General Section			Multi Unit development			LOG	GED:	JS	
SOIL DESCRIPTION SOIL DESCRIPTION Get 10.0m becomes weathered SILT STONE, dark brown mottled dark grey The state of the	210010	CLIENT:	KIPG			CHE	KCED:	GK	
@10.0m becomes weathered SILT STONE, dark brown mottled dark grey 26 24 N=50+ for 55mm N=50+ for 35mm 13 14						DRILL	. DATE:	15/01/2021	
dark grey 26 24 N=50+ for 55mm N=50+ for 35mm 13 14 14 14 15 15 16 16 16 16 16 16	DЕРТН (m)	SOIL DESCRIPT	TION	LEGEND	Shear Vane (kPa)	Recovery (%)		Notes.	
	@10.0m becomes dark grey		ONE, dark brown mottled	(2 2 40 60 80	24 N=50+ for 55mm N=50+ for		
		O LIMITFI	D				GEOTE	ECHNICAL ENGINEERS	

CLIENT: Silverdale RSA Auger Hole No: AH101 Soil&Rock Consultants Geotechnical Investigation, 43A Vipond Road & 20 PROJECT: Sheet 1 of 1 For well-grounded solutions Melia Place, Stanmore Bay Drill Type: 50mm Hand Auger Project No: 18334 DEG Logged By: Shear Vane No - Calibration Date: DR1768 - 21/12/2017 Drilled Rv DEG Coordinates Date Started: 4/10/18 Ground Elevation Surface Conditions: Slightly Sloping, Grass 4/10/18 Date Finished: Water Level: 3.0m 5/10/2018 SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 GRAPHIC LOG -ABORATORY **WATER LEVEL** $\widehat{\mathbb{E}}$ DEPTH (m) (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 20 30 (E DEPTH "Guidelines for Field Description of Soil and Rock in SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 SILT, minor clay, brown, firm, moist, non to slightly plastic \mathbf{Z} (TOPSOIL) 11/1/ clayey SILT, orange, very stiff, moist, moderately plastic (WEATHERED WAITEMATA GROUP SOILS) minor fine sand, orange, yellow, yellowish grey light yellow, orange, light grey some clay to clayey, trace fine to medium sand, slightly to moderately plastic 1.0 stiff to very stiff × light grey with orange and yellow mottles 122 1.5 stiff OND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R_2013.GDT 10/25/18 clayey, moist to wet, moderately plastic WEATHERED WAITEMATA GROUP SOILS 108 V 2.0 2.0 some clay to clayey, minor fine to medium sand, slightly to moderately plastic orange, pinkish orange, yellow orange with yellow speckles 5/10/2018 minor fine to medium sand, trace coarse sand 47 3.0 silty CLAY, orange, stiff, moist to wet, highly plastic clayey SILT, orange, yellow, stiff, moist to wet, moderately plastic some clay, some fine sand, orange and grey, slightly plastic grey 40 r 72\ orange and grey clayey, grey with orange streaks, moderately plastic some clay, some fine sand, grey 18334 - AH 01 trace lignite fragments as black speckles 38 r 4.0 SILT, some fine to medium sand to sandy, some clay, grey, stiff, wet, slightly plastic water seepage HAND AUGER LOG WITH SCALA LOG clayey SILT, grey, stiff, moist, to wet, moderately plastic some clay to clayey, minor fine to medium sand, slightly to moderately plastic 32 r 58 V 4.5 4.5 firm to stiff

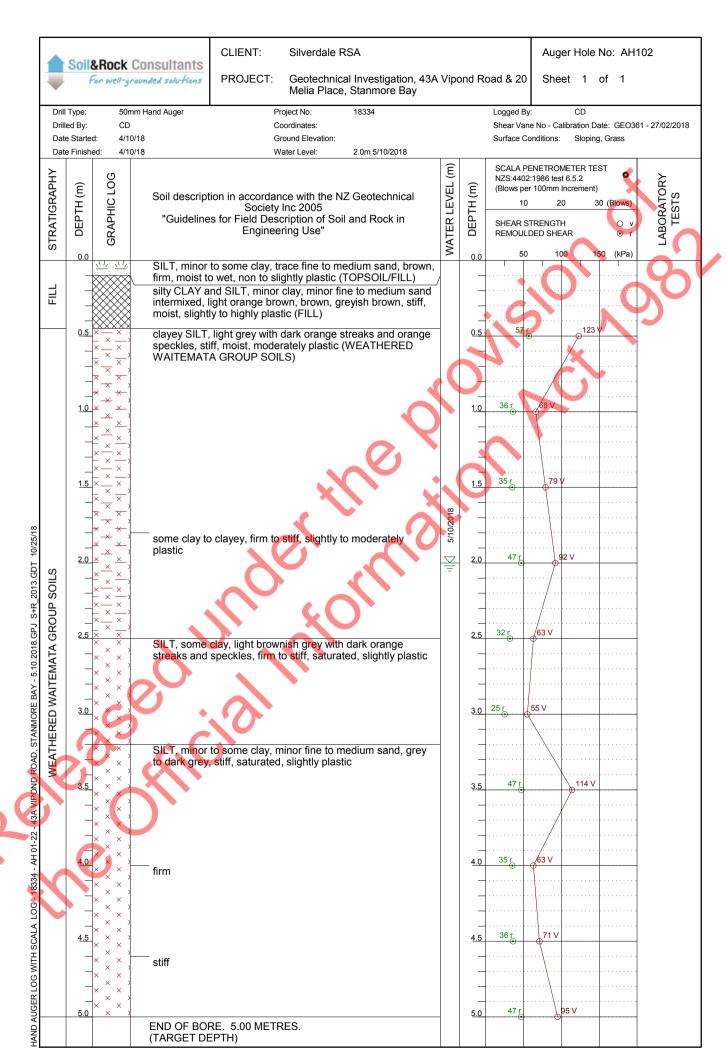
32 <u>r</u>

5.0

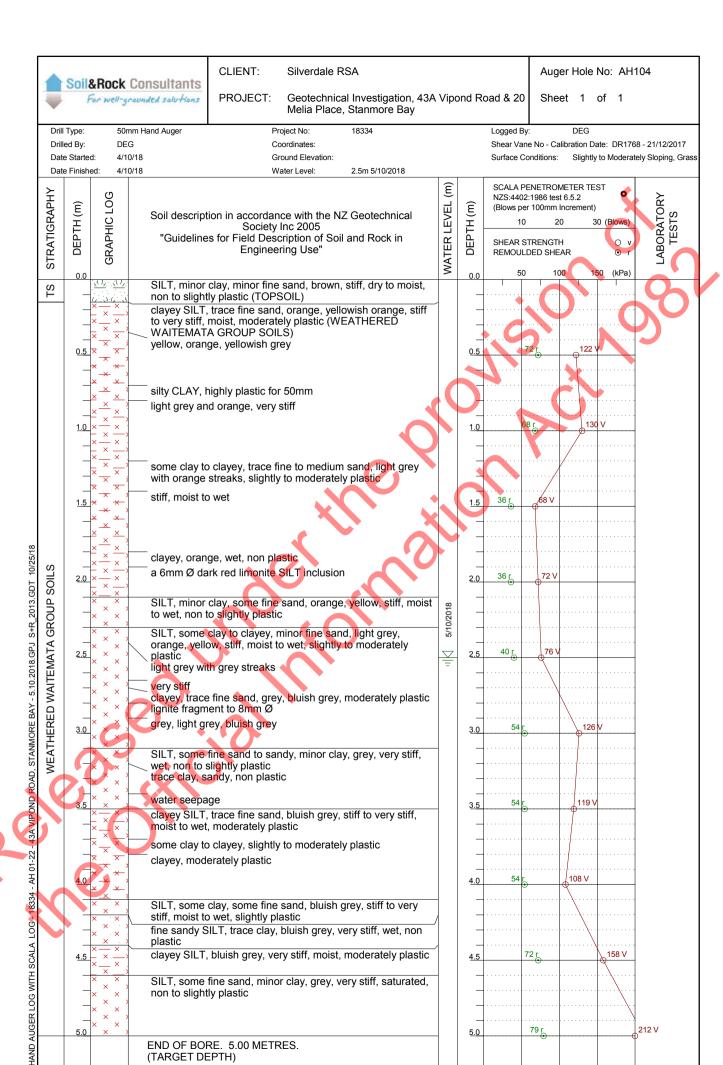
54 V

clayey, trace fine sand, moderately plastic

END OF BORE. 5.00 METRES.

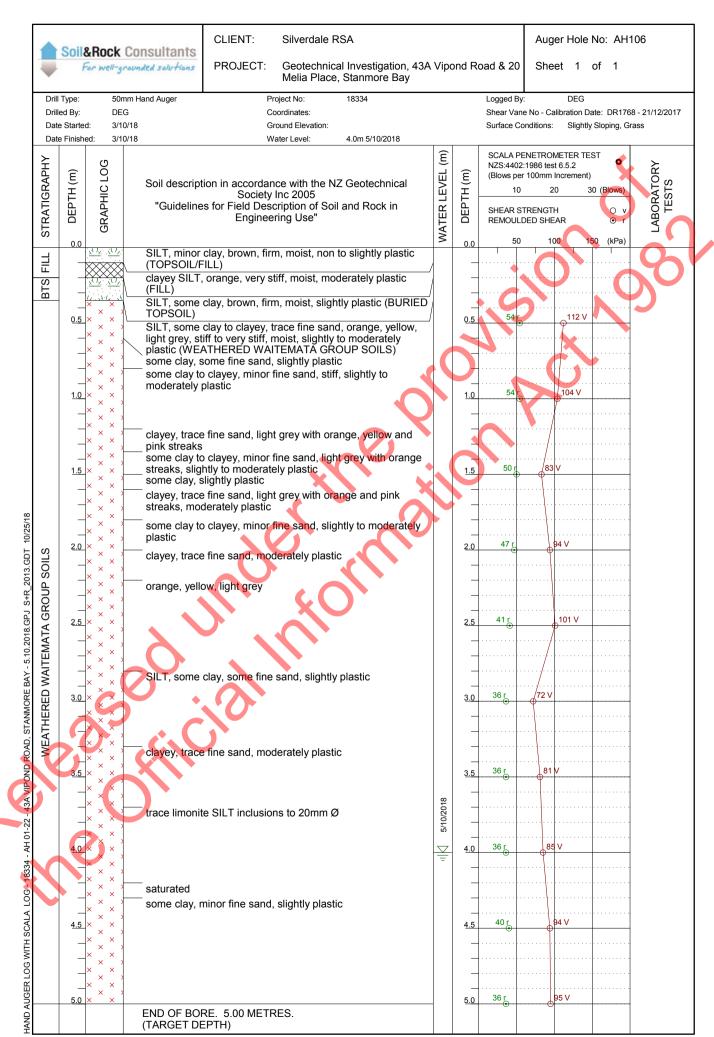


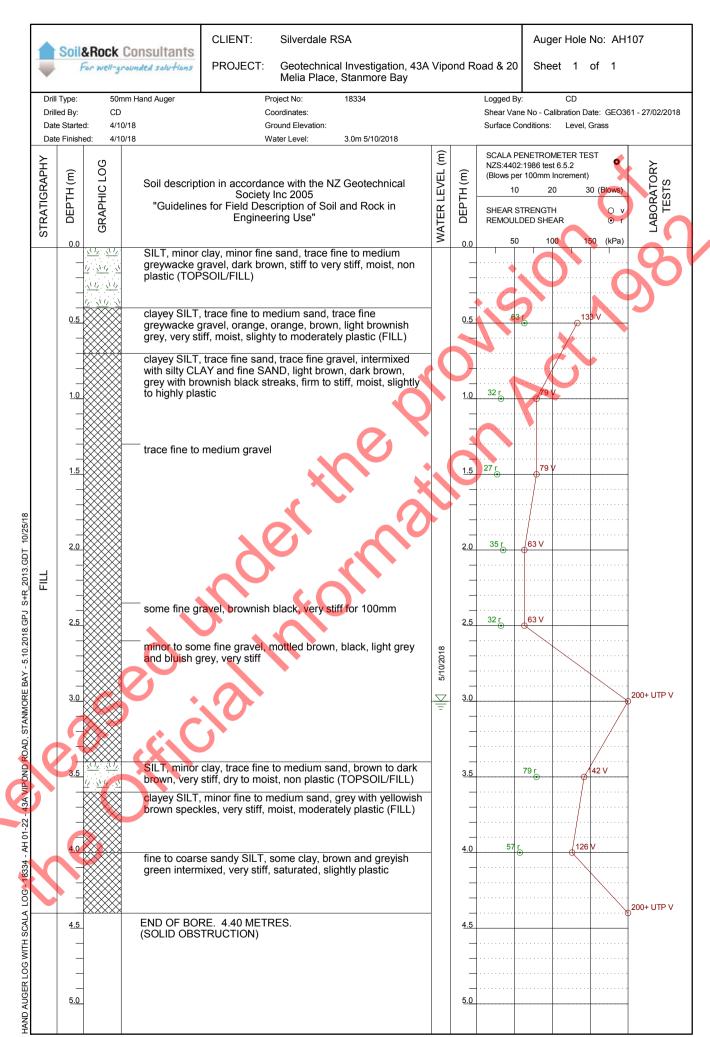
CLIENT: Silverdale RSA Auger Hole No: AH103 Soil&Rock Consultants PROJECT: Geotechnical Investigation, 43A Vipond Road & 20 Sheet 1 of 1 For well-grounded solutions Melia Place, Stanmore Bay Drill Type: 50mm Hand Auger Project No: 18334 CD Logged By: Drilled By: CD Shear Vane No - Calibration Date: GEO361 - 27/02/2018 Coordinates Date Started: 4/10/18 Ground Elevation: Surface Conditions: Sloping, Grass Date Finished: 4/10/18 Water Level: 1.9m 5/10/2018 SCALA PENETROMETER TEST $\widehat{\Xi}$ STRATIGRAPHY NZS:4402:1986 test 6.5.2 GRAPHIC LOG -ABORATORY **WATER LEVEL** $\widehat{\mathbb{E}}$ DEPTH (m) (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 30 (E DEPTH "Guidelines for Field Description of Soil and Rock in SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 SILT, trace to minor clay, trace fine to medium sand, dark brown, firm, moist, non plastic (TOPSOIL/FILL) clayey SILT, trace fine to medium sand intermixed with SILT, some fine sand, brown and light greyish brown intermixed, orange streaks, stiff, moist, slightly to moderately plastic clayey SILT, trace fine sand, light greyish brown with dark orange streaks and speckles, stiff, moist, moderately plastic (WEATHERED WAITEMATA GROUP SOILS) firm to stiff 28 <u>r</u> 1.0 some clay, stiff, slightly plastic DND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R_2013.GDT 10/25/18 trace to minor fine sand, moist to wet WEATHERED WAITEMATA GROUP SOILS 28 r 2.0 2.0 minor fine sand, wet to saturated 28 r SILT, minor to some fine to medium sand, trace to minor clay, grey, dark grey, stiff, wet to saturated, non plastic stiff 47 111 V 3.0 114 V ery stiff 18334 - AH 01 4.0 HAND AUGER LOG WITH SCALA LOG 200+ UTP V 4.5 200+ UTP V 5.0 END OF BORE. 5.00 METRES.



CLIENT: Silverdale RSA Auger Hole No: AH105 Soil&Rock Consultants Geotechnical Investigation, 43A Vipond Road & 20 PROJECT: Sheet 1 of 1 For well-grounded solutions Melia Place, Stanmore Bay 50mm Hand Auger Drill Type: Project No: 18334 DEG Logged By: DEG Drilled By: Coordinates Shear Vane No - Calibration Date: DR1768 - 21/12/2017 Date Started: 4/10/18 Ground Elevation Surface Conditions: Slightlly to Moderately Sloping, Gras 4/10/18 Date Finished: Water Level: 2.3m 5/10/2018 SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 **3RAPHIC LOG** -ABORATORY LEVEL $\widehat{\Xi}$ DEPTH (m) (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 20 30 (E DEPTH "Guidelines for Field Description of Soil and Rock in WATER I SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 \mathbf{S} SILT, minor clay, minor fine sand, brown, stiff, moist, non to slightly plastic (TOPSOIL) clayey SILT, orange, light grey, stiff, moist, moderately plastic (WEATHERED WAITEMATA GROUP SOILS) some clay to clayey, trace fine sand, slightly to moderately clayey, trace fine to coarse limonite sand, orange, wet, moderately plastic no limonité, light grey and orange 36 r 101 V 1.0 some clay to clayey, slightly to moderately plastic clayey, moderately plastic some clay to clayey, minor fine sand, slightly to moderately light grey, light yellowish grey 1.5 1.5 SILT, minor clay, some fine sand to sandy, grey, stiff, wet, non to slightly plastic orange grey, yellow, grey OND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R 2013.GDT 10/25/18 WEATHERED WAITEMATA GROUP SOILS 36 r 5/10/2018 2.0 clayey SILT, trace fine sand, bluish grey, very stiff, moist, moderately plastic some clay to clayey, slightly to moderately plastic clayey, moderately plastic 3.0 some clay to clayey, slightly to moderately plastic some clay, some fine sand, slightly plastic 36 r 153 V water seepage, saturated 18334 - AH 01-22 clayey, moderately plastic 148 V 4.0 some clay, some fine sand, slightly plastic HAND AUGER LOG WITH SCALA LOG some clay to clayey, minor fine sand, slightly to moderately plastic 108 189 V 4.5 4.5 some clay, some fine sand, slightly plastic 72 r 167 V 5.0

END OF BORE. 5.00 METRES.





CLIENT: Silverdale RSA Auger Hole No: AH108 Soil&Rock Consultants Geotechnical Investigation, 43A Vipond Road & 20 PROJECT: Sheet 1 of 1 For well-grounded solutions Melia Place, Stanmore Bay 50mm Hand Auger Project No: Drill Type: 18334 NC Logged By: Shear Vane No - Calibration Date: GEO119 - 23/11/2017 Drilled By: NC: Coordinates Date Started: 3/10/18 Ground Elevation Surface Conditions: Near Level, Grass Date Finished: 3/10/18 Water Level: 1.6m 5/10/2018 SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 GRAPHIC LOG -ABORATORY LEVEL DEPTH (m) Ξ (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 20 30 (E DEPTH "Guidelines for Field Description of Soil and Rock in WATER SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 SILT, trace clay, minor fine sand, dark brown, moist to wet, non plastic (TOPSOIL/FILL) 111, 글 SILT, some clay, minor fine to medium sand, orange, light grey, stiff, moist to wet, slightly plastic (FILL) clayey SILT, trace fine sand, light orange, light grey with pink streaks, stiff, saturated, moderately plastic (WEATHERED WAITEMATA GROUP SOILS) 117 \ 1.0 silty CLAY, minor fine to medium sand, orange, light grey light orange, very stiff, moist, highly plastic 5/10/2018 brown organic staining SILT, some clay to clayey, minor fine to medium sand, 86 156 V 1.5 1.5 orange, light orange, very stiff, moist, slightly to moderately plastic OND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R_2013.GDT 10/25/18 brown organic stains and mottles 2.0 **VEATHERED WAITEMATA GROUP SOILS** clayey, trace fine sand, orange light grey, very stiff, moderately plastic 78 r some clay, slightly plastic fine to medium sandy SILT, trace to minor clay, light brownish grey, stiff, saturated, non to slightly plastic 26 r clayey SILT, minor fine to medium sand, brownish orange, 18334 - AH 01 stiff, saturated, non plastic 4.0 HAND AUGER LOG WITH SCALA LOGfine to medium sandy SILT, trace clay, orange, light orange, stiff, saturated, non plastic 4.5 4.5 clayey SILT, trace fine sand, light grey, pink streaks, stiff, saturated, moderately plastic

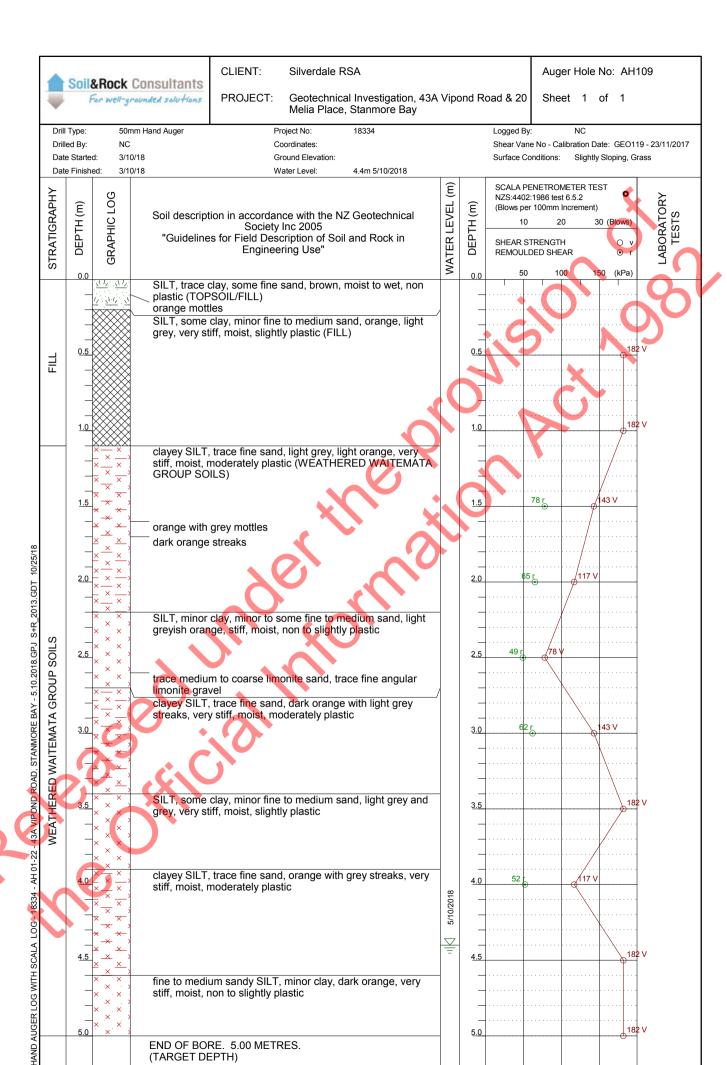
END OF BORE. 5.00 METRES.

(TARGET DEPTH)

52

5.0

99 V



CLIENT: Silverdale RSA Auger Hole No: AH110 Soil&Rock Consultants PROJECT: Geotechnical Investigation, 43A Vipond Road & 20 Sheet 1 of 1 For well-grounded solutions Melia Place, Stanmore Bay Drill Type: 50mm Hand Auger Project No: 18334 Logged By: DEG Drilled By: DEG Coordinates Shear Vane No - Calibration Date: DR1768 - 21/12/2017 Date Started: 3/10/18 Ground Elevation Surface Conditions: Slightly Sloping, Trees 3/10/18 Date Finished: Water Level: 1.9m 5/10/2018 SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 **3RAPHIC LOG** -ABORATORY LEVEL DEPTH (m) Ξ (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 20 30 (B DEPTH "Guidelines for Field Description of Soil and Rock in WATER SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 0.0 SILT, minor clay, minor fine sand, brown, stiff, dry to moist, non to slightly plastic (TOPSOIL/FILL) 1/ 1/1/ 1 some dark orange clayey SILT inclusions 11/2:11/ SILT, some clay, topsoil and clayey SILT intermixed, some fine to coarse sand, brown, dark brown, grey, dark orange, stiff, moist, slightly to moderately plastic (FILL) 긆 trace decomposed wood fragments to 5mm Ø SILT, some clay, brown, firm, moist, slightly plastic (TOPSOIL/FILL) 1.0 1.0 clayey SILT, greyish brown, light grey, stiff, moist, moderately plastic (FILL) \vdash SILT, minor clay, brown, moist, non to slightly plastic (BURIED TOPSOIL) × clayey SILT, trace fine sand, yellow, orange, yellowish orange, stiff, moist, moderately plastic (WEATHERED 32 1.5 1.5 WAITEMATA GROUP SOILS) silty CLAY, yellow, orange, light grey, stiff, moist to wet, -22 - 434 VIPOND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R_2013.GDT 10/25/18 highly plastic 2.0 2.0 clayey SILT, trace fine sand, yellow, orange, very stiff, moist, moderately plastic saturated 43 إ WEATHERED WAITEMATA GROUP SOILS SILT, minor clay, some fine sand, bluish grey with orange streaks, very stiff, wet, non to slighty plastic 148 V 36 r 3.0 some limonite SILT for 50mm, dark red clayey SILT, bluish grey, grey, orange, very stiff to hard, 216 V saturated, moderately plastic SILT, minor clay, minor fine sand, dark orange, hard, saturated, non to slightly plastic grey, dark orange some clay, slightly plastic - AH 01 200+ UTP V 4.0 18334 some fine sand, grey, very stiff to hard HAND AUGER LOG WITH SCALA LOGvery stiff 216 V 4.5 4.5 very stiff to hard

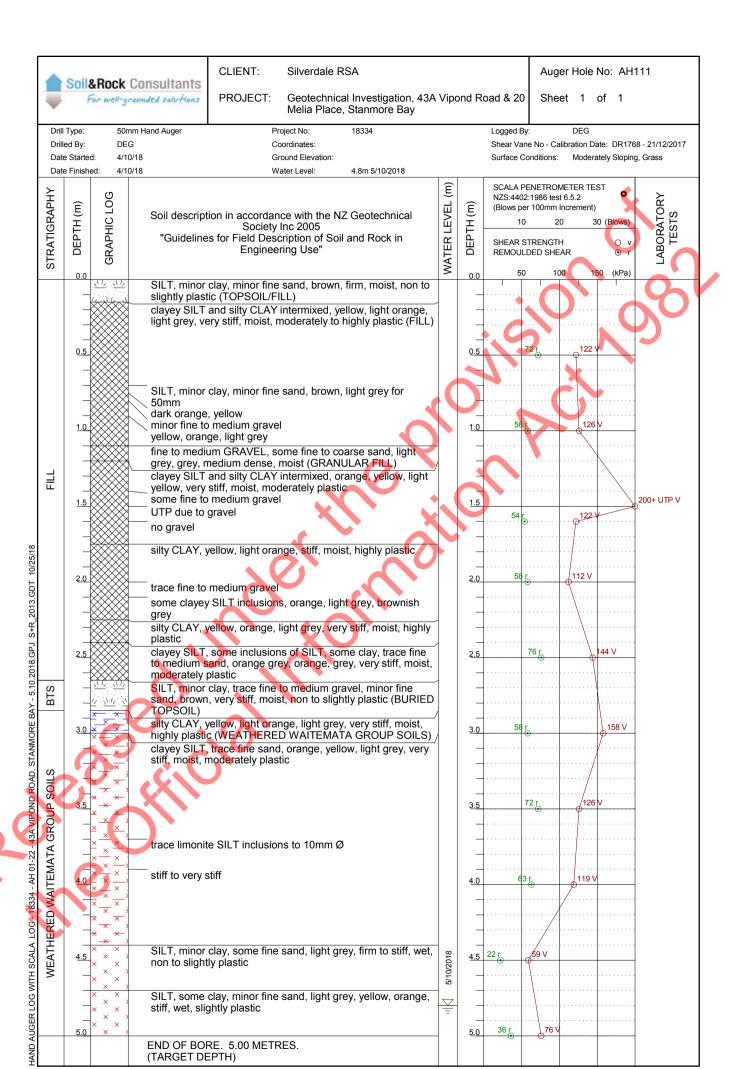
5.0

200+ UTP V

some clay to clayey, hard, slightly to moderately plastic

fine SAND, some silt, grey, medium dense, saturated

END OF BORE. 5.00 METRES





DND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R_2013.GDT 10/25/18

18334 - AH 01.

HAND AUGER LOG WITH SCALA LOG

CLIENT: Silverdale RSA

Geotechnical Investigation, 43A Vipond Road & 20 PROJECT:

Sheet 1 of 1

Auger Hole No: AH112

Melia Place, Stanmore Bay Project No: 18334 DEG Logged By: Coordinates Shear Vane No - Calibration Date: DR1768 - 21/12/2017 Date Started: 3/10/18 Ground Elevation Surface Conditions: Moderately Sloping, Grass 3/10/18 Date Finished: Water Level: 2.2m 5/10/2018 SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 **3RAPHIC LOG** -ABORATORY LEVEL DEPTH (m) Ξ (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 20 30 (E DEPTH "Guidelines for Field Description of Soil and Rock in WATER SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 0.0 SILT, some clay, brown, firm, wet, slightly plastic (TOPSOIL/FILL) 1/ 1/1/ 1 some organic clayey SILT inclusions to 20mm Ø SILT, some clay, some clayey SILT inclusions, greyish brown, orange, yellowish brown, firm, wet, slightly plastic tree roots to 1mm Ø some decomposed wood fragments to 10mm Ø no decomposed wood some fine to coarse sand silty CLAY, orange, yellow, firm, wet, highly plastic 22 r 1.0 1.0 clayey SILT, trace fine gravel, light grey, orange, yellow, firm to stiff, moist to wet, moderately plastic wet SILT, some clay, yellow, orange, soft, saturated, slightly 63 V 1.5 1.5 plastic, hole collapse in this layer clayey SILT, light grey, yellow, orange, grey, yellow, light yellow, firm, saturated, moderately plastic some silty CLAY inclusions minor fine to medium gravel 5/10/201 clayey SILT, trace fine to coarse sand, organic stained <u>√162</u> V 90 g 2.0 2.0 brown, very stiff, moist, moderately plastic some fine to medium gravel trace gravel light grey, light yellow with brown streaks 2.5 20 r no gravel, light grey, yellow, brown SILT, some clay, some fine to coarse sand, organic stained brown, greyish brown, very stiff, saturated, slightly plastic minor fine to medium gravel piece of yellow plastic to 6mm Ø trace grass inclusions 43 1 <u>3.5</u> race fine to medium gravel, trace decomposed wood fragments to 20mm Ø some light grey streaks 90 r 4.0 SILT, some clay, brown, very stiff, saturated, slightly plastic (BURIED TOPSOIL) silty CLAY, yellow, orange, stiff, saturated, highly plastic (WEATHERED WAITEMATA GROUP SOILS) 108 V 4.5 yellow, orange, light grey 5.0 END OF BORE. 5.00 METRES. (TARGET DEPTH)

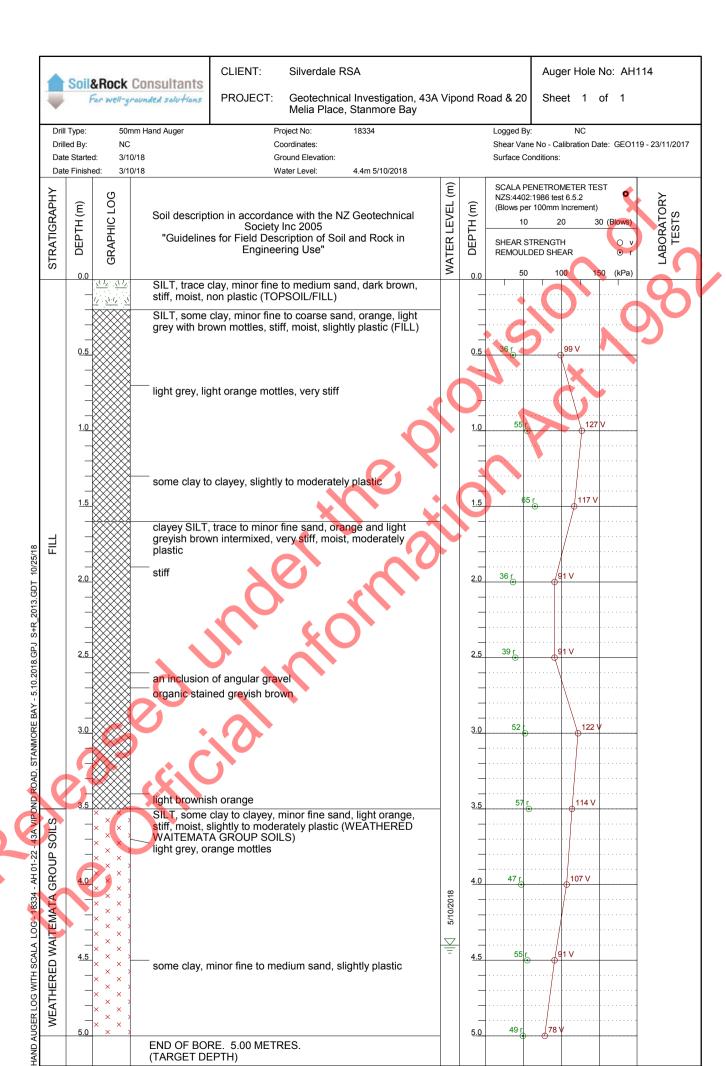


CLIENT: Silverdale RSA

Geotechnical Investigation, 43A Vipond Road & 20 PROJECT:

Auger Hole No: AH113

Sheet 1 of 1 Melia Place, Stanmore Bay Drill Type: 50mm Hand Auger Project No: 18334 Logged By: NG Shear Vane No - Calibration Date: GEO122 - 1/12/2017 Drilled Rv NG Coordinates Date Started: 3/10/18 Ground Elevation: Surface Conditions: Sloping, Soil 3/10/18 Date Finished: Water Level: 2.0m 5/10/2018 SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 **3RAPHIC LOG** -ABORATORY LEVEL DEPTH (m) Ξ (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 20 30 (B DEPTH "Guidelines for Field Description of Soil and Rock in WATER SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 0.0 SILT, some clay, trace fine sand, brown, black, yellow, stiff, moist, slightly plastic (TOPSOIL/FILL) 1/ 1/1/ 7 11/2:11/ brown with orange streaks 1/ 11/ Ⅱ SILT, some fine sand, minor clay, brown with orange mottles, stiff, moist, non to slightly plastic 17. 11.17. 3 11/2 11/ trace roots clayey SILT, minor fine sand, brown, orange streaks, stiff moist, moderately plastic (PUKETOKA FORMATION) 22 104 V 1.0 1.0 PUKETOKA FORMATION orange with brownish grey streaks SILT, some fine sand, trace clay, orange with brownish grey × streaks, stiff, wet, non plastic 1.5 1.5 SILT, some clay to clayey, orange and brownish grey, stiff, wet, slightly to moderately plastic SILT, minor to some fine sand, minor to some clay, orange 2.0 18 г 2.0 with brown and grey streaks, stfff, wet, non to slightly plastic (WEATHERED WAITEMATA GROUP SOILS) -22 - 43A VIPOND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R 2013.GDT SILT, minor fine sand, minor clay, orange with brown and grey streaks, very stiff, wet to saturated, non to slightly plastic 27 r saturated 2.5 WEATHERED WAITEMATA GROUP SOILS minor to some clay, non to slightly plastic SILT, some clay to clayey, minor fine sand, orange with 60 V 30 r brown and grey streaks, very stiff, saturated, slightly to moderately plastic trace fine siltstone gravel SILT, minor clay, minor fine sand, grey, very stiff, saturated, 200+ UTP V non to slightly plastic orange limonite as 3mm wide streaks 18334 - AH 01 200+ UTP V 4.0 HAND AUGER LOG WITH SCALA LOG-200+ UTP V END OF BORE. 4.40 METRES. 4.5 4.5 (TOO HARD TO AUGER) 5.0 5.0



CLIENT: Silverdale RSA Auger Hole No: AH115 Soil&Rock Consultants Geotechnical Investigation, 43A Vipond Road & 20 PROJECT: Sheet 1 of 1 For well-grounded solutions Melia Place, Stanmore Bay Project No: Drill Type: 50mm Hand Auger 18334 CD Logged By: CD Shear Vane No - Calibration Date: GEO361 - 27/02/2018 Drilled By: Coordinates Date Started: 3/10/18 Ground Elevation Surface Conditions: Sloping, Soil 3/10/18 Date Finished: Water Level: Not Encountered SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 GRAPHIC LOG -ABORATORY **WATER LEVEL** DEPTH (m) Ξ (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 20 30 (E DEPTH "Guidelines for Field Description of Soil and Rock in SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 SILT, minor fine to medium sand, trace clay, trace fine gravel, brown to dark brown, firm, dry to moist, non plastic 711/ (TOPSOIL/FILL) SILT, some clay, trace fine to medium sand, intermixed SILT inclusions, orange brown and light grey intermixed, stiff to very stiff, moist, slightly plastic (FILL) clayey, moderately plastic fine angular gravel for 50mm clayey SILT to silty CLAY, light orange brown with brown 1.0 and grey mottles, stiff, moist, moderately to highly plastic black streaks trace fine siltstone gravel DND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R_2013.GDT 10/25/18 clayey SILT with some fine angular greywacke gravel, trace fine to medium sand, orange brown, brown and bluish grey intermixed, stiff, moist, moderately plastic clayey SILT to silty CLAY, light grey, orange and yellow intermixed, stiff, moist, moderately to highly plastic clayey SILT, minor fine gravel, trace to minor fine to medium sand, orange brown and orange light grey intermixed, bluish grey speckles, stiff to very stiff, moist, moderately plastic very stiff intermixed with topsoil inclusions 79 r 85 r 4.0 minor lenses of light grey fine sand <10mm thick HAND AUGER LOG WITH SCALA LOG 4.5 SILT, some clay, trace to minor fine to medium sand, trace fine gravel, grey and light grey intermixed, very stiff, dry to moist, non to slightly plastic 5.0

END OF BORE. 5.00 METRES.

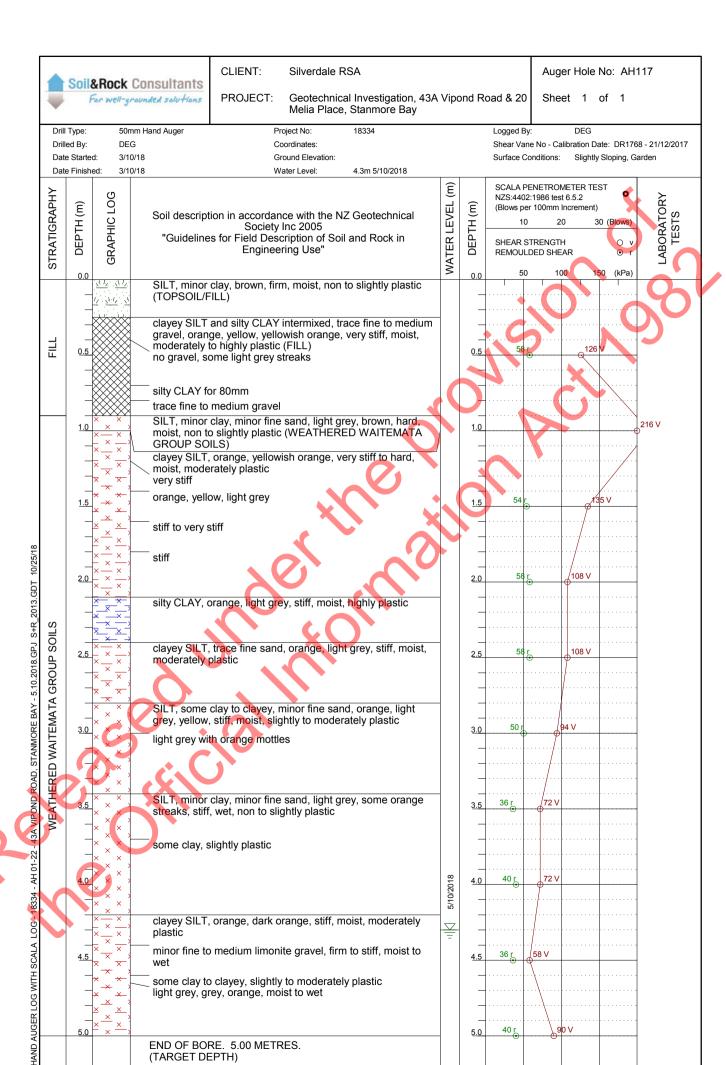
CLIENT: Silverdale RSA Auger Hole No: AH116 Soil&Rock Consultants Geotechnical Investigation, 43A Vipond Road & 20 PROJECT: Sheet 1 of 1 For well-grounded solutions Melia Place, Stanmore Bay 50mm Hand Auger Drill Type: Project No: 18334 NG Logged By: Shear Vane No - Calibration Date: GEO122 - 1/12/2017 Drilled By: NG Coordinates Date Started: 3/10/18 Ground Elevation Surface Conditions: Level Grass 3/10/18 Date Finished: Water Level: 5.0m 5/10/2018 SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 GRAPHIC LOG -ABORATORY LEVEL DEPTH (m) Ξ (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 20 30 (E DEPTH "Guidelines for Field Description of Soil and Rock in WATER I SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 0.0 SILT, minor clay, minor fine sand, brown and black, stiff, moist, non to slightly plastic (TOPSOIL/FILL) 111, SILT, some clay to clayey, trace fine sand, trace fine angular gravel, orange with black and grey streaks, very stiff, moist, slightly to moderately plastic (FILL) clayey SILT, moderately plastic fine sandy SILT, brown and grey, stiff to very stiff, dry, non ᆵ plastic clayey SILT, orange, grey and brown streaks, very stiff moist, moderately plastic 100mm of SILT, some clay as buried topsoil SILT, some clay, trace fine sand, brown, very stiff, moist, 210 V 2.0 2.0 434 VIPOND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R 2013.GDT slightly plastic (BURIED TOPSOIL) 1.11 clayey SILT, dark grey, brown, yellow, very stiff, moist, moderately plastic (WEATHERED WAITEMATA GROUP silty CLAY, orange and yellow with grey streaks, very stiff, moist, highly plastic GROUP SOILS 33 r 3.0 SILT, some clay to clayey, orange and yellow with grey streaks, very stiff, moist, slightly to moderately plastic 451 WEATHERED WAITEMAT stiff HAND AUGER LOG WITH SCALA LOG-18334 - AH 01-22 silty CLAY, grey with orange and yellow streaks, stiff, wet, 34 <u>r</u> 4.0 highly plastic trace roots for 50mm 42 r 4.5 5/10/2018 clayey SILT to silty CLAY, orange and grey, stiff, moist, moderately to highly plastic

END OF BORE. 5.00 METRES.

(TARGET DEPTH)

102 V

5.0



END OF BORE. 5.00 METRES.

(TARGET DEPTH)

40 r

5.0

CLIENT: Silverdale RSA Auger Hole No: AH118 Soil&Rock Consultants Geotechnical Investigation, 43A Vipond Road & 20 PROJECT: Sheet 1 of 1 For well-grounded solutions Melia Place, Stanmore Bay Drill Type: 50mm Hand Auger Project No: 18334 Logged By: DEG DEG Drilled Rv Coordinates Shear Vane No - Calibration Date: DR1768 - 21/12/2017 Date Started: 5/10/18 Ground Elevation Surface Conditions: Slightly Sloping, Garden 5/10/18 Date Finished: Water Level: 3.1m 5/10/2018 SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 GRAPHIC LOG -ABORATORY LEVEL DEPTH (m) Ξ (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 20 30 (E DEPTH "Guidelines for Field Description of Soil and Rock in WATER SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 0.0 SILT, trace clay, some fine to medium gravel, brown, firm, Ħ dry, non plastic (TOPSOIL/FILL) 711 no gravel a 30mm Ø root clayey SILT, orange, stiff, moist, moderately plastic (WEATHERED WAITEMATA GROUP SOILS) 108 V yellow and orange trace fine sand some clay to clayey, minor fine sand, slightly to moderately plastic clayey, trace fine sand, slightly to moderately plastic 1.0 1.0 × × some clay to clayey, light grey, yellow, orange, slightly to moderately plastic × clayey, trace fine sand, moderately plastic some clay to clayey, minor fine sand, light grey with yellow mottles, slightly to moderately plastic 36 1.5 1.5 moist to wet OND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R_2013.GDT 10/25/18 some clay, some fine sand, slightly plastic WEATHERED WAITEMATA GROUP SOILS 40 r 2.0 2.0 clayey, trace fine sand, moderately plastic × some clay, minor fine sand, slightly plastic, water seepage 40 g 5/10/2018 wet dark orange, yellow 49 76 3.0 clayey, trace fine sand, moderately plastic SILT, some fine to medium sand to sandy, minor clay, orange, stiff, saturated, non to slightly plastic clayey SILT, orange, stiff, saturated, moderately plastic some clay to clayey, minor fine sand, grey with orange 112 V streaks, slightly to moderately plastic clayey, bluish grey, very stiff, moderately plastic 18334 - AH 01 4.0 fine sandy SILT, minor clay, grey, very stiff, saturated, non HAND AUGER LOG WITH SCALA LOGclayey SILT, trace fine sand, bluish grey, stiff to very stiff, saturated, moderately plastic some clay to clayey, stiff, slightly to moderately plastic 36 r 4.5 4.5 clayey, very stiff, moderately plastic 72 r

5.0

200+ UTP V

very stiff to hard

(TARGET DEPTH)

END OF BORE. 5.00 METRES.

CLIENT: Silverdale RSA Auger Hole No: AH119 Soil&Rock Consultants Geotechnical Investigation, 43A Vipond Road & 20 PROJECT: Sheet 1 of 1 For well-grounded solutions Melia Place, Stanmore Bay Drill Type: 50mm Hand Auger Project No: 18334 DEG Logged By: DEG Shear Vane No - Calibration Date: DR1768 - 21/12/2017 Drilled By: Coordinates Date Started: 3/10/18 Ground Elevation Surface Conditions: Slightly Sloping, Garden 3/10/18 Date Finished: Water Level: 2.6m 5/10/2018 SCALA PENETROMETER TEST Ξ STRATIGRAPHY NZS:4402:1986 test 6.5.2 GRAPHIC LOG -ABORATORY **WATER LEVEL** DEPTH (m) Ξ (Blows per 100mm Increment) Soil description in accordance with the NZ Geotechnical Society Inc 2005 10 20 30 (E DEPTH "Guidelines for Field Description of Soil and Rock in SHEAR STRENGTH **Engineering Use"** REMOULDED SHEAR 100 0.0 SILT, minor clay, some fine to medium gravel, trace fine to coarse sand, brown, firm, moist, non to slightly plastic 111/ (TOPSOIL/FILL) clayey SILT and silty CLAY intermixed, trace fine to coarse 븚 sand, trace fine gravel, yellow, orange, very stiff, moist, moderately to highly plastic (FILL) SILT, minor clay, minor fine sand, light grey, light brownish grey, very stiff, moist, non to slightly plastic (WEATHERED WAITEMATA GROUP SOILS) clayey SILT, trace fine sand, orange, yellow, very stiff, moist, 158 V 1.0 moderately plastic some greyish brown organic stains organic stained greyish brown for 30mm no organic stains, stiff 104 V orange, yellow, light grey OND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R_2013.GDT 10/25/18 silty CLAY, yellow, orange, light grey, stiff, moist to wet, highly plastic 2.0 EATHERED WAITEMATA GROUP SOILS 5/10/2018 clayey SILT, trace fine sand, yellow, orange, light grey, stiff, moist, moderately plastic light grey, some orange streaks 40 r SILT, minor clay, minor fine sand, orange, light grey, firm to 22 r stiff, wet, non to slightly plastic, water seepage SILT, some clay to clayey, trace fine sand, orange, firm to stiff, wet, slightly to moderately plastic saturated orange with grey streaks 18334 - AH 01 67 V 4.0 SILT, minor clay, minor fine sand, grey, firm to stiff, saturated, non to slightly plastic HAND AUGER LOG WITH SCALA LOG some clay, slightly plastic minor clay, non to slightly plastic 4.5 18 r 58 V 4.5 clayey SILT, grey, stiff, saturated, moderately plastic SILT, some clay, minor fine sand, stiff, saturated, slightly

76

36 r

5.0

plastic

(TARGET DEPTH)

END OF BORE. 5.00 METRES.

CLIENT: Silverdale RSA Soil&Rock Consultants PROJECT: For well-grounded solutions Melia Place, Stanmore Bay Drill Type: 50mm Hand Auger Project No: 18334 NC Drilled By: Coordinates Date Started: 3/10/18 Ground Elevation: 3/10/18 Date Finished: Water Level: 3.9m 5/10/2018 STRATIGRAPHY GRAPHIC LOG DEPTH (m) Soil description in accordance with the NZ Geotechnical Society Inc 2005 "Guidelines for Field Description of Soil and Rock in **Engineering Use"** fine sandy SILT, trace clay, light grey, very stiff, moist, non plastic (FILL) trace to minor coarse sand, dark orange streaks Ⅱ clayey SILT, trace fine sand, light grey with pink streaks, very stiff, moist, moderately plastic (WEATHERED WAITEMATA GROUP SOILS) minor fine sand, orange mottles and streaks 1.0

pink streaks and orange mottles, trace fine sand

minor fine to medium sand, dark orange

trace fine sand, light orange mottles

well cemented limonite for 3mm

saturated, moderately plastic

END OF BORE. 5.00 METRES.

(TARGET DEPTH)

minor fine sand, stiff

clayey SILT, trace fine sand, bluish grey, very stiff,

light grey

DND ROAD, STANMORE BAY - 5.10.2018.GPJ S+R_2013.GDT 10/25/18

18334 - AH 01

HAND AUGER LOG WITH SCALA LOG

WEATHERED WAITEMATA GROUP SOILS

2.0

Geotechnical Investigation, 43A Vipond Road & 20

 Ξ

WATER LEVEL

5/10/2018

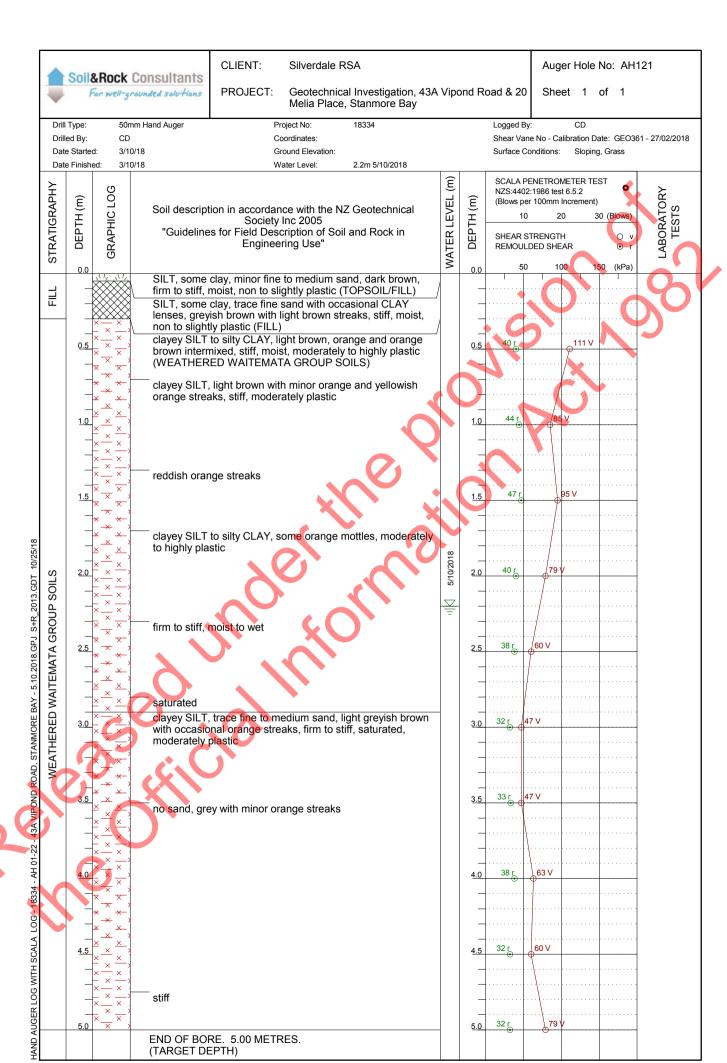
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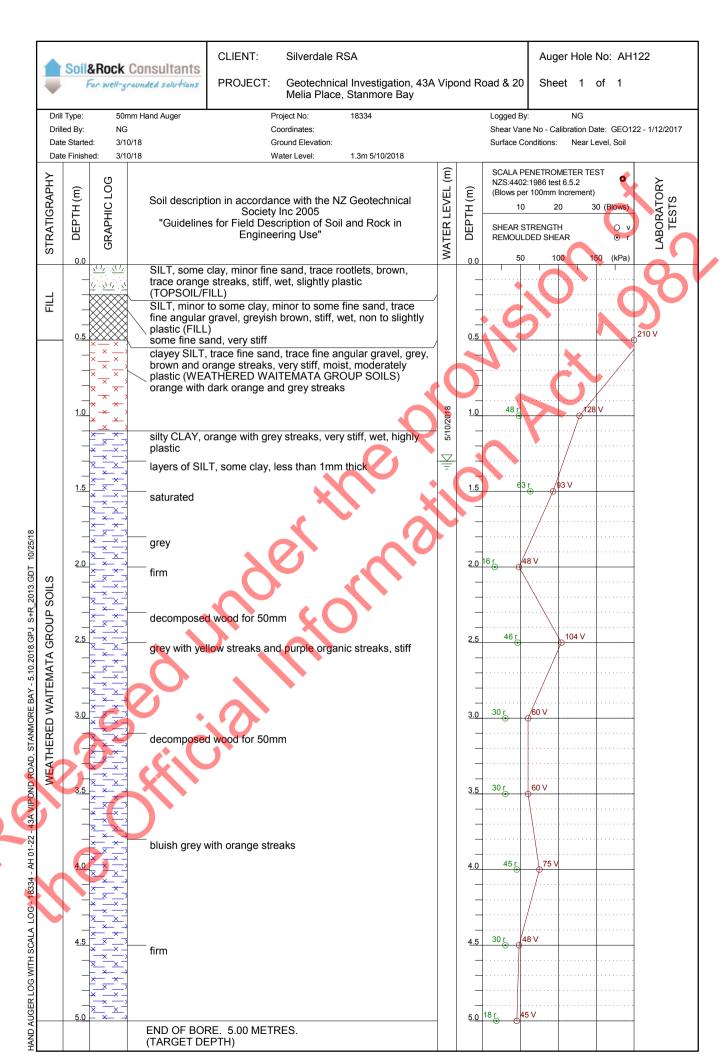
5.0

Auger Hole No: AH120

Sheet 1 of 1 NC Logged By: Shear Vane No - Calibration Date: GEO119 - 23/11/2017 Surface Conditions: Slightly Sloping, Grass SCALA PENETROMETER TEST NZS:4402:1986 test 6.5.2 -ABORATORY $\widehat{\mathbb{E}}$ (Blows per 100mm Increment) 10 20 30 (E DEPTH SHEAR STRENGTH REMOULDED SHEAR 100 0.0 78 117 ۱ 47 3.0 47 107 V 4.0 109 V 4.5

117 V









SCALA PENETROMETER SHEET - TABLE OF BLOWS PER INCREMENT

JOB NAME: 43A Vipond Rd & 20 Melia Pl JOB NO: 18334 TESTED BY: CD/etc.. DATE: 3-5/10/18

Testing Method: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer





SCALA PENETROMETER SHEET - TABLE OF BLOWS PER INCREMENT

JOB NAME: 43A Vipond Rd & 20 Melia PI JOB NO: 18334 TESTED BY: CD/etc.. DATE: 3-5/10/18 Stanmore Bay

	Stariiio										
Depth of Penetration [mm]	AH113	AH114	AH116	AH117	AH118	AH119		AH120	AH121	AH122	8
DEPTH START[m]	4.40	5.00	5.00	5.00	5.00	5.00	7.00	5.00	5.00	5.00	
50 mm	2	SUNK	SUNK	0.5	3	0.5	9	1	5	1	
100	4		\vee	0.5	5	0.5	10	2	1	1	
150	5	V	1	1	5	1	10	2	2	12	
200	3	1	2	1	6	1	10	2	2		
250	4	1	2	1	8	1	10	2	2	1)	
300	3	1	2	2	8	1	41	4	2	2	
350	5	1	2	2	10	2		3	3	2	
400	10	1	2	2	11	2		4	3	2	
450	9	2	3	3	14	2		4	2	2	
500	10	2	3	3	14	2		4	3	2	
550	10	3	3	3	13	2		4	4	2	
600	10	3	4	3		2	X	5	3	3	
650	6	2	3	3		3		6	3	3	
700	6	3	3	4		3		6	3	3	
750	10	5	3	4		3		6	4	3	
800	10	6	4	4		3		9	4	4	
850	10	5	4	4	•	4		11	4	4	
900	10	5	7	5	^	4		11	4	4	
950	10	5	10	5		4		10	4	4	
1000		5	10	7		4		12	5	9	
1050		12	6	7		5		12	5	7	
1100		7	7	7		6			5	11	
1150			7	7		5			8	8	
1200		7	7	9		6			5	12	
1250		7	7	9		6			7	10	
1300		7	7	7		6			6	10	
1350		7	7	7		7			7	10	
1400		10	8	10		8			6	10	
1450		10	10	10		7			7		
1500		10	10	10		7			8		
1550		11	10	10		7			6		
1600		10	10	10		7			7		
1650			10			8			10		
1700						8			10		
1750						9			10		
1800						9			10		
1850						10			10		
1900						10					
1950						9					
2000						7					
DEPTH END [m]	5.35	6.60	6.65	6.60	5.55	7.00	7.30	6.05	6.85	6.40	

Testing Method: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer



SS105 (AH105)

Shrink-Swell Test Results

Job Name: 43A Vipond Road, Stanmore Bay
Date: 04.10.18
Sample Location: SS105 (AH105)
Sampling method: Push Tube
Sampling depth (m): 0.4-0.7
Sample condition: Good

Job No: 18334
Tested By: SV
Date Sampled: 03.10.18
Sampled By: CD
Inert inclusions (%): CD
Extent of cracking (%): 5
Extent of crumbling (%): 0

Silty CLAY, trace fine sand, trace rootlet inclusions, light grey and orange, very

Sample description: stiff, moist, highly plastic (Natural)

Wet Density Dry Density $\gamma (t/m^3) = 1.73$ $\gamma_d (t/m^3) = 1.14$

Shrinkage Test

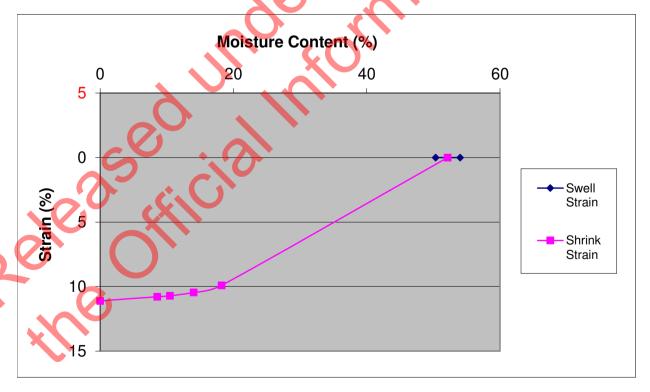
Initial moisture content (%) = 52.1 ϵ_{sh} = Magnitude of total shrinkage strain (%) = 11.1

Swell Test

 ε_{sw} = Magnitude of the swelling strain (%) = 0.1

(Note: The ε_{sw} value is negative if the sample has undergone consolidation)

Initial moisture content (%) = 54.0 Final moisture content (%) = 50.3



Shrink-Swell index

Iss = 6.2 Strain per Δ pF (%)

Testing Method: AS1289.7.1.1 - 1998 Soil reactivity tests



SS111 (AH111)

Shrink-Swell Test Results

Job Name: 43A Vipond Road, Stanmore Bay Job No: 18334 Date: 04.10.18 Tested By: SV Sample Location: SS111 (AH111) Date Sampled: 03.10.18 Sampling method: Push Tube Sampled By: CD Sampling depth (m): Inert inclusions (%): 0.4 - 0.7Sample condition: Good Extent of cracking (%): Extent of crumbling (%)

Clayey SILT, trace fine sand, light grey and light orange, very stiff, moist,

Sample description: moderately plastic (Natural)

Wet Density
Dry Density

 $\gamma (t/m^3) = 1.74$ $\gamma_d (t/m^3) = 1.17$

Shrinkage Test

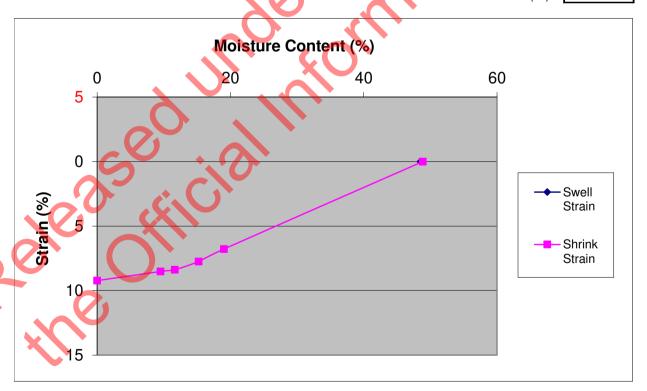
Initial moisture content (%) = $\frac{48.9}{\epsilon_{sh}}$ = Magnitude of total shrinkage strain (%) = $\frac{9.2}{\epsilon_{sh}}$

Swell Test

 ε_{sw} = Magnitude of the swelling strain (%) = -0.1

(Note: The ε_{sw} value is negative if the sample has undergone consolidation)

Initial moisture content (%) = 48.5 Final moisture content (%) = 48.9



Shrink-Swell index

Iss = 5.1 Strain per Δ pF (%)

Testing Method: AS1289.7.1.1 - 1998 Soil reactivity tests



SS119 (AH119)

Shrink-Swell Test Results

Job Name: 43A Vipond Road, Stanmore Bay
Date: 04.10.18
Sample Location: SS119 (AH119)
Sampling method: Push Tube
Sampling depth (m): 0.5-0.9
Sample condition: Good

Job No: 18334
Tested By: SV
Date Sampled: 03.10.18
Sampled By: CD
Inert inclusions (%):
Extent of cracking (%): 5
Extent of crumbling (%): 0

Clayey SILT, trace fine to medium sand, orange with grey streaks, very stiff,

Sample description: moist, moderately plastic (Natural)

Wet Density Dry Density $\gamma (t/m^3) = 1.84$ $\gamma_d (t/m^3) = 1.34$

Shrinkage Test

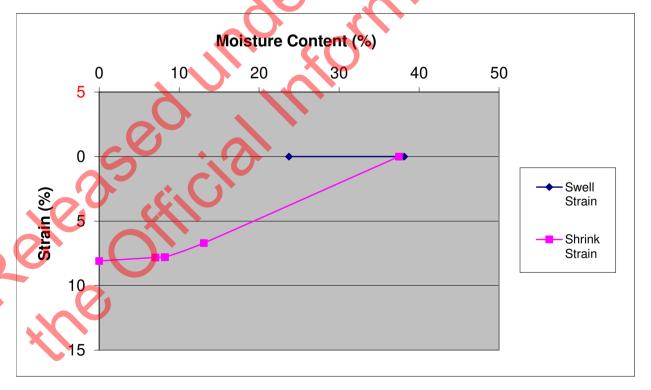
Initial moisture content (%) = $\frac{37.5}{\epsilon_{sh}}$ = Magnitude of total shrinkage strain (%) = $\frac{8.1}{\epsilon_{sh}}$

Swell Test

 ε_{sw} = Magnitude of the swelling strain (%) = -1.0

(Note: The ε_{sw} value is negative if the sample has undergone consolidation)

Initial moisture content (%) = 38.2 Final moisture content (%) = 23.7



Shrink-Swell index

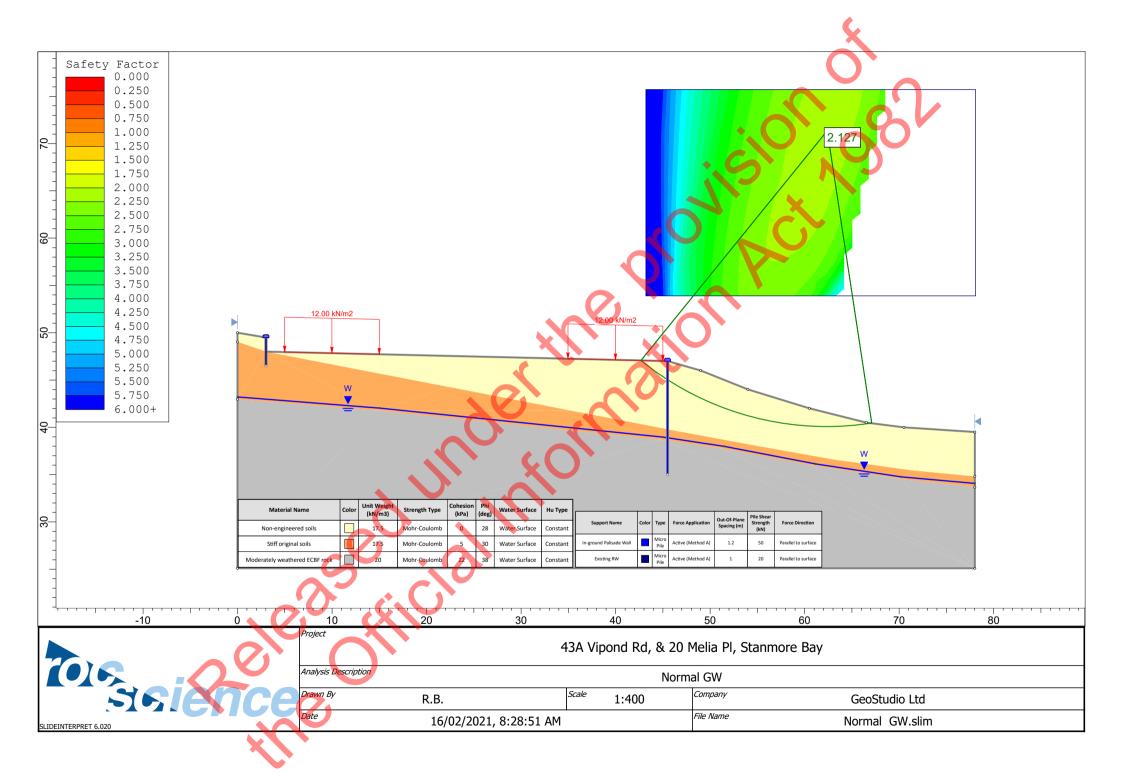
Iss = 4.5 Strain per Δ pF (%)

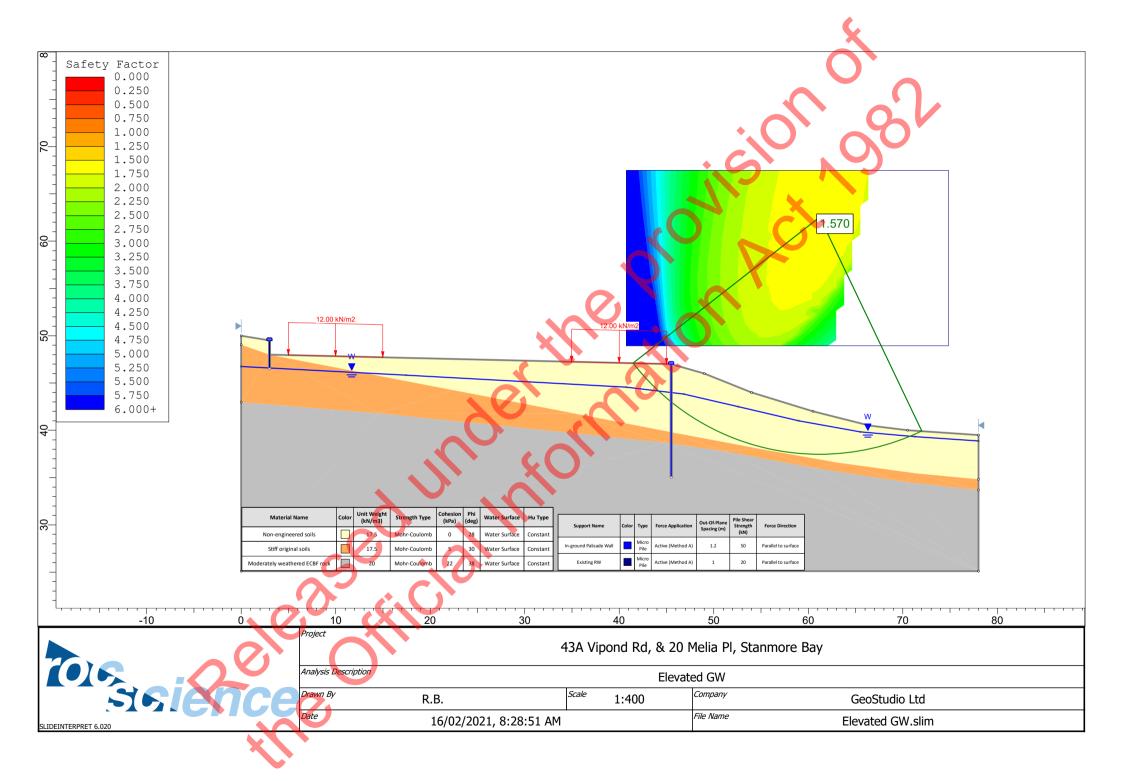
Testing Method: AS1289.7.1.1 - 1998 Soil reactivity tests

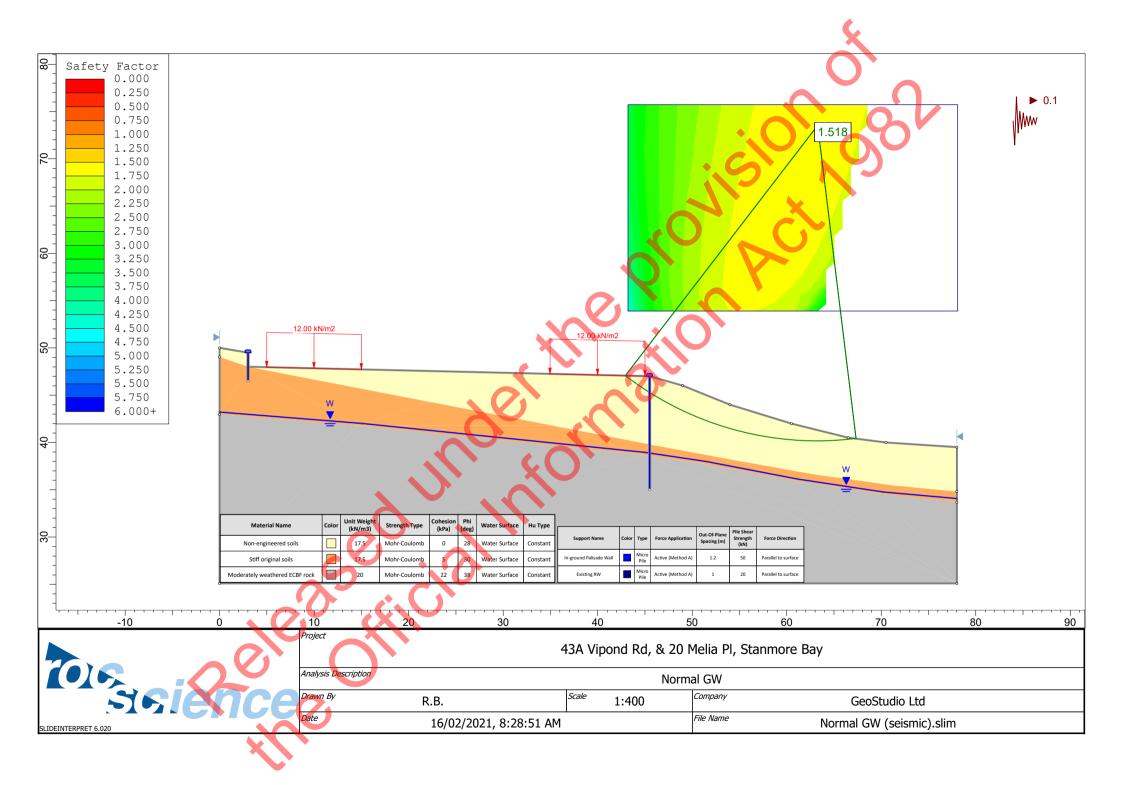


Appendix C

Slope Stability Analyses





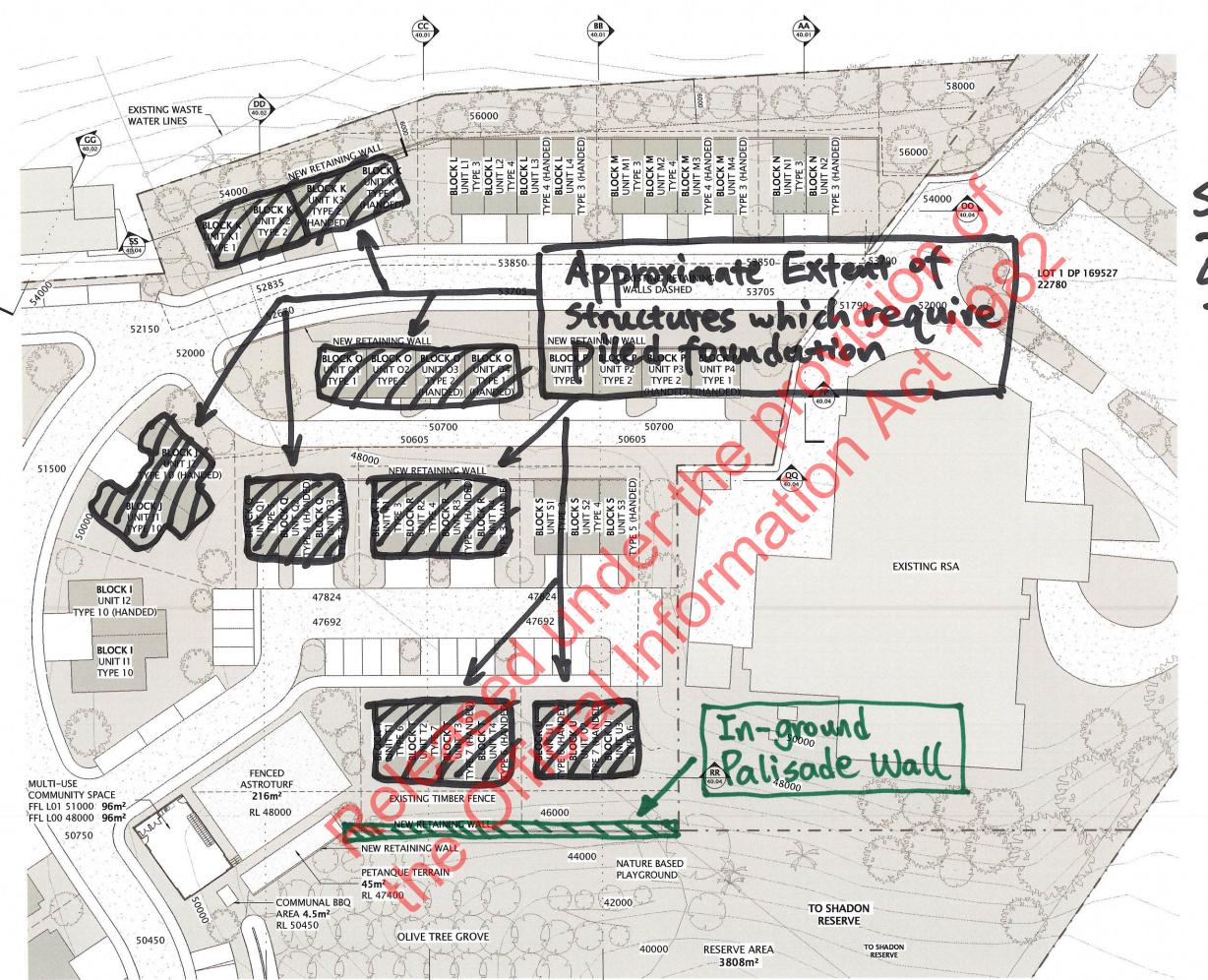




Appendix D

SK-2282-01, SK-2282-02 & SK-2282-03

GeoStudio Ltd D
www.geostudio.co.nz
2282_43A Vipond Road & 20 Melia Place, Stanmore Bay - Geotechnical Investigation Report B.docx



PROPOSED OVERALL DEVELOPMENT – ZONE 2

SCALE: 1:250

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STANDARDS AND LOCAL TERRITORIAL AUTHORITY RECULATIONS
DRAWINGS TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT
DOCUMENTATION
DO NOT SCALE OFF DRAWINGS

SK-22l2-01 Feastudio Ltd

DRAFT

REV DATE INITIAL AMENDMEN

REVISIONS

PROJECT STATUS

MELIA DEVELOPMENT LIMITED

20 MELIA PLACE

20 MELIA PLACE, WHANGAPARAOA

SHEET TITLE

PROPOSED DEVELOPMENT ZONE 2

TRUE NORTH

DESIGN DRAWN

CM LM

SCALE @ A1 (HALF SCALE IF PRINTED @ A3) 1:250

FIRST ISSUE DATE PROJECT No. 01/27/21

20053 REVISION

10.03



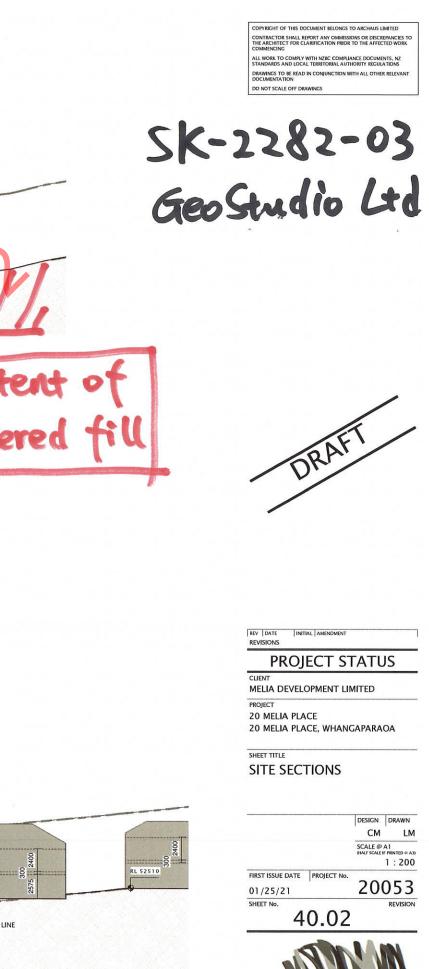
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10.01 SCALE: 1:200 1:28 ACCESSWA EXTG FENCE RL 56750 8m HEIGHT PLANE RL 53798 RL 54650 RL 53451 RL 50798 RL 50704 EXISTING RETAINING WALLS SHOWN RED RL 49289 RL 52054 NEW RETAINING EXISTING RETAINING RL 47121 REV DATE INITIAL AMENDM OLIVE TREE GROVE RL 47304 REVISIONS **PROJECT STATUS** MELIA DEVELOPMENT LIMITED PROJECT BB SITE SECTION B-B
10.01 SCALE: 1:200 20 MELIA PLACE 20 MELIA PLACE, WHANGAPARAOA SHEET TITLE SITE SECTIONS CM LM SCALE @ A1 (HALF SCALE IF PRINTED @ A3) ROAD PARKING 1:200 FIRST ISSUE DATE PROJECT No. 20053 01/25/21 8m HEIGHT PLANE 40.01 RL 54381 RL 50704 RL 53210 RL 50798 EXISTING RETAINING WALLS SHOWN RED RL 47765 RL 48004 NEW RETAINING WALL EXISTING RETAINING WALL NEW RETAINING WALL paterson + cullen + archaus CC SITE SECTION C-C 10.01 SCALE: 1:200

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PROJECT STATUS

MELIA DEVELOPMENT LIMITED

20 MELIA PLACE 20 MELIA PLACE, WHANGAPARAOA

SITE SECTIONS

CM LM

SCALE @ A1 (HALF SCALE IF PRINTED @ A3)

1:200

01/25/21

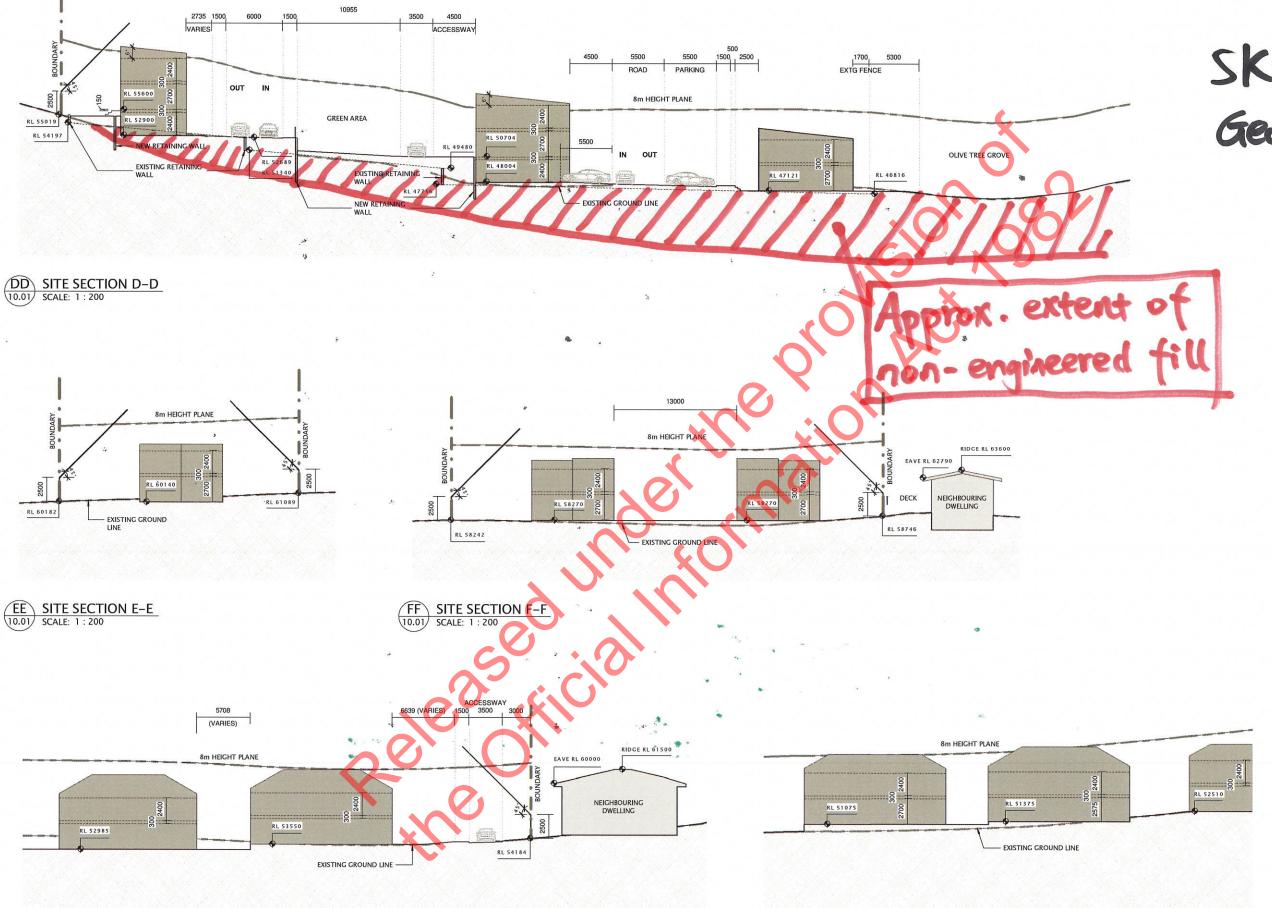
20053

40.02

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HH SITE SECTION H-H
10.01 SCALE: 1 : 200

GG SITE SECTION G-G
10.01 SCALE: 1:200